DeKalb County

DeKalb County Department of Planning & Sustainability

330 Ponce De Leon Avenue, Suite 500 Decatur, GA 30030

(404) 371-2155 / plandev@dekalbcountyga.gov

Michael Thurmond Chief Executive Officer Planning Commission Hearing Date: January 4, 2022 Board of Commissioners Hearing Date: January 27, 2022

STAFF ANALYSIS

Case No.:	SLUP-21-1244886	Agenda #: D3	
Location/Address:	3795 North Druid Hills Ro Decatur, GA 30033	ad, Commission District: 02 Super District: 06	
Parcel ID:	18-100-04-019		
Request:	Special Land Use Permit (SLUP) request to allow for a drive-through restaurant.		
Property Owner(s):	Midtown National Group, LP		
Applicant/Agent:	David Kirk, on behalf of Chick-Fil-A, Inc.		
Acreage:	1.04 acres		
Existing Land Use:	Retail Commercial		
Surrounding Properties:	To the north of the subject property is North Druid Hills Road, to the south is North DeKalb Mall, west is commercial, and to the east is commercial.		
Adjacent Zoning:	North: R75, O-I South: C	7-1 East: C-1 West: C-1 X	
Comprehensive Plan:	Town Center (TC)	Consistent Inconsistent	
Proposed Density: N.A.		Existing Density: N.A.	
Proposed Square Ft.: 2,800 Sq. Ft.		Existing Units/Square Feet: N.A.	
Proposed Lot Coverage: 6.14%		Existing Lot Coverage: N.A.	

Subject Property and Surrounding Area

The subject property is a 1.04-acre site located on the south side of North Druid Hills Road bordering the north side of the North DeKalb Mall. An on-ramp to I-285 is located approximately 1,700 feet to the east of Lawrenceville Highway (Highway 78). The property is currently occupied by a vacant commercial retail establishment; formerly a Pier 1 Imports store. The adjoining and nearby land uses to the east, south, and west are commercial uses zoned C-1 and NS. The adjoining and nearby land uses to the north are mix of residential and commercial uses zoned R-75 and O-I. The commercial uses to the east of the subject property and fronting on North Druid Hills Road consist of four drive-through restaurants (Zaxby's, McDonalds, Chickfil-A, and Checkers). Also, to the east of the subject property, uses include an existing dry-cleaners, auto repair shop, liquor store, gas station, and jeweler. There is also an existing 3-story commercial building that appears to have Class C office space with various existing businesses which include a store front church, hair salon, and a driving school. West of the subject property are two existing one story buildings home to Peachtree Spine Physicians as well as a pet hospital. South of the subject property is an undeveloped lot and North Dekalb Mall. It is important to note that the mall technically closed in 2020, however, there are a few stores, a U.S. Postal Service branch, and an AMC movie theatre still operating along the perimeter of the physical mall site. To the north of the subject property, is a mix of residential and commercial uses. The residential uses are mostly located in the North Druid Woods neighborhood. It is a modest neighborhood consisting of one store ranch style homes. Office residential uses along this portion of the corridor consist of a holistic health center, auto insurance office, a dry cleaner, travel agents office, massage therapist, and chiropractor. In recent months there have been discussions about the re-development of the nearby North Druid Hills mall site into a mixed-use development. Because malls such as North Druid Hills mall are typical of suburban areas, we do not expect that the development of a mixed-use product will create a highly dense and walkable urban landscape independent of cars, adequate parking, and wider roads.

Zoning History

Based on DeKalb County records, it appears that the NS (Neighborhood Shopping) zoning of the property has not changed since adoption of the first zoning ordinance and map in 1956. The *DeKalb County 2035 Comprehensive Plan* designates the subject property's future land use as Town Center (TC).

Project Analysis

The applicant is requesting to rezone the subject property from NS (Neighborhood Shopping) to C-1 (Local Commercial) for the purpose of constructing a drive-through restaurant. The applicant's request proposes the relocation of the Chick-fil-A restaurant from 3905 N. Druid Hills Road to the subject property. Based on the site plan provided by the applicant, the construction of the 2,800 square foot drive through restaurant will require the demolition of the vacant one-story building which was a furniture and home essentials retail establishment (Pier 1 Imports). The project will make use of the two existing connections to Birch Road and North Druid Hills Road. These connections will serve as both entrances and exits for customers. The North Druid Hills access will serve as a right-in/right-out only. The drive-through restaurant will consist of three drive-through lanes that merge into two lanes as they approach the drive through service area, which will permit servers to walk food out instead of using a drive-through window. The redeveloped site is proposed to accommodate vehicular stacking for 41 cars and include 29 parking spaces, of which, two will be designated for patrons with disabilities. The restaurant will also have outdoor seating for customers on the east side of the building. Additionally, the developer intends to install a variety of trees and shrubs around the site to beautify it and buffer some of the vehicular activity.

COMPLIANCE WITH DISTRICT STANDARDS

STANDARD	REQUIREMENT	PROPOSED	COMPLIANCE
MAX LOT COVERAGE	90%	69.3%	Yes
BUILDING MATERIALS	Brick, stone, stucco, architectural concrete, glass.	Mostly brick.	Yes.
FRONT BUILDING SETBACK	10ft min/60ft max	< 60ft.	YES
SIDE – CORNER LOT ON PUBLIC STREETS SETBACK	30ft.	< 30ft.	NO (variance needed)
SIDE INTERIOR BUILDING SETBACK	15 Feet	>15ft.	Yes
REAR BUILDING SETBACK	20 Feet	> 20ft.	YES
HEIGHT	2 stories/35 feet	1 story/21 feet	Yes
PARKING	1:150sf (min)= 19 spaces 1:75sf (max) = 37 spaces	29 parking spaces	Yes
PARKING CONFIGURATION	Non-residential buildings in an Activity Center shall have no more than one (1) double row of parking within the front yard where there is no intervening building between parking and the street.	No parking is proposed in the front yard. All parking is on side and in back.	Yes

PARKING CONFIGURATION	Non-residential buildings in an Activity Center shall be allowed to locate parking along the side or rear or as on-street parking dedicated as right-of-way by the applicant for a land disturbance permit or	Parking is located to the side and rear of the building.	Yes
OPEN SPACE	building permit.	30.6%	Yes
SIDEWALKS AND STREETSCAPING	6-ft. sidewalk, 10-ft. landscape strip, street trees 40 ft. on center (N. Druid Hills Rd.)	10ft Yes	No. However, adjustments may be made pending GDOT, County Public Works, or ZBA variances
	6-ft. sidewalk, 6-ft. landscape strip, street trees 50 ft. on center (Birch Rd.)		Yes

Impact Analysis

Section 27-7.4.6 of the DeKalb County Code states that the following criteria shall be applied in evaluating and deciding any application for a Special Land Use Permit.

A. Adequacy of the size of the site for the use contemplated and whether or not adequate land area is available for the proposed use including provision of all required yards, open space, off-street parking, and all other applicable requirements of the zoning district in which the use is proposed to be located.

Located on 1.04 acres, adequate land area is available to operate a restaurant with a drive-through lane and comply with all required yards, open space, and off-street parking required within the C-1 (Local Commercial) Zoning District. However, there are transportation concerns stemming from future expansion of North Druid Hills Road. Road network studies are still in progress, so it is unknown what the road design will be and what the right-of-way width will be in the future. Nonetheless, the applicant has proposed to dedicate the necessary right-of-way to satisfy transportations comments in the short-term. The applicant has submitted an unofficial rendering of a widened right-of-way with the redevelopment (see *Future Roadway Improvement Concept Option*). Related to the future right-of-way width, the applicant proposes to situate the building closer to North Druid Hills Road to achieve activity center goals. However, the North Druid Hills Road side of the property is technically the corner side and not its front. Thus, the proposed building placement does not comply with 30-foot minimum setback. Additionally, the building placement may conflict with future road widening.

B. Compatibility of the proposed use with adjacent properties and land uses and with other properties and land uses in the district.

The proposed drive-through restaurant is consistent with the surrounding drive-through facilities in the area including a Zaxby's drive-through restaurant directly adjacent to the subject property, a McDonalds drive-through restaurant, and a Checkers drive-through restaurant. There is also a Chevron gas station and an auto repair shop nearby. All of these sites are east of the subject property along North Druid Hills Road. In addition, there are existing residential office uses to the northeast of the subject site comprising of a mixture of uses that include a dry cleaner, insurance agency, massage therapist, and travel agency.

C. Additionally, it is important to note that since this proposal was last presented, North DeKalb Mall has been purchased with the intent to redevelop the site. As the core of this Town Center activity center, it is the desire of the community to see the site redeveloped in a manner consistent with *Comprehensive Plan* (60DUs/acre, 6-story maximum height, and a mixture of residential and nonresidential uses at the core). It is likely that any development constructed with the density and intensity envisioned in the *Comprehensive Plan* will have a significant impact on the subject property and the surrounding area. The development potential of the site is slated to be studied over the next few months. Consequently, a redevelopment plan has not been submitted to the Planning and Sustainability Department. Adequacy of public services, public facilities, and utilities to serve the proposed use.

Given that the area along North Druid Hills Road is developed with various commercial uses, it appears that there are adequate public services and utilities for the proposed drive-through restaurant.

D. Adequacy of the public street on which the use is proposed to be located and whether or not there is sufficient traffic-carrying capacity for the use proposed so as not to unduly increase traffic and create congestion in the area.

North Druid Hills Road is a major arterial. The County Transportation Division is desirous of a westbound left turn lane on North Druid Hills Road at Burch Road. From observation, left turns at this intersection are challenging once eastbound traffic on N. Druid Hills Road builds. The applicant has provided a memo that affirms the need for the turning lane (see *CFA North Druid Hills Relo Traffic Memo*). A right in/right out ingress/egress is proposed, therefore, patrons approaching from the east will need that turn lane to more efficiently access the site from Birch Road. Major concerns for transportation staff are future roadway expansion that will be designed to alleviate congestion along the North Druid Hills Road corridor and how subject properties along the corridor may be impacted. The amount of land the county will need to obtain is currently unknown. Other transportation concerns are traffic conditions and access into and out of the site.

E. Whether existing land uses located along access routes to the site will be adversely affected by the character of the vehicles or the volume of traffic generated by the proposed use.

Given the surrounding drive-through businesses, it does not appear that the character of the vehicles will adversely impact existing land uses along access routes to the site. Staff performed a site visit on June 16, 2021 at 1:30pm and traffic congestion was observed in both directions on North Druid Hills Road. Additionally, it was observed that the carrying capacity for the dedicated right turn lane on North Druid Hills Road adjacent to the subject property and Zaxby's may not be sufficient to accommodate right out traffic from the subject property and the full access ingress/egress at Zaxby's during peak hours, which may present some vehicular conflicts. Concerns have been raised about access onto North Druid Hills Road from Druid Hills Court if this site is developed into a Chick-Fil-A. While this is a reasonable concern that may or may not be exacerbated by the redevelopment proposal, it is noteworthy to mention that residents in the community have multiple entrance and exit options. Those alternatives include Mistletoe Road, Laurel Hill Drive, Arbordale Drive, Harcourt Drive, and Willivee Drive.

F. Adequacy of ingress and egress to the subject property and to all proposed buildings, structures, and uses thereon, with particular reference to pedestrian and automotive safety and convenience, traffic flow and control, and access in the event of fire or other emergency.

Two points of access are proposed. One point of access is on North Druid Hills Road (right in/right out only) and the other point of access (full access) is on Birch Road. Emergency vehicles can access the site from either point of ingress/egress. In addition to the observations in Criterion E, Zaxby's secondary ingress/egress is onto an access easement to its south which directs traffic to Birch Road and Sweet Briar Road adjacent to the North DeKalb Mall.

According to the site plan, internal circulation is centered on a "horseshoe" pattern to access three canopied drive-through lanes that narrow to two lanes for food delivery and parking around the perimeter of the site. Assuming the bulk of sales are via drive-through service, the proposed circulation plan may create a few issues: 1. the 2 parking spaces for disabled patrons are away from the building and close to the right in/right out access point at North Druid Hills Road; 2. drive-thru patrons who have received their food will also depart near the handicapped spaces and as patrons enter from North Druid Hills; 3. given the proposed layout, reasonable signage may be necessary to ensure internal circulation flows as intended.

Given the popularity of the proposed establishment, it may be more efficient and may lessen potential conflicts, if the applicant considers, full access to/from Birch Road, and either a connection to the existing access easement to distribute traffic around the mall (where there are multiple controlled access intersections to North Druid Hills Road and Lawrenceville Highway) and/or install adequate signage to encourage drivers to take advantage of multiple ingress/egress points around the mall. It is currently unknown what the scope of work will consist of for the North Druid Hills Road improvements, however, when that scope is fully developed it could impact the site. Additionally, the applicant provided a crash data comparison (see *Crash Data Exhibit*) of the existing location and the subject property along with an intersection analysis (see *N Druid Hills and Birch Rd Intersection Analysis*) which indicates that the relocation of the Chick-Fil-A will place it at a location with fewer accidents and that the redevelopment will not significantly alter the level of service at the Birch Road/North Druid Hills Road intersection, respectively. The Transportation Division is still reviewing this analysis.

G. Whether the proposed use will create adverse impacts upon any adjoining land use by reason of noise, smoke, odor, dust, or vibration generated by the proposed use.

The proposed drive-through restaurant should not create significant adverse impacts upon any adjoining land use by reason of noise, smoke, odor, dust or vibration. Considering that the restaurant is already located along the corridor, its relocation may generate only marginal change compared to what exists presently.

Prepared 12/20/2021 by: DJ SLUP-21-1244886/D2

H. Whether the proposed use will create adverse impacts upon any adjoining land use by reason of the hours of operation of the proposed use.

The hours of operation for the proposed drive-through lane should not create adverse impacts upon adjoining land uses. The proposed use will not be a 24-hour establishment. It will operate on standard Chick-Fil-A hours of operation which are Monday through Saturday 6am to 10pm.

I. Whether the proposed use will create adverse impacts upon any adjoining land use by reason of the manner of operation of the proposed use.

The manner of operation for the proposed drive-through restaurant could possibly impact the flow of traffic in an existing right turn lane that serves an existing Zaxby's drive-through restaurant, However the transition from dine-in at the current location to drive-thru (and walk-up) only at the proposed location will have limited impact on the adjoining land uses. See criteria E and F also.

J. Whether the proposed use is otherwise consistent with the requirements of the zoning district classification in which the use is proposed to be located.

The applicant currently has a companion rezoning application on the agenda to rezone the subject property from NS (Neighborhood Shopping) to C-1 (Local Commercial). As a preferred zoning district within the town center character area, it is implied that the C-1 zoning district and its collection of permissible principal and accessory uses are suitable; prohibited uses are not suitable; and that other uses are subject to special land use permit approval in order to evaluate the appropriateness of those uses based on impacts that may be unique depending on their settings. Drive-through restaurants are permitted uses within the C-1 zoning district with an approved Special Land Use Permit (SLUP). While the proposed site plan complies with most of the C-1 zoning requirements, the building location would be subject to variance approval from the Zoning Board of Appeals because it would be within the required 30-foot side corner setback.

K. Whether the proposed use is consistent with the policies of the comprehensive plan.

The site is located within a Town Center Character Area designated by the *DeKalb County 2035 Comprehensive Plan*. See criterion S also.

Although a drive-through establishment is not the ideal use for this activity center, mitigating factors may include that the establishment is relocating about one-quarter of a mile away from its existing location within the activity center, and it would be located along a major arterial road. Because the subject property is located on the perimeter of the TC activity center, consideration should be given to striking an appropriate balance based on current conditions and future objectives for parcels with direct access to North Druid Hills Road such as shared access, inter-parcel connectivity, and fewer curb cuts until a major catalyst for change occurs at the core to set the baseline for surrounding concurrency.

Furthermore, there have been other similar special land use permits that have been approved within activity centers. In November 2020, the Board of Commissioners approved SLUP 20-1244105 request for a drive-thru restaurant located at 2933 N. Druid Hills Road (Planning Commission voted to approve with conditions; Staff also recommended approval). In January 2021, the Board of Commissioners approved SLUP 21-1244417 request for a drive-thru restaurant located at 3033 North Druid Hills Road (Planning Commission voted to approve with conditions; Staff also recommended approval).

L. Whether the proposed use provides for all required buffer zones and transitional buffer zones where required by the regulations of the zoning district in which the use is proposed to be located.

The proposed site does not abut any residentially zoned properties. Therefore, buffer zones are not required.

M. Whether there is adequate provision of refuse and service areas.

Adequate refuse areas will be provided based on the submitted information by the applicant.

N. Whether the length of time for which the special land use permit is granted should be limited in duration.

There does not appear to be any compelling reasons for limiting the duration of the requested Special Land Use Permit.

O. Whether the size, scale and massing of proposed buildings are appropriate in relation to the size of the subject property and in relation to the size, scale and massing of adjacent and nearby lots and buildings.

The proposed one-story building would be compatible in size and massing with adjacent commercial buildings in the area.

P. Whether the proposed use will adversely affect historic buildings, sites, districts, or archaeological resources.

There are no known historic building, sites, districts or archeological resources in the immediate area that will be adversely affected by the proposed use.

Q. Whether the proposed use satisfies the requirements contained within the supplemental regulations for such special land use permit.

The proposed drive-through restaurant complies with most of the following supplemental regulations per Sec.27-4.2.23 of the DeKalb County Zoning Code:

- a. **Drive-through facilities shall not be located within sixty feet of a residentially zoned property.** The nearest residential zoning is the R-75 zoning to the north approximately 90 feet away.
- b. No drive-through facility shall be located on property less than ten thousand square feet in area, unless part of a mixed-use development. Stacking spaces for queuing of cars shall be provided for the drive-through area as required in Article 6. All driveway entrances, including stacking lane entrances, must be at least fifty (50) feet from an intersection. The distance is measured along the street from the junction of the two street curb lines to the nearest edge of the entrance. The property is 45,302 square feet (1.04 acres) in size. Forty-one stacking spaces are provided. The nearest intersection is at North Druid Hills Road and Birch Road. The North Druid Hills Road entrance is located about 100 feet from the intersection and the Birch Road entrance is located about 130 feet from the intersection.
- c. Drive-through lanes and service windows serving drive-through lanes shall only be located to the side or rear of buildings. The proposed drive through lanes and one service door are located to the rear of the proposed building.
- d. Drive-through canopies and other structures, where present, shall be constructed from the same materials as the primary building and with a similar level of architectural quality and detailing. Per submitted plans, it appears that the canopies will be constructed with the same materials as the building.

- e. Speaker boxes shall be pointed away from any adjacent residential properties and shall require masonrysound attenuation walls with landscaping or other speaker volume mitigation measures.Speaker boxes shall not play music but shall only be used for communication for placing orders. Per the submitted plans, the speaker boxes will be positioned so that they are not interfering with residential uses.
- f. All lighting from drive-through facilities shall be shaded and screened so as to be directed away from any adjacent residential property. No lighting or photometric plan was submitted.
- g. Stacking spaces shall be provided for any use having a drive-through facility or areas having drop-off andpick- up areas in accordance with the following requirements. Stacking spaces shall be a minimum of ten
 - (10) feet wide and twenty-five (25) feet long. Stacking spaces shall begin at the last service window for thedrive- through lane (typically the "pick-up" window). Per the submitted preliminary site plan, the stacking spaces are 11.5' wide and meet the 25'minimum in length.
- h. All drive-through facilities with the exception of drive-through restaurants shall provide at least three stacking spaces for each window or drive-through service facility. Per the submitted preliminary site plan
 - ,there are 41 car stacking spaces provided.
- i. The following standards shall apply to all stacking spaces and drive-through facilities:
 - i. Drive-through lanes shall not impede on- and off-site traffic movements, shall not cross or passthrough off-street parking areas, and shall not create unsafe conditions where crossed by pedestrian access to a public entrance of a building. The proposed drive-through lane, as currently designed, might cause internal circulation issues that could affect traffic both on-site andoff-site. These circulation issues might also interfere with pedestrians going to and from the handicapped spaces.
 - *ii.* **Drive-through lanes shall be separated by striping or curbing from off-street parking areas.** Individual lanes shall be striped, marked, or otherwise distinctly delineated. *Based on the submittedpreliminary site plan, there appears to be striping and material delineation shown to separate lanes.*
 - iii. All drive-through facilities shall include a bypass lane with a minimum width of ten (10) feet, bywhich traffic may navigate around the drive-through facility without traveling in the drive-through lane. The bypass lane may share space with a parking access aisle.
 - iv. Drive-through lanes must be set back five (5) feet from all lot lines and roadway right-of-way lines. The proposed drive-through lane is at least 15 feet away from all property lines and roadwayright- of-way lines.
 - v. Owner and operator are responsible for daily litter clean-up to ensure the property remains freeof trash, litter, and debris. The owner and operator shall be responsible for daily litter clean-up to ensure the property remains free of trash, litter, and debris.
 - vi. Drive-through restaurants shall not be located within five hundred (500) feet of an elementary, middle, or high school. The proposed drive-through restaurant is a least 1,000 feetfrom Laurel Ridge Elementary School.
 - vii. Drive-through restaurants located in activity centers require a special land use permit. In all othercharacter areas, a special land use permit is required unless the facility can meet at least two of the following criteria: a. Facility is located within four hundred (400) feet of an intersection of a major arterial street and a major or minor arterial street, or within one

thousand (1,000) feet of an interstate highway interchange. b. Facility is accessible only through inter-parcel access or through a shared driveway. c. Facility is part of a major development as defined in Art. 8.1.16. The proposed drive-through restaurant is located within a Town Center activity center and is requesting a Special Land Use Permit.

R. Whether the proposed use will create a negative shadow impact on any adjoining lot orbuilding as a result of the proposed building height.

The proposed one-story drive-through restaurant will not create a negative shadow impact on any adjoining lotor building.

S. Whether the proposed use would be consistent with the needs of the neighborhood or the community as a whole, be compatible with the neighborhood, and would not be in conflict with the overall objective of the comprehensive plan.

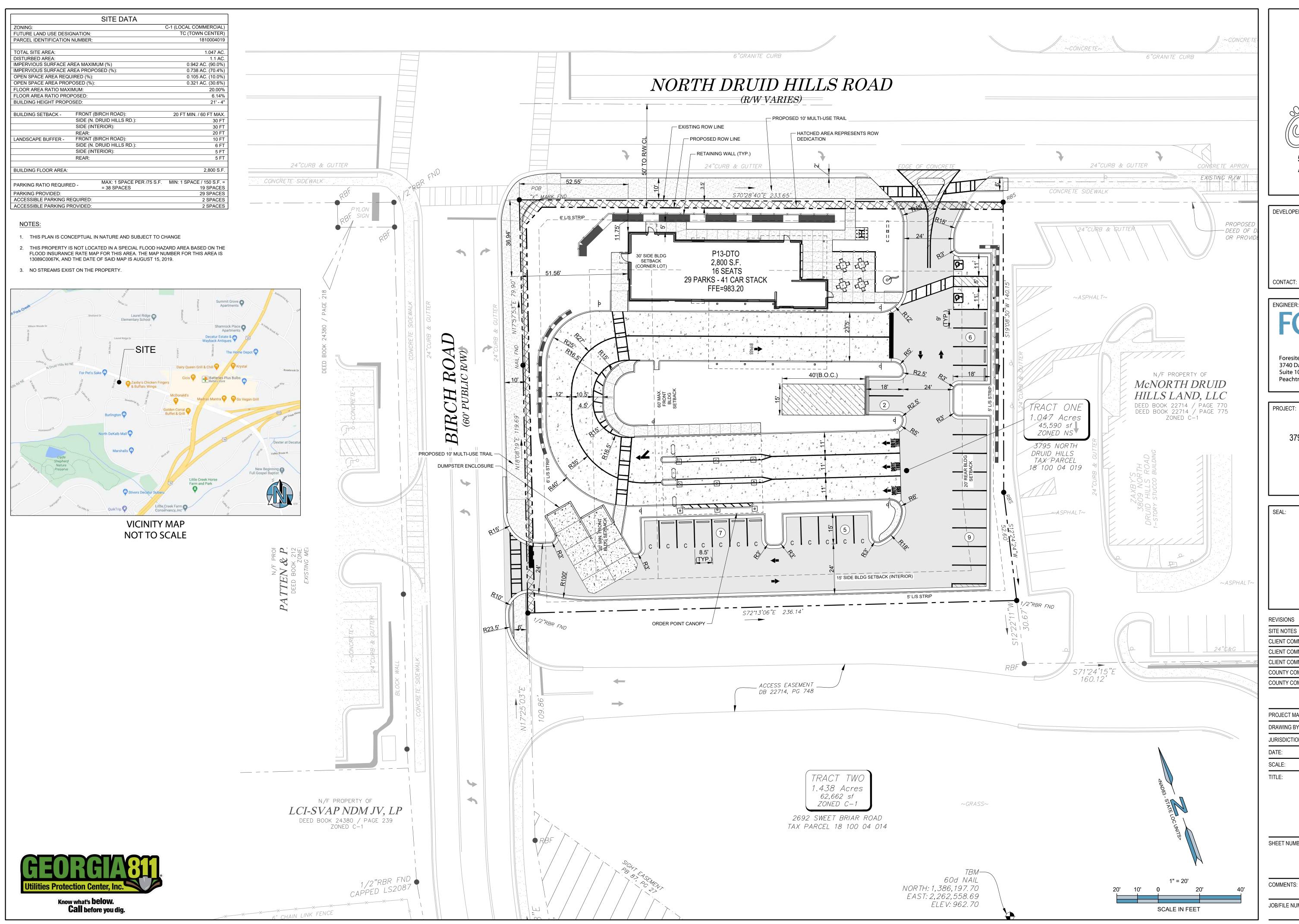
The complexity of this proposal is a conflict of aspirational versus practical; future versus present. The subject property is located in the Town Center (TC) activity center future land use designation. The intent of the Town Center character area is "to promote the concentration of residential and commercial structures, which serve many communities in order to reduce automobile travel, promote walkability and increased transit usage. The areas consist of a focal point for several neighborhoods with a variety of activities such as general retail, commercial, professional office, higher-density housing, and appropriate public and open space uses that are easily accessible by pedestrians." Associated planning principles that are envisioned in this activity center include: pedestrian-friendly design, mixed-use development, greater density of development, taller development, and parking at the rear of building among others. Therefore, the applicant proposes to situate the new building closer to the N. Druid Hills Road along with installation of parking and drive-thru facilities to the side and/or rear. However, it is important to note that the subject property sits on the outer edge of this TC activity center. Some of the preferred development standards apply more so at the core and taper off towards the outer edges. Additionally, since N. Druid Hills Road is a major arterial road, along this portion of the corridor, it may not be prudent to promote greater pedestrian activity directly along the corridor. Pedestrian facilities and gathering places may be more suitable internally between the properties along N. Druid Hills Road and the North DeKalb Mall site. While we desire a more pedestrian friendly environment overall for this activity center at its core, the parcels along North Druid Hills Road may be suitable for auto dependent uses.

Moreover, where you have an existing urban fabric that is dense and walkable is where high density land uses integrate best. In cases where there's existing suburban and auto focused land uses, higher density projects may not integrate into the existing environment fully and yield the desired benefits. Because there is an existing suburban fabric the need for wider roads and more parking ties up much of the land and reduces pedestrian activity which makes high density land uses much more difficult establish.

If the Planning Commission recommends approval of this request, then the Department of Planning and Sustainability recommends the following conditions below:

- 1. The subject property shall be developed in substantial conformity with the following site plans: "Preliminary Site Plan", prepared by Foresite Group, dated 11-17-2021 and with the building elevation prepared by Selser Schaefer Architects, dated 7-14-2021.
- 2. No exterior sound systems, outdoor speakers or sound amplification systems are permitted.
- 3. The Special Land Use Permit shall be issued to Midtown National Group, LP for a Chick-Fil-A drive-through restaurant only and shall not be transferrable for the purposes of establishing any other drive-through establishment.
- 4. The approval of this Special Land Use Permit application by the Board of Commissioners has nobearing on the requirements for other regulatory approvals under the authority of the Zoning Board of Appeals, or otherentities whose decision should be based on the merits of the application under review by such entity.
- 5. Truck loading/delivery shall not impede internal circulation on-site.
- 6. The applicant shall install adequate internal signage. Illustrations and design specifications for internal signage must be provided to the Director of Planning & Sustainability prior to building permit approval.
- 7. If the restaurant partners with meal delivery service providers (e.g., Door Dash, Uber Eats, etc.), a portion of the excess parking spaces for passenger vehicles shall be designated for those meal delivery service providers.

UPDATED PLANS SUBMITTED BY APPLICANT ON November 17, 2021





ATLANTA, GA 30349-2998

DEVELOPER:

CHICK-FIL-A, INC. 5200 BUFFINGTON ROAD ATLANTA GA, 30349-2998 (404) 808-1145

CONTACT: MR. JASON POCIASK

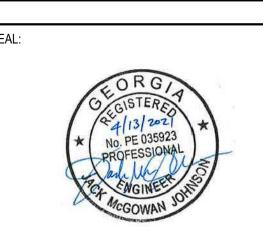
Foresite Group, LLC. 3740 Davinci Ct. Suite 100

w | www.fg-inc.net **o** | 770.368.1399 **f |** 770.368.1944 Peachtree Corners, GA 30092

PROJECT:

3795 NORTH DRUID HILLS RD. DECATUR, GA 30033 PARCEL #:1810004019

FSR #04846



REVISIONS	DATE
SITE NOTES	2021.04.29
CLIENT COMMENTS	2021.06.25
CLIENT COMMENTS	2021.07.13
CLIENT COMMENTS	2021.09.08
COUNTY COMMENTS	2021.10.21
COUNTY COMMENTS	2021.11.17

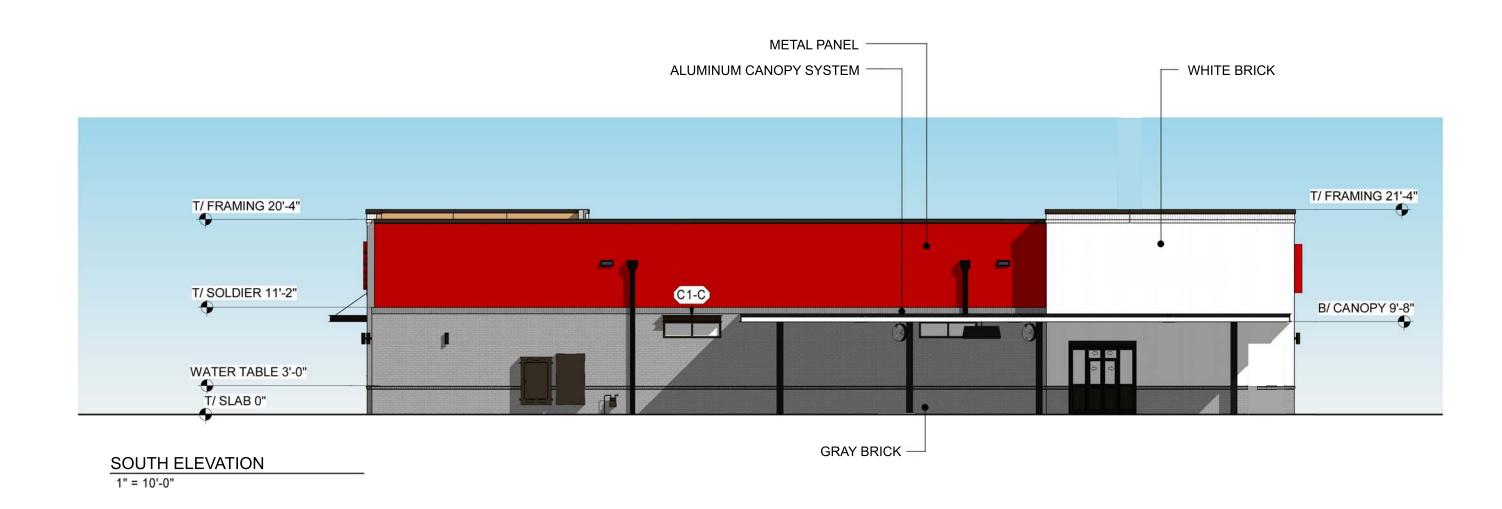
PROJECT MANAGER:	JMJ
DRAWING BY:	JRH
JURISDICTION:	DEKALB COUNTY, GA
DATE:	2021-04-12
SCALE:	1" = 20'
TITLE:	

PRELIMINARY SITE PLAN

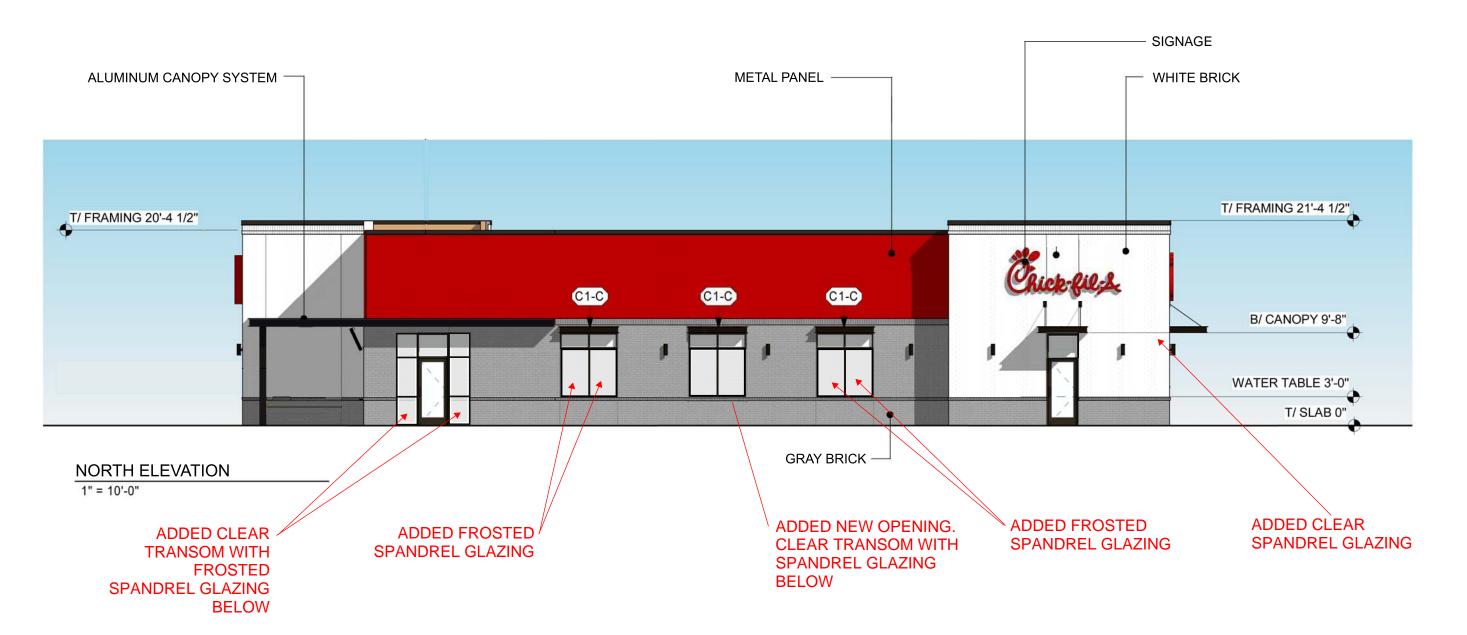
SHEET NUMBER:

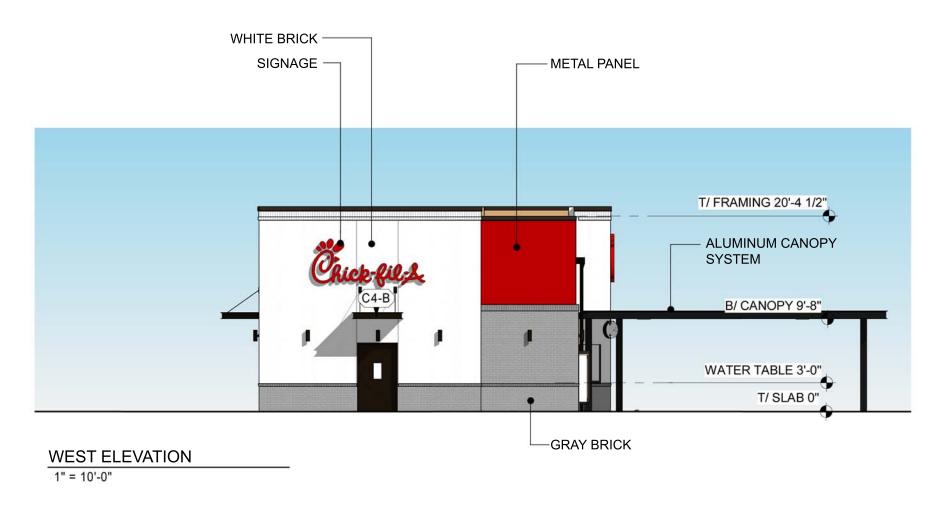
NOT RELEASED FOR CONSTRUCTION

JOB/FILE NUMBER: 397.010

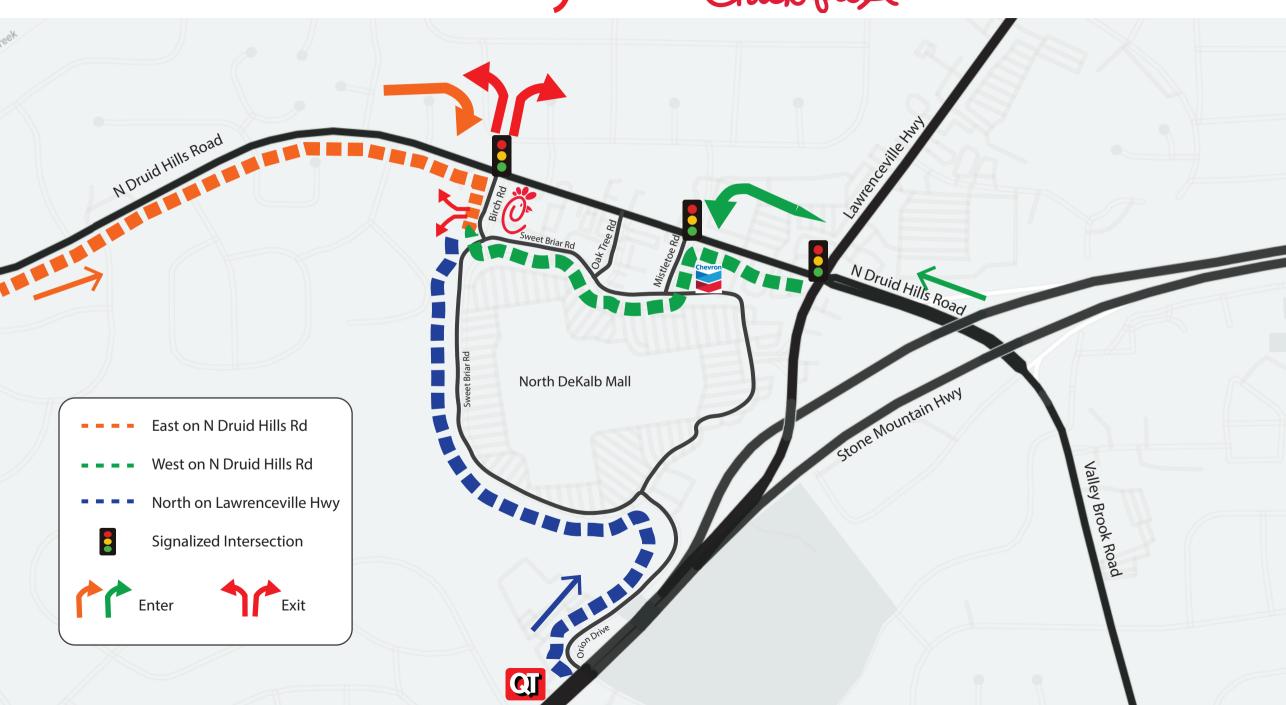








Best ways to Chick-file: &





DEPARTMENT OF PLANNING & SUSTAINABILITY

SPECIAL LAND USE PERMIT APPLICATION

Amendments will not be accepted after 5 working days after the filing date.

Date Received:	Application No.:
	E:David Kirk, on behalf of Chick-fil-A, Inc.
Daytime Phone #:	404-885-3415 Fax #: N/A
	E-mail:David.Kirk@troutman.com
	Midtown National Group LP
more than one own	ner, attach contact information for each owner)
Daytime Phone #: _	N/A Fax #: Fax #:
Mailing Address:	9171 TOWNE CENTRE DR, STE 335, SAN DIEGO CA 92122
	E-mail:N/A
SUBJECT PROPE	RTY ADDRESS OR LOCATION: 3795 DRUID HILLS RD N
Decatur	, DeKalb County, GA,
District(s): Unknown	Land Lot(s): 100 Block(s): Unknown Parcel(s): 18 100 04 019
	Feet: Commission District(s): 2 and SD6 Existing Zoning:NS (C-1 Propose
Proposed Special L	and Use (SLUP):Restaurant with Drive-through
I hereby authorize t subject of this appli	the staff of the Planning and Development Department to inspect the property that is the cation.
Owner: Agent (Check One)	t: X Signature of Applicant: Den C. Kill
Printed Name of A	Applicant: David C. Kirk
Notary Signature and	Seal:
Comme W	Caldwell Rublic Public

My Commission Expires
January 14, 2022

Troutman Pepper Hamilton Sanders LLP 600 Peachtree Street NE, Suite 3000 Atlanta, GA 30308-2216

troutman.com



David C. Kirk david.kirk@troutman.com

May 5, 2021

VIA U.S. CERTIFIED MAIL

Address Line Address Line

Re: DeKalb County Community Meeting #2

Dear Neighbor:

I am writing to inform you of Chick-fil-A, Inc.'s ("Chick-fil-A") proposed Rezoning and Special Land Use Permit Applications (the "Applications") to be submitted to the DeKalb County Department of Planning & Sustainability to allow for the construction and operation of a new Chick-fil-A drive-through restaurant at 3795 North Druid Hills Road NE. This new restaurant will replace the older drive-through restaurant located nearby at 3905 North Druid Hills Road. The requested Rezoning Application seeks to rezone the property from Neighborhood Shopping ("NS") to Local Commercial ("C-1"). The Special Land Use Permit Application seeks approval of the associated drive-through facility. I am attaching for your review a copy of the Site Plan showing the proposed new restaurant and associated drive-through.

Chick-fil-A is holding a second Virtual Community Meeting via Zoom on <u>Thursday, May 20,</u> <u>2021</u> at 5:30 P.M., at which time Chick-fil-A will share details of the proposal with attendees. To join the Virtual Community Meeting, please enter the web address below into your internet browser (with no spaces) and follow the prompts to join the meeting.

Web Address:

https://troutman.zoom.us/j/92657834227?pwd=K1pGRWpKSk0xM1pBMldyKzdNREIKZz 09

Meeting ID: 926 5783 4227

Password: 425462



You may also join the Virtual Community Meeting by phone via the following number:

Number: 1-929-436-2866

Meeting ID: 926 5783 4227

Passcode: 425462

Should you have any questions about the Applications and proposed Chick-fil-A restaurant, please do not hesitate to give me a call at (404) 885-3415.

Sincerely,

David C. Kirk

CHICK-FIL-A COMMUNITY MEETING SIGN-IN SHEET

Wednesday, April 28, 2021 5:30 p.m. Virtual Meeting Via Zoom

NAME	ADDRESS
Stacey Russell	Eyetravel737@gmail.com (Mount Olive Drive)
Cedric Hudson	(County)

Troutman Pepper Hamilton Sanders LLP 600 Peachtree Street NE, Suite 3000 Atlanta, GA 30308-2216

troutman.com



David C. Kirk david.kirk@troutman.com

April 29, 2021

VIA FEDERAL EXPRESS AND EMAIL

Mr. Andrew Baker, AICP Director DeKalb County Department of Planning & Sustainability 330 West Ponce de Leon Avenue, Suites 100-500 Decatur, Georgia 30030

Re: Rezoning and Special Land Use Permit Application for 3795 North Druid Hills Road

Dear Mr. Baker:

On behalf of Chick-fil-A, Inc. ("Chick-fil-A" or the "Applicant"), I am pleased to provide for review and consideration by DeKalb County this letter of intent and the accompanying application materials in support of the requested Rezoning of the above-referenced property (the "Subject Property") from its current classification Neighborhood Shopping (NS) to the Local Commercial (C-1) classification and Special Land Use Permit ("SLUP") to allow for a drive-through restaurant. If approved, the Rezoning and SLUP will allow for the redevelopment of the Subject Property, which currently contains a vacant retail storefront, into a new, custom-designed 2,800 square-foot Chick-fil-A restaurant with a drive-through facility, outdoor seating, pedestrian improvements, and enhanced landscaping. The proposed development will update the Subject Property into an attractive, modern restaurant in keeping with the demands of the current market, customer expectations and team member needs, and current County requirements.

Included with this letter of intent are the following materials:

- A. An Application to Amend the Official Zoning Map of DeKalb County;
- B. A Special Land Use Permit Application;
- C. A Site Plan;
- D. A Survey and Legal Description of the Subject Property;
- E. A Landscape Plan:



- F. Architectural Elevations and Renderings; and
- G. An impact analysis for the requested SLUP and justification for the proposed Rezoning (included within the body of this letter).

Summary of the Proposed Project

As noted above, the Applicant seeks approvals necessary to redevelop the Subject Property, which now contains a vacant retail store (previously occupied by Pier 1 Imports) and associated street-fronting parking lot. On this property, Chick-fil-A proposes to construct and operate a new, custom-designed restaurant containing approximately 2,800 square feet of space. The restaurant will include a drive-through facility located in the rear of the property with 41 stacking spaces, 29 off-street parking spaces, outdoor seating, an improved streetscape, and enhanced landscaping. The Subject Property is currently zoned Neighborhood Shopping (NS), which does not permit drive-through restaurants. The Applicant requests the Subject Property to be rezoned from NS to Local Commercial (C-1) to allow for a drive-through restaurant. The proposed new restaurant will replace the older Chick-fil-A drive-through restaurant located nearby at 3905 North Druid Hills Road and is designed to better reflect the County's desired goals for design, traffic efficiency, and pedestrian accessibility.

Zoning Map Amendment Review and Approval Criteria

Proposed zoning amendments are evaluated in light of the following standards.

1. Whether the zoning proposal is in conformity with the policy and intent of the Comprehensive Plan.

Chick-fil-A's proposed redevelopment of the Subject Property is consistent with the County's Comprehensive Development Plan and Future Land Use Map, which designates the Subject Property as "Town Center." The Subject Property is currently occupied by a commercial retail use and will continue to be used commercially as a result of the proposed redevelopment. The Town Center character area specifically permits C-1 zoning classifications. The proposed building design, including its orientation to the street, enhanced streetscape improvements, and "walk-up" window will encourage pedestrian accessibility consistent with the Town Center goals.

2. Whether the zoning proposal will permit a use that is suitable in view of the use and development of adjacent and nearby properties.

The Subject Property is directly adjacent to a Zaxby's drive-through restaurant and is located across the street from a physician's office and the North DeKalb Mall. The proposed restaurant is consistent with the adjacent and nearby commercial uses located along North Druid Hills Road and will replace the vacant retail storefront with a custom-designed restaurant. The Applicant respectfully submits the zoning proposal will permit a use that is suitable in view of the use and development of adjacent and nearby properties and will not only efficiently accommodate vehicular traffic but will also be more welcoming to pedestrians.



3. Whether the property to be affected by the zoning proposal has a reasonable economic use as currently zoned.

The Applicant respectfully submits the proposed redevelopment will substantially enhance the economic use of the property by replacing the existing vacant retail store into a new, custom-designed Chick-fil-A restaurant with a drive-through facility, outdoor seating, enhanced landscaping, and streetscape improvements.

4. Whether the zoning proposal will adversely affect the existing use or usability of adjacent or near-by properties.

The proposed redevelopment will include a custom-designed, well-landscaped restaurant, and associated drive-through. The proposed redevelopment will have a positive effect on the surrounding neighborhood as it will encourage and accommodate pedestrian access.

5. Whether there are other existing or changing conditions affecting the use and development of the property which give supporting grounds for either approval or disapproval of the zoning proposal.

The Subject Property is currently occupied by a vacant retail storefront and associated street-fronting parking lot. The zoning proposal is intended to allow for the redevelopment of the Subject Property as a Chick-fil-A restaurant in a manner that better reflects the County's desired goals for design, traffic efficiency, and pedestrian accessibility. Chick-fil-A seeks to make a significant investment in this location and redevelop the existing site in a manner that it believes will benefit the surrounding community. The proposed drive-through will be located behind the new restaurant building and thus will be shielded from view along the public right-of-way. The proposed site improvements will result in more efficient, inviting, and safe pedestrian and traffic movement within the Subject Property and on nearby roadways and sidewalks.

6. Whether the zoning proposal will adversely affect historic buildings, sites, districts, or archaeological resources.

The Applicant respectfully submits the zoning proposal will have no adverse effect on any historic or archaeological resources. The redevelopment will be designed consistent with County's design, transportation accessibility, and landscape goals and the Applicant believes the improvement of the Subject Property will have a positive impact on adjacent and nearby properties.



7. Whether the zoning proposal will result in a use which will or could cause excessive or burden-some use of existing streets, transportation facilities, utilities or schools.

The zoning proposal will not cause an excessive or burdensome use of existing streets, transportation facilities, or utilities and will have no impact on school enrollment. The more pedestrian-oriented design of the site should encourage customers who live or work nearby to walk to the restaurant.

Special Land Use Permit Review Criteria

 Adequacy of the size of the site for use contemplated and whether or not adequate land area is available for the proposed use including provision of all required yards, open space, off-street parking, and all other applicable requirements of the zoning district in which the use is proposed to be located.

The Subject Property is just over one acre in size and is sufficient to accommodate the proposed restaurant, outdoor seating, drive-through facility, and necessary parking.

2. Compatibility of the proposed use with adjacent properties and land uses and with other properties and land uses in the district.

The Subject Property is directly adjacent to a Zaxby's drive-through restaurant and is located across the street from a physician's office and the North DeKalb Mall. The proposed restaurant is consistent with the commercial uses located in this portion of the County and along North Druid Hills Road and will replace the vacant retail storefront with a custom-designed restaurant. The Applicant respectfully submits the zoning proposal will permit a use that is suitable in view of the use and development of adjacent and nearby properties and will not only efficiently accommodate vehicular traffic but will also be more welcoming to pedestrians

3. Adequacy of public services, public facilities, and utilities to serve the use contemplated.

Existing public facilities and services are adequate to serve the proposed restaurant and drive-through facility.

4. Adequacy of the public street on which the use is proposed to be located and whether or not there is sufficient traffic-carrying capacity for the use proposed so as not to unduly increase traffic and create congestion in the area.

There is sufficient traffic carrying capacity for the proposed restaurant use on North Druid Hills Road. The zoning proposal will allow for the redevelopment of the Subject Property in a manner that better reflects the County's desired goals for design, traffic efficiency, and pedestrian accessibility. The proposed drive-through will be located



behind the new restaurant building and thus will be shielded from view along the public right-of-way. The proposed site improvements will result in efficient, inviting, and safe pedestrian and traffic movement within the Subject Property and on nearby roadways and sidewalks.

5. Whether or not existing land uses located along access routes to the site will be adversely affected by the character of the vehicles or the volume of traffic generated by the proposed use.

The Applicant submits the proposal will have no adverse effect on existing uses located on North Druid Hills Road. The proposed redevelopment should have no impact on the character or volume of vehicular traffic on North Druid Hills Road.

6. Ingress and egress to the subject property and to all proposed buildings, structures, and uses thereon, with particular references to pedestrian and automotive safety and convenience, traffic flow and control, and access in the event of fire or other emergency.

Ingress and egress will be provided from North Druid Hills Road and Birch Road. The proposed site improvements will result in efficient, inviting, and safe pedestrian and traffic movement within the Subject Property and on nearby roadways and sidewalks.

7. Whether or not the proposed use will create adverse impacts upon any adjoining land use by reason of noise, smoke, odor, dust, or vibration generated by the proposed use.

The proposed use will not create noise, smoke, odor, dust, or vibration.

8. Whether or not the proposed use will create adverse impacts upon any adjoining land use by reason of the hours of operation of the proposed use.

The proposed restaurant use will be open Monday through Saturday from approximately 6:30 a.m. to 10:00 p.m. The proposed hours of operation are consistent with other commercial uses located along North Druid Hills Road and will have no negative impact on adjoining land uses.

9. Whether or not the proposed use will create adverse impacts upon any adjoining land use by reason of the manner of operation of the proposed use.

The proposed use will be operated in a safe and efficient manner by a local Chick-fil-A operator and will have no adverse impact on adjoining property.

10. Whether or not the proposed plan is consistent with all of the requirements of the zoning district classification in which the use is proposed to be located.

The proposed Chick-fil-A restaurant is designed to be consistent with the goals and site development standards of the proposed C-1 zoning district.



11. Whether or not the proposed use is consistent with the policies of the Comprehensive Plan.

Chick-fil-A's proposed redevelopment of the Subject Property is consistent with the County's Comprehensive Development Plan and Future Land Use Map, which designates the Subject Property as "Town Center." The Subject Property is currently occupied by a commercial retail use and will continue to be used commercially as a result of the proposed redevelopment. The Town Center character area specifically permits C-1 zoning classifications. The proposed building design, including its orientation to the street, enhanced streetscape improvements, and "walk-up" window will encourage pedestrian accessibility consistent with the Town Center goals.

12. Whether or not the proposed plan provides for all required buffer zones and transitional buffer zones where required by the regulations of the district in which the use is proposed to be located.

The proposed site design provides for all required setbacks and buffer zones.

13. Whether or not there is adequate provision of refuse and service areas.

As shown on the attached Site Plan, the proposed site design provides for adequate refuse and service areas.

14. Whether the length of time for which the special land use permit is granted should be limited in duration.

Given Chick-fil-A's significant proposed investment in the redevelopment of the Subject Property, Chick-fil-A requests the Special Land Use Permit be granted with no expiration.

15. Whether or not the size, scale and massing of proposed buildings are appropriate in relation to the size of the subject property and in relation to the size, scale and massing of the adjacent and nearby lots and buildings.

The proposed one-story 2,800 square foot restaurant is appropriate in scale and size relative to the Subject Property and relative to other nearby buildings, which are predominantly one-story in height.

16. Whether the proposed plan will adversely affect historic buildings, sites, districts, or archaeological resources.

The zoning proposal will have no adverse effect on any historic or archaeological resources.



17. Whether the proposed use satisfies the requirements contained within the Supplemental Regulations for such special land use permit.

The proposed restaurant and associated drive-through facility meets the standards provided in Section 4.2.23 of the County's Ordinance concerning drive-through facilities.

18. Whether or not the proposed building as a result of its proposed height will create a negative shadow impact on any adjoining lot or building.

The proposed one-story building will have no negative shadow impact on adjacent properties.

19. Whether the proposed use would result in a disproportional proliferation of that or similar uses in the subject character area.

Approval of the proposed redevelopment of the Subject Property will not result in a disproportional proliferation of restaurant or drive-through uses. The proposed restaurant will replace the existing Chick-fil-A drive-through restaurant located nearby at 3905 North Druid Hills Road.

20. Whether the proposed use would be consistent with the needs of the neighborhood or of the community as a whole, be compatible with the neighborhood, and would not be in conflict with the overall objectives of the comprehensive plan.

Consistent with the County's design goals, the objectives of the comprehensive plan, and the neighborhood and community character, the proposed new restaurant is designed to better reflect the County's desired goals for design, traffic efficiency, and pedestrian accessibility.

Summary and Conclusion

I believe this letter of intent, together with the accompanying application and supporting documents, provide all the information required by the County to review and evaluate this request. If there are other materials or information you believe would be helpful to your review of this request, please do not hesitate to contact me. I look forward to our continued cooperative efforts on this important matter.

Yours very truly,

Dan C. Kiel

David C. Kirk Attorney for Chick-fil-A

EXHIBIT A



404.371.2155 (o) 404.371.4556 (f) DeKalbCountyGa.gov Clark Harrison Building 330 W. Ponce de Leon Ave Decatur, GA 30030

Chief Executive Officer
Michael Thurmond

DEPARTMENT OF PLANNING & SUSTAINABILITY

Director

Andrew A. Baker, AICP

PRE-APPLICATION FORM REZONE, SPECIAL LAND USE PERMIT, MODIFICATION, AND LAND USE (Required prior to filing application: signed copy of this form must be submitted at filing)

Applicant Name: David Kirk, on behalf of Chick-fil-A Phone: 404-885-3415 Property Address: 3795 North Druid Hills Road Tax Parcel ID: 18 100 04 019 Comm. District(s): 2 Acreage: ____ Commercial (Retail) Commercial (Restaurant) Existing Use: Proposed Use Supplemental Regs: _____ Overlay District: _____ DRI: ____ Rezoning: Yes X No ____ Existing Zoning: NS Proposed Zoning: C-1 Square Footage/Number of Units: 2,800 sf Rezoning Request: Chick-fil-A requests to rezone the property from NS to C-1 to allow for development of a Chick-fil-A restaurant and drive-through. Chick-fil-A intends to also submit a Special Land Use Permit application to allow for the proposed drive-through facility. Land Use Plan Amendment: Yes____No_X__ Existing Land Use: _____ Proposed Land Use: _____ Special Land Use Permit: Yes x No Article Number(s) 27- Section 4.2.23 **Drive-through Facility** Special Land Use Request(s) Major Modification: Existing Case Number(s): Condition(s) to be modified:



DEPARTMENT OF PLANNING & SUSTAINABILITY

WHAT TO KNOW BEFORE YOU FILE YOUR APPLICATION		
Pre-submittal Community Meeting: Review Calendar Dates: PC:BOC:		
Letter of Intent:Impact Analysis: Owner Authorization(s): Campaign Disclosure:		
Zoning Conditions: Community Council Meeting: Public Notice, Signs:		
Tree Survey, Conservation: Land Disturbance Permit (LDP): Sketch Plat:		
Bldg. Permits: Fire Inspection: Business License: State License:		
Lighting Plan: Tent Permit: Submittal Format: NO STAPLES, NO BINDERS PLEASE		
Review of Site Plan		
Density: Density Bonuses: Mix of Uses: Open Space: Enhanced		
Open Space: Setbacks: front sides side corner rear Lot Size:		
Frontage: Street Widths: Landscape Strips: Buffers:		
Parking Lot Landscaping: Parking - Auto: Parking - Bicycle: Screening:		
Streetscapes: Sidewalks:Fencing/Walls: Bldg. Height: Bldg.		
Orientation: Bldg. Separation: Roofs: Fenestration: Roofs: Fenestration: Roofs: Fenestration: Roofs: Fenestration:		
Façade Design: Garages: Pedestrian Plan: Perimeter Landscape Strip:		
Possible Variances: retaining wall is 5' wigh @ highest point -		
no variance meder		
Comments:		
Applicant requested but did not receive Neighborhood Registry of		
complete list of confacts - will set up 200 Community acceptage		
actore amenament date of May 6.		
Planner: Mulore Furman Date 4/28/21		
Filing Fees		
REZONING: RE, RLG, R-100, R-85, R-75, R-60, MHP, RSM, MR-1 \$500.00		
RNC, MR-2, HR-1, HR-2, HR-3, MU-1, MU-2, MU-3, MU-4, MU-5 \$750.00		
OI, OD, OIT, NS, C1, C2, M, M2 \$750.00		
LAND USE MAP AMENDMENT \$500.00		
SPECIAL LAND USE PERMIT \$400.00		



404.371.2155 (o) 404.371.4556 (f) DeKalbCountyGa.gov Clark Harrison Building 330 W. Ponce de Leon Ave Decatur, GA 30030

Chief Executive Officer

DEPARTMENT OF PLANNING & SUSTAINABILITY

Director

Michael Thurmond

Andrew A. Baker, AICP

REZONE APPLICATION AUTHORIZATION

Completion of this form is required if the individual making the request is not the owner of the property.

DATE: 4/26/2021	
CHECK TYPE OF APPLICATION:	
() LAND USE PLAN	
(x) REZONE	
() MINOR MODIFICATION	
TO WHOM IT MAY CONCERN:	
(1) (1)	on behalf of Midtown National Group LP
	Name of owner(s))
being (owner)/(owners) of the property desc	cribed below or attached hereby delegate authority to
David C. Kirk, Troutman Pepper Hamilton Sa	anders LLP ant or Agent Representing Owner)
to file an application on (my) / (our) behalf.	
Notary Public	Owner
Notary Public	Owner
Notary Public	Owner
see attached	Hotory acknowledgment

X(0)(6)(6)(6)(6)(6)(6)(6)(6)(6)(6)(6)(6)(6)	international and a test action action and a test action and a test action action action and a test action ac
A notary public or other officer completing this certificate document to which this certificate is attached, and not the	ate verifies only the identity of the individual who signed the ne truthfulness, accuracy, or validity of that document.
State of California County of San Diego On Aprul 26, 2021 before me, Girl Date personally appeared San Diego	Here Insert Name and Title of the Officer Name(\$) of Signer(\$)
who proved to me on the basis of satisfactory subscribed to the within instrument and acknowly	evidence to be the person(s) whose name(s) is/afe ledged to me that ne/she/they executed the same in is/her/their signature(s) on the instrument the person(s),
GIOVANNA DELIA Notary Public - California San Diego County Commission # 2352087	I certify under PENALTY OF PERJURY under the laws of the State of California that the foregoing paragraph is true and correct. WITNESS my hand and official seal. Signature Signature of Notary Public
Place Notary Seal Above	TIONAL
Though this section is optional, completing this fraudulent reattachment of this	TIONAL information can deter alteration of the document or form to an unintended document.
Description of Attached Document	
Title or Type of Document: Document Date:	Number of Pages:
Signer(s) Other Than Named Above:	
Capacity(ies) Claimed by Signer(s) Signer's Name: Corporate Officer — Title(s):	
☐ Partner — ☐ Limited ☐ General	☐ Partner — ☐ Limited ☐ General
☐ Individual ☐ Attorney in Fact	☐ Individual ☐ Attorney in Fact
☐ Trustee ☐ Guardian or Conservator	☐ Trustee ☐ Guardian or Conservator
☐ Other: Signer Is Representing:	☐ Other:Signer Is Representing:

EXHIBIT B

404.371.2155 (o) 404.371.4556 (f) DeKalbCountyGa.gov Clark Harrison Building 330 W. Ponce de Leon Ave Decatur, GA 30030

DEPARTMENT OF PLANNING & SUSTAINABILITY

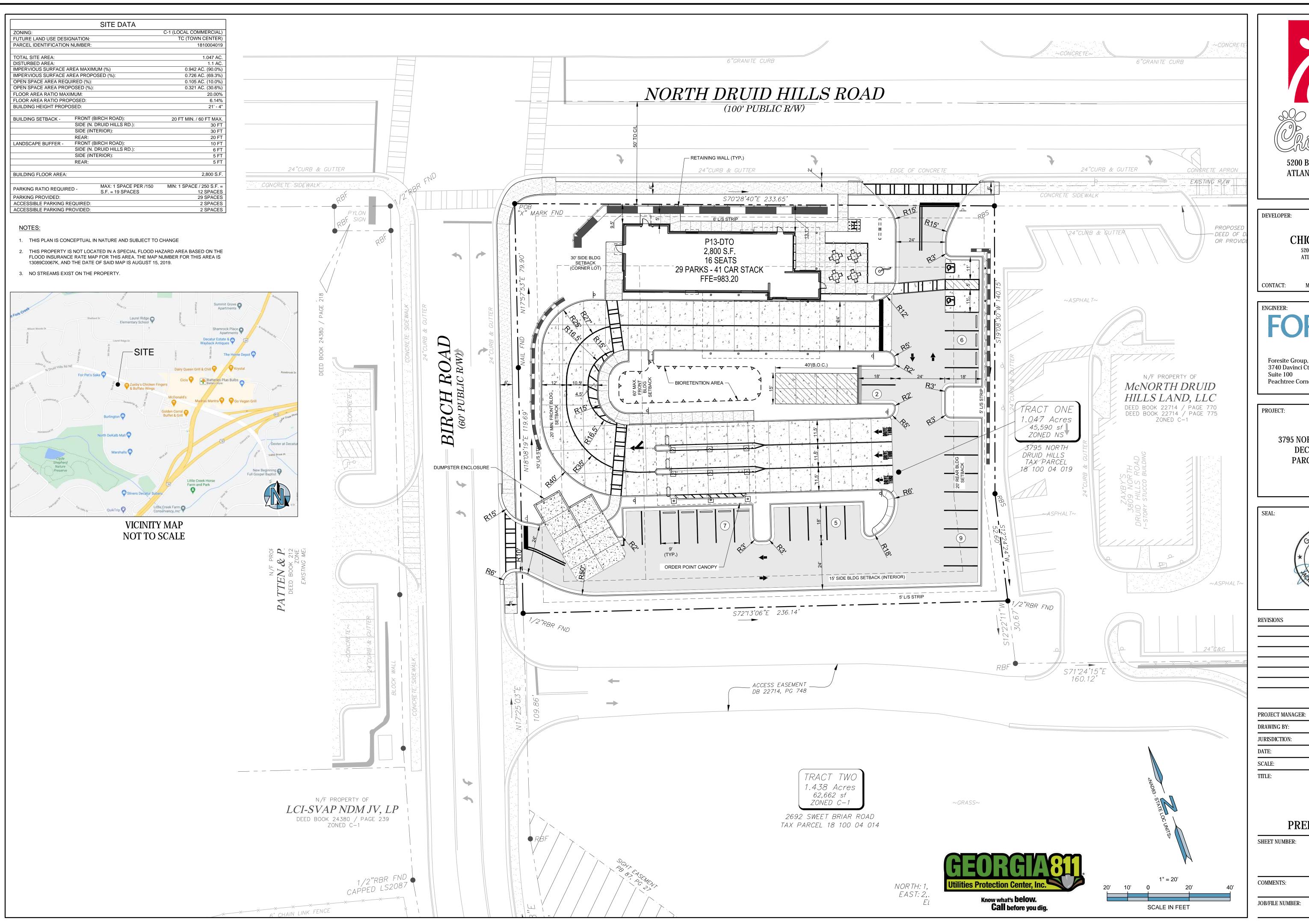
SPECIAL LAND USE PERMIT APPLICATION AUTHORIZATION

The property owner should complete this form or a similar, signed and notarized form if the individual who will file the application with the County is not the property owner.

Date: 4/26/2021	
TO WHOM IT MAY CONCERN:	
(1) (WE), Danon Young	, on behalf of Midtown National Group LP
	Name of Owner(\$\psi\$)
being (owner) (owners) of the subject property David C. Kirk, Troutman Pepper Hamilton	described below or attached hereby delegate authority to n Sanders LLP
Nam	ne of Applicant or Agent
to file an application on (my) (our) behalf.	
Notary Public	Owner
Notary Public	Owner
Notary Public	Owner
Notary Public	Owner
see attached	d Hotary acknowledgment

A notary public or other officer completing this certificate document to which this certificate is attached, and not the	e verifies only the identity of the individual who signed the truthfulness, accuracy, or validity of that document.
State of California County of San Diego On April 26, 2021 before me, Gior Date personally appeared Danon	Vanna Delia Notany Public, Here Insert Name and Title of the Officer Voung Name(s) of Signer(s)
subscribed to the within instrument and acknowle	evidence to be the person(s) whose name(s) (s/are dged to me that he/she/they executed the same in her/their signature(s) on the instrument the person(s), ed, executed the instrument.
GIOVANNA DELIA Notary Public - California San Diego County Commission # 2352087	certify under PENALTY OF PERJURY under the laws f the State of California that the foregoing paragraph true and correct. //ITNESS my hand and official seal. ignature Signature of Notary Public
Place Notary Seal Above	ONAL
Though this section is optional, completing this in	onal of the document or of the document or
Description of Attached Document Title or Type of Document: Document Date: Signer(s) Other Than Named Above:	Number of Pages:
Capacity(ies) Claimed by Signer(s) Signer's Name: Corporate Officer — Title(s): Partner — Limited General Individual Attorney in Fact Trustee Guardian or Conservator Other: Signer Is Representing:	Signer's Name: Corporate Officer — Title(s): Partner — Limited General Individual Attorney in Fact Trustee Guardian or Conservator Other: Signer Is Representing:

EXHIBIT C





DEVELOPER:

CHICK-FIL-A, INC. 5200 BUFFINGTON ROAD ATLANTA GA, 30349-2998 (404) 808-1145

CONTACT: MR. JASON POCIASK

Foresite Group, LLC. w | www.fg-inc.net 3740 Davinci Ct. o | 770.368.1399 Suite 100 f | 770.368.1944 Peachtree Corners, GA 30092

PROJECT:

3795 NORTH DRUID HILLS RD. DECATUR, GA 30033 PARCEL #:1810004019

FSR #04846



PROJECT MANAGER:	J IVIJ
DRAWING BY:	JRH
JURISDICTION:	DEKALB COUNTY, GA
DATE:	2021-04-12
SCALE:	1" = 20'
TITLE:	

PRELIMINARY SITE PLAN

SHEET NUMBER:

NOT RELEASED FOR CONSTRUCTION

397.010

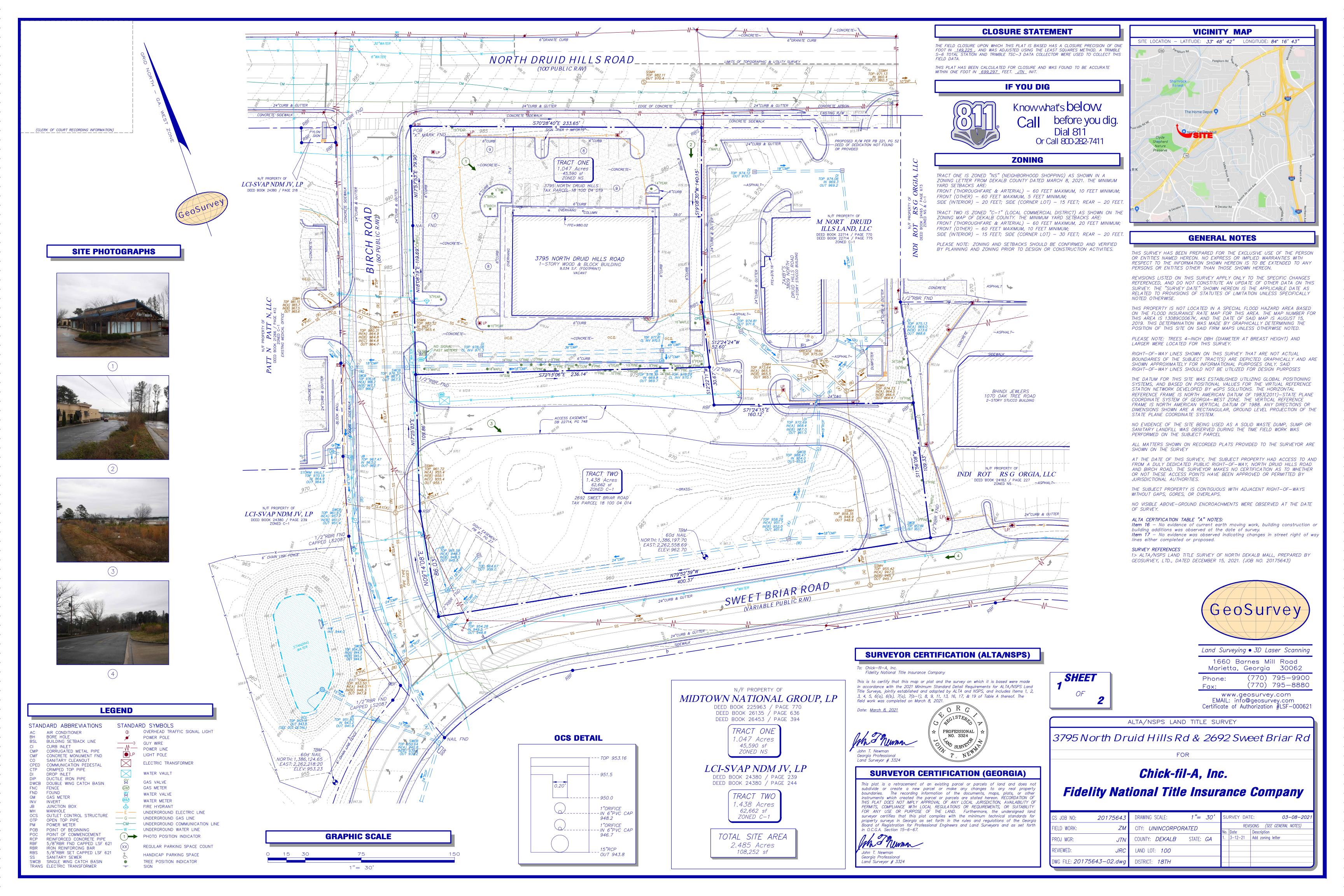
EXHIBIT D

TRACT ONE PROPERTY DESCRIPTION

All that tract or parcel of land lying or being in Land Lot 100, 18th District, Dekalb County, Georgia, and being more particularly described as follows:

Beginning at an X mark found at the intersection of the Easterly right-of-way of Birch Road (60 foot right-of-way) with the Southerly right-of-way of North Druid Hills Road (100 foot right-of-way); thence along said right-of-way of North Druid Hills Road South 70 degrees 28 minutes 40 seconds East, a distance of 233.65 feet to a 5/8 inch rebar set; thence leaving said right-of-way South 19 degrees 08 minutes 30 seconds West, a distance of 140.15 feet to a 5/8 inch rebar set; thence South 12 degrees 24 minutes 24 seconds West, a distance of 52.60 feet to a 1/2 inch rebar found; thence North 72 degrees 13 minutes 06 seconds West, a distance of 236.14 feet to a 1/2 inch rebar found on the Easterly right-of-way of Birch Road; thence along said right-of-way North 18 degrees 08 minutes 19 seconds East, a distance of 119.69 feet to a nail found; thence North 17 degrees 57 minutes 53 seconds East, a distance of 79.90 feet to an X mark found, said point being the True Point of Beginning.

Said tract of land contains 1.047 Acres.



TITLE EXCEPTIONS

THE FOLLOWING EXCEPTIONS ARE LISTED IN SCHEDULE B, SECTION 2, OF A COMMITMENT FOR TITLE INSURANCE, AS PREPARED BY FIDELITY NATIONAL TITLE INSURANCE COMPANY, COMMITMENT NO. 201819GA, EFFECTIVE DATE OCTOBER 26, 2020. 13. Easements from T.C. Ho1ms to Georgia Power Company as follows:

a. Dated January 4, 1953, filed August 4, 1953, Recorded in Deed Book 985, Page 486, MAY AFFECT SITE — BLANKET EASEMENT WITH VAGUE DESCRIPTION — NOT PLOTTABLE b. Dated December 3, 1954, filed January 4, 1955, Recorded in Deed Book 1080, Page

398 aforesaid records AFFECTS SITE - BLANKET EASEMENT - NOT PLOTTABLE

14. Right of Way Deed from Beech-Nut, Inc. to DeKalb County, dated July 28, 1969, filed August 6, 1969, Recorded in Deed Book 2449, Page 324, aforesaid records. AFFECTŠ SITE — BLANKET EASEMENT — NOT PLOTTABLĚ

15. Right of Way Easement from City Ice Delivery Company to Georgia Power Company, dated October 13, 1969, filed November 13, 1969, Recorded in Deed Book 2481, Page AFFECTS SITE - BLANKET EASEMENT - NOT PLOTTABLE

16. Right of Way Easement from Dobbs Houses, Division of Beech-Nut, Inc. to Georgia Power Company, dated September 20, 1969, filed November 13, 1969, Recorded in Deed

Book 2481, Page 89, aforesaid records. AFFECTS 3809 NORTH DRUID HILLS ROAD — BLANKET EASEMENT — NOT PLOTTABLE 17. Right of Way Easement from Dobbs Houses, Inc. to Georgia Power Company, dated December 9, 1969, filed January 30, 1970, Recorded in Deed Book 2501, Page 365, AFFECTS 3809 NORTH DRUID HILLS ROAD — BLANKET EASEMENT — NOT PLOTTABLE

18. Stormwater Detention Facility Inspection and Maintenance Agreement by and between Hendon Columbia, LLC and DeKalb County, Georgia, executed December 17, 2010, filed December 21, 2010, Recorded in Deed Book 22282, Page 388, aforesaid records. AFFECTS SURVEY TRACT TWO - BLANKET EASEMENTS & RESTRICTIONS - NOT

19. All matters affecting subject property as shown on the following plats, all aforesaid

records: (A) Plat Book 24, Page 14 (Fee Simple); NÓ EASEMENTS AFFECTING SITE DEPICTED ON DOCUMENT

(B) Plat Book 219, Page 113 (Easement Parcel 1); NO EASEMENTS AFFECTING SITE DEPICTED ON DOCUMENT

(C) Plat Book 221, Page 52 (Easement Parcel 2). NÓ EASEMENTS AFFECTING SITE DEPICTED ON DÓCUMENT

As to Fee Parcel: 20. Memorandum of Lease by and between Citizens and Southern Trust Company (Georgia), N.A., as Trustees, et al, Romar Joint Venture and Pier 1 Imports (U.S.), Inc., a Delaware corporation, dated May 7, 1990, filed June 18, ____ and recorded in Deed Book 6727, Page 256, aforesaid records.

NOT A SURVEY MATTER 21. Easements as contained in that certain Right of Way Deed from O.S. Cofer to DeKalb County, Georgia, dated July 5, 1944, filed August 31, 1944 and recorded in Deed Book 606. Page 33. aforesaid records.

AFFECT SITE - BLANKET EASEMENT FOR DRAINAGE - NOT PLOTTABLE 22. Easements as contained in that certain Right of Way Deed from Robert C. Crim to DeKalb County, a political subdivision of the State of Georgia, dated October ____, 1988, filed October 18, 1988 and recorded in Deed Book 6270, Page 79, aforesaid records. DOES NOT AFFECT SITE As to Easement Parcel 2:

23. Assignment and Assumption of Declaration of Easements, Covenants and Restrictions among Hendon Columbia, LLC, and LCI-SVAP NDM JV, LP, a Delaware limited liability company, dated May 9, 2014, filed May 15, 2014 and recorded in Deed Book 24380, Page 247, aforesaid records, as it affects that certain Declaration of Easements, Covenants and Restrictions by Hendon Columbia, LLC, a Georgia limited liability company, dated October 31, 2011, filed November 7, 2011 and recorded in Deed Book 22714, Page 748, aforesaid records. AFFECTS SITE AS SHOWN

UTILITY NOTE

OVERALL PROPERTY DESCRIPTION

Said tract of land contains 2.485 Acres.

degrees 14 minutes 08 seconds East, a distance of 86.03 feet to a 5/8

inch rebar found; thence North 17 degrees 25 minutes 03 seconds East, a

distance of 109.86 feet to a 1/2 inch rebar found; thence North 18 degrees 08 minutes 19 seconds East, a distance of 119.69 feet to a nail

found; thence North 17 degrees 57 minutes 53 seconds East, a distance of

79.90 feet to an X mark found, said point being the True Point of

THE UNDERGROUND UTILITIES SHOWN HEREON ARE BASED ON LOCATION OF MARKINGS PROVIDED BY:

SUBSURFACE UTILITY INVESTIGATIONS, LLC 898 SWEET BRIAR TRAIL CONYERS, GEORGIA 30094 (770) 557-4142

SUBSURFACE UTILITY INVESTIGATIONS, LLC UTILIZED SEVERAL TECHNIQUES INCLUDING, BUT NOT LIMITED TO, ELECTROMAGNETIC, MAGNETIC LOCATION FOR FERROUS METALS, ACOUSTIC AND PASSIVE FREQUENCIES TO DESIGNATE AND MARK BURIED UTILITIES ON THE SURFACE WITH PAINT AND FLAGS COVERING THE ENTIRE AREA OF THE PROJECT.

THE SURVEYOR MAKES NO GUARANTEES THAT THE UNDERGROUND UTILITIES SHOWN COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN-SERVICE OR ABANDONED. UNDERGROUND UTILITIES NOT OBSERVED OR LOCATED UTILIZING THIS TECHNIQUE MAY EXIST ON THIS SITE BUT NOT BE SHOWN, AND MAY BE FOUND UPON EXCAVATION. THE SURVEYOR FURTHER DOES NOT WARRANT THAT THE UNDERGROUND UTILITIES SHOWN ARE IN THE EXACT LOCATION INDICATED ALTHOUGH THE SURVEYOR DOES CERTIFY THAT THEY ARE LOCATED AS ACCURATELY AS POSSIBLE FROM INFORMATION AVAILABLE.

INFORMATION REGARDING MATERIAL AND SIZE OF UTILITIES IS BASED ON RECORDS ACQUIRED FROM THE UTILITY OWNERS.

PROPERTY DESCRIPTIONS

TRACT ONE PROPERTY DESCRIPTION

right-of-way of North Druid Hills Road (100 foot right-of-way); thence right-of-way); thence right-of-way); thence right-of-way); thence along said right—of—way of North Druid Hills Road South 70 degrees 28 along said right—of—way of North Druid Hills Road South 17 degrees 57 minutes 53 minutes 40 seconds East, a distance of 233.65 feet to a 5/8 inch rebar minutes 40 seconds East, a distance of 233.65 feet to a 5/8 inch rebar seconds West, a distance of 79.90 feet to a nail found; thence South 18 degrees 24 minutes 15 seconds East, a distance of 160.12 feet to a 5/8 right—of—way of Birch Road; thence along said right—of—way North 18 inch rebar found; thence South 71 degrees 24 minutes 15 seconds East, a inch rebar found; thence South 11 degrees 56 minutes 08 seconds West, a degrees 08 minutes 19 seconds East, a distance of 119.69 feet to a nail distance of 160.12 feet to a 5/8 inch rebar found; thence South 11

distance of 400.37 feet to a 1/2 inch rebar found on the Easterly right—of—way of Birch Road; thence along said right—of—way North 09 Said tract of land contains 1.047 Acres.

TRACT TWO PROPERTY DESCRIPTION All that tract or parcel of land lying or being in Land Lot 100, 18th All that tract or parcel of land lying or being in Land Lot 100, 18th District, Dekalb County, Georgia, and being more particularly described as District, Dekalb County, Georgia, and being more particularly described as

Beginning at an X mark found at the intersection of the Easterly Beginning at an X mark found at the intersection of the Easterly right-of-way of Birch Road (60 foot right-of-way) with the Southerly right-of-way of Birch Road (60 foot right-of-way) with the Southerly set; thence leaving said right—of—way South 19 degrees 08 minutes 30 set; thence leaving said right—of—way South 19 degrees 08 minutes 30 degrees 08 minutes 19 seconds West, a distance of 119.69 feet to a 1/2 seconds West, a distance of 140.15 feet to a 5/8 inch rebar set; thence seconds West, a distance of 140.15 feet to a 5/8 inch rebar set; thence inch rebar found, said point being the True Point of Beginning; thence South 12 degrees 24 minutes 24 seconds West, a distance of 52.60 feet to South 12 degrees 24 minutes 24 seconds West, a distance of 52.60 feet to leaving said right—of—way South 72 degrees 13 minutes 06 seconds East, a a 1/2 inch rebar found; thence South 12 degrees 22 minutes 11 seconds a 1/2 inch rebar found; thence North 72 degrees 13 minutes 06 seconds distance of 236.14 feet to a 1/2 inch rebar found; thence South 12 West, a distance of 30.67 feet to a 5/8 inch rebar found; thence South 71 West, a distance of 236.14 feet to a 1/2 inch rebar found on the Easterly degrees 22 minutes 11 seconds West, a distance of 30.67 feet to a 5/8 distance of 109.33 feet to a 1/2 inch rebar found on the Northerly found; thence North 17 degrees 57 minutes 53 seconds East, a distance of degrees 56 minutes 08 seconds West, a distance of 109.33 feet to a 1/2 right—of—way of Sweet Briar Road (variable right—of—way); thence along said right—of—way North 79 degrees 52 minutes 39 seconds West, a distance of 109.33 feet to a 1/2 (variable right—of—way); thence along said right—of—way North 79 degrees 52 minutes 39 seconds West, a Beginning. 52 minutes 39 seconds West, a distance of 400.37 feet to a 1/2 inch rebar found on the Easterly right-of-way of Birch Road; thence along said right-of-way North 09 degrees 14 minutes 08 seconds East, a distance of 86.03 feet to a 5/8 inch rebar found; thence North 17 degrees 25 minutes 03 seconds East, a distance of 109.86 feet to a 1/2 inch rebar found, said point being the True Point of Beginning.

Said tract of land contains 1.438 Acres.

VICINITY MAP SITE LOCATION — LATITUDE: 33° 48′ 42" LONGITUDE: 84° 16′ 43"

GENERAL NOTES

THIS SURVEY HAS BEEN PREPARED FOR THE EXCLUSIVE USE OF THE PERSON OR ENTITIES NAMED HEREON. NO EXPRESS OR IMPLIED WARRANTIES WITH RESPECT TO THE INFORMATION SHOWN HEREON IS TO BE EXTENDED TO ANY PERSONS OR ENTITIES OTHER THAN THOSE SHOWN HEREON.

REVISIONS LISTED ON THIS SURVEY APPLY ONLY TO THE SPECIFIC CHANGES REFERENCED, AND DO NOT CONSTITUTE AN UPDATE OF OTHER DATA ON THIS SURVEY. THE "SURVEY DATE" SHOWN HEREON IS THE APPLICABLE DATE AS RELATED TO PROVISIONS OF STATUTES OF LIMITATION UNLESS SPECIFICALLY NOTED OTHERWISE.

THIS PROPERTY IS NOT LOCATED IN A SPECIAL FLOOD HAZARD AREA BASED ON THE FLOOD INSURANCE RATE MAP FOR THIS AREA. THE MAP NUMBER FOR THIS AREA IS 13089C0067K. AND THE DATE OF SAID MAP IS AUGUST 15. 2019. THIS DETERMINATION WAS MADE BY GRAPHICALLY DETERMINING THE POSITION OF THIS SITE ON SAID FIRM MAPS UNLESS OTHERWISE NOTED.

PLEASE NOTE: TREES 4-INCH DBH (DIAMETER AT BREAST HEIGHT) AND LARGER WERE LOCATED FOR THIS SURVEY.

RIGHT-OF-WAY LINES SHOWN ON THIS SURVEY THAT ARE NOT ACTUAL BOUNDARIES OF THE SUBJECT TRACT(S) ARE DEPICTED GRAPHICALLY AND ARE SHOWN APPROXIMATELY FOR INFORMATIONAL PURPOSES ONLY. SAID RIGHT-OF-WAY LINES SHOULD NOT BE UTILIZED FOR DESIGN PURPOSES

THE DATUM FOR THIS SITE WAS ESTABLISHED UTILIZING GLOBAL POSITIONING SYSTEMS, AND BASED ON POSITIONAL VALUES FOR THE VIRTUAL REFERENCE STATION NETWORK DEVELOPED BY eGPS SOLUTIONS. THE HORIZONTAL REFERENCE FRAME IS NORTH AMERICAN DATUM OF 1983(2011)-STATE PLANE COORDINATE SYSTEM OF GEORGIA-WEST ZONE. THE VERTICAL REFERENCE FRAME IS NORTH AMERICAN VERTICAL DATUM OF 1988. ANY DIRECTIONS OR DIMENSIONS SHOWN ARE A RECTANGULAR, GROUND LEVEL PROJECTION OF THE STATE PLANE COORDINATE SYSTEM.

NO EVIDENCE OF THE SITE BEING USED AS A SOLID WASTE DUMP, SUMP OR SANITARY LANDFILL WAS OBSERVED DURING THE TIME FIELD WORK WAS PERFORMED ON THE SUBJECT PARCEL

ALL MATTERS SHOWN ON RECORDED PLATS PROVIDED TO THE SURVEYOR ARE SHOWN ON THE SURVEY

AT THE DATE OF THIS SURVEY, THE SUBJECT PROPERTY HAD ACCESS TO AND FROM A DULY DEDICATED PUBLIC RIGHT-OF-WAY, NORTH DRUID HILLS ROAD AND BIRCH ROAD. THE SURVEYOR MAKES NO CERTIFICATION AS TO WHETHER OR NOT THESE ACCESS POINTS HAVE BEEN APPROVED OR PERMITTED BY JURISDICTIONAL AUTHORITIES.

THE SUBJECT PROPERTY IS CONTIGUOUS WITH ADJACENT RIGHT-OF-WAYS WITHOUT GAPS, GORES, OR OVERLAPS.

NO VISIBLE ABOVE-GROUND ENCROACHMENTS WERE OBSERVED AT THE DATE

ALTA CERTIFICATION TABLE "A" NOTES: Item 16 - No evidence of current earth moving work, building construction or building additions was observed at the date of survey. Item 17 - No evidence was observed indicating changes in street right of way lines either completed or proposed.

SURVEY REFERENCES

1> ALTA/NSPS LAND TITLE SURVEY OF NORTH DEKALB MALL, PREPARED BY GEOSURVEY, LTD., DATED DECEMBER 15, 2021. (JOB NO. 20175643)

CLOSURE STATEMENT

THE FIELD CLOSURE UPON WHICH THIS PLAT IS BASED HAS A CLOSURE PRECISION OF ONE FOOT IN $\underline{149,225}$, AND WAS ADJUSTED USING THE LEAST SQUARES METHOD. A TRIMBLE S-6 TOTAL STATION AND TRIMBLE TSC-3 DATA COLLECTOR WERE USED TO COLLECT THIS

IF YOU DIG

THIS PLAT HAS BEEN CALCULATED FOR CLOSURE AND WAS FOUND TO BE ACCURATE



WITHIN ONE FOOT IN 699,297 FEET. JTN INIT.

(CLERK OF COURT RECORDING INFORMATION)

Knowwhat's below. before you dig. **Dial 811** Or Call 800-282-7411

GRAPHIC SCALE 1"= 30'

N/F PROPERTY OF MIDTOWN NATIONAL GROUP, LP DEED BOOK 225963 / PAGE 770 DEED BOOK 26135 / PAGE 636 DEED BOOK 26453 / PAGE 394 TRACT ONE

> 45,590 sf ZONED NS LCI-SVAP NDM JV, LP

DEED BOOK 24380 / PAGE 239 DEED BOOK 24380 / PAGE 244

1.047 Acres

TRACT TWO 1.438 Acres 62,662 sf ZONED C-1

TOTAL SITE AREA 2.485 Acres 108,252 sf

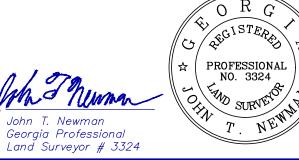


Land Surveying • 3D Laser Scanning 1660 Barnes Mill Road Marietta, Georgia 30062 (770) 795-9900 Phone:

(770) 795-8880www.geosurvey.com EMAIL: info@geosurvey.com

Certificate of Authorization #LSF-000621





ALTA/NSPS LAND TITLE SURVEY

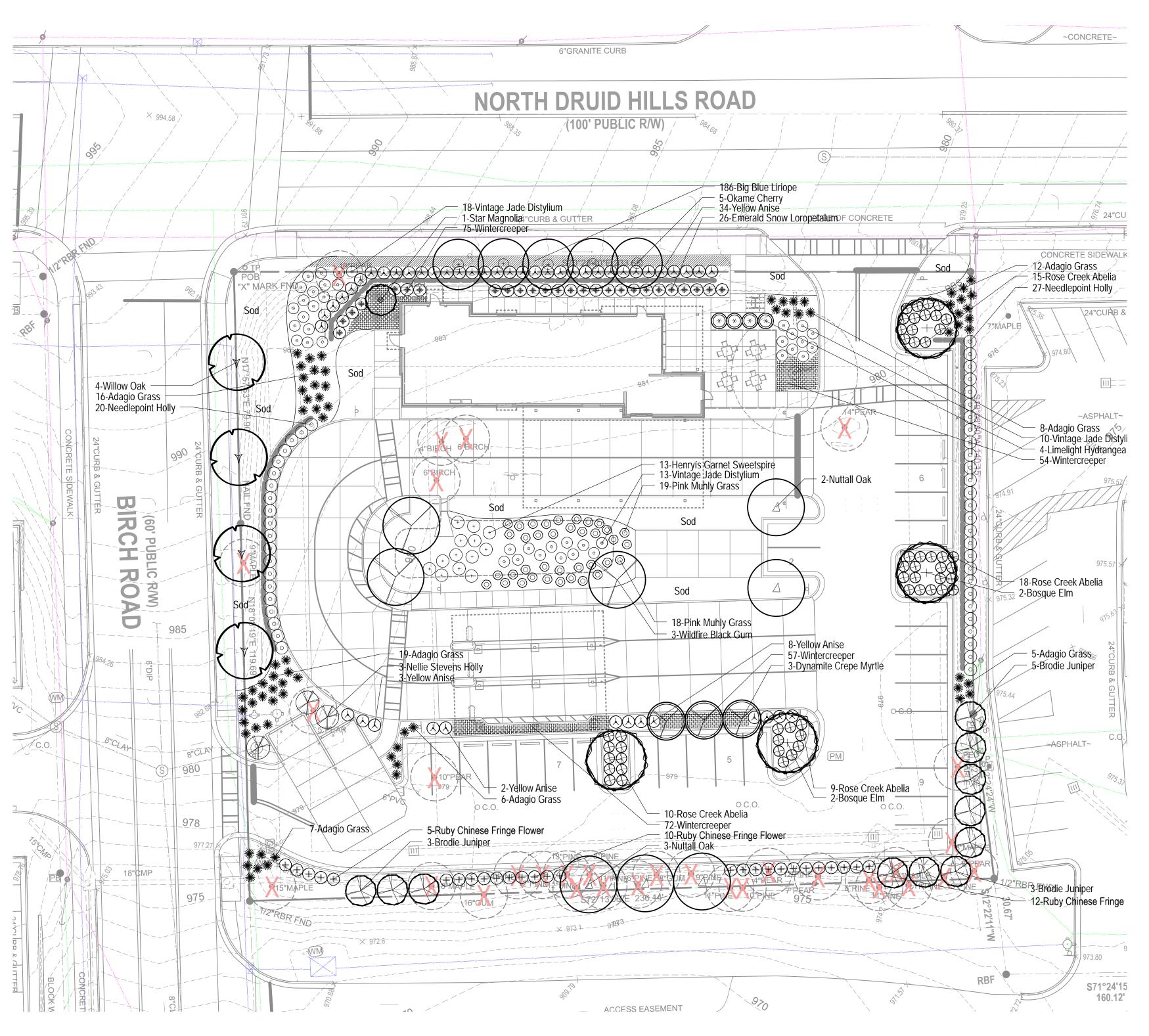
3795 North Druid Hills Rd & 2692 Sweet Briar Rd

FOR

Chick-fil-A, Inc. Fidelity National Title Insurance Company

GS JOB NO:	20175643	DRAWING SCALE:	1 "=	<i>30'</i>	SURVEY DATE:		MARCH 2, 2021				
FIELD WORK:	ZM	CITY: UNINCORPO	RATED	No	REV Date	ISIONS ((SEE GENERAL NOTES)				
PROJ MGR:	JTN	COUNTY: <i>DEKALB</i>	STATE:	GA		3-12-21	Add zoni				
REVIEWED:	JRC	LAND LOT: 100			F						
DWG FILE: 20175	643-02.dwg	DISTRICT: 18TH									

EXHIBIT E







LEGEND



EXISTING TREE TO BE REMOVED



EXISTING TREE TO REMAIN



TREE PROTECTION FENCING

LANDSCAPE NOTES

SOUTHEAS

- Landscape Contractor to read and understand the Landscape Specifications (sheet L-102) prior to finalizing
- bids. The Landscape Specifications shall be adhered to throughout the construction process.Contractor is responsible for locating and protecting all underground utilities prior to digging.
- Contractor is responsible for protecting existing trees from damage during construction.
 All tree protection devices to be installed prior to the start of land disturbance, and maintained until final
- landscaping.All tree protection areas to be protected from sedimentation.
- 6. All tree protection fencing to be inspected daily, and repaired or replaced as needed.
- 7. No parking, storage or other construction activities are to occur within tree protection areas.8. All planting areas shall be cleaned of construction debris (ie. concrete, rock, rubble, building materials, etc) prior
- to adding and spreading of the topsoil.

 9. General Contractor is responsible for adding a min of 4" clean friable topsoil in all planting beds and all grassed areas. Graded areas to be held down the appropriate elevation to account for topsoil depth. See Landscape
- Specifications for required topsoil characteristics.

 10. In all parking lot islands, the General Contractor is responsible to remove all debris, fracture/loosen subgrade to a min. 24" depth. Add topsoil to a 6"-8" bermed height above island curbing; refer to landscape specifications
- 11. Prior to beginning work, the Landscape Contractor shall inspect the subgrade, general site conditions, verify elevations, utility locations, irrigation, approve topsoil provided by the General Contractor and observe the site conditions under which the work is to be done. Notify the General Contractor of any unsatisfactory conditions, work shall not proceed until such conditions have been corrected and are acceptable to the Landscape
- Contractor.

 12. Any deviations from the approved set of plans are to be approved by the Landscape Architect.
- 13. Landscaping shall be installed in conformance with ANSI Z60.1 the "American Standard for Nursery Stock" and the accepted standards of the American Association of Nurserymen.
- 14. Existing grass in proposed planting areas shall be killed and removed. Hand rake to remove all rocks and debris larger than 1 inch in diameter, prior to adding topsoil and planting shrubs.

- 15. Soil to be tested to determine fertilizer and lime requirements prior to laying sod.16. Annual and perennial beds: add min. 4 inch layer of organic material and till to a min. depth of 12 inches. Mulch
- annual and perennial beds with 2-3 inch depth of mini nuggets.
- 17. All shrubs beds (existing and new) to be mulched with a min. 3 inch layer of mulch (double shredded hardwood
- 18. Planting holes to be dug a minimum of twice the width of the root ball, for both shrub and tree. Set plant
- material 2-3" above finish grade. Backfill planting pit with topsoil and native excavated soil.

 19. Sod to be delivered fresh (Cut less than 24 hours prior to arriving on site), laid immediately, rolled, and watered thoroughly immediately after planting. Edge of sod at planting beds are to be "V" trenched; see Landscape
- 20. Any existing grass disturbed during construction to be fully removed, regraded and replaced. All tire marks and
- indentions to be repaired.

 21. Water thoroughly twice in first 24 hours and apply mulch immediately.
- 22. The Landscape Contractor shall guarantee all plants installed for one full year from date of acceptance by the owner. All plants shall be alive and at a vigorous rate of growth at the end of the guarantee period. The Landscape Contractor shall not be responsible for acts of God or vandalism. See Landscape Specifications for Warranty requirements/expectations.
- 23. Any plant that is determined dead, in an unhealthy, unsightly condition, lost its shape due to dead branches, or other symptoms of poor, non-vigorous growth, shall be replaced by the Landscape Contractor. See Landscape Specifications for warranty requirements/expectations.
- 24. Site to be 100% irrigated in all planting beds and grass area by an automatic underground Irrigation System. See Irrigation Plan L-200 for design. Irrigation as-built shall be provided to the Landscape Architect within 24 hours of irrigation install completion.
- 25. Stake all evergreen and deciduous trees as shown in the planting detail and as per the Landscape
- 26. Remove stakes and guying from all trees after one year from planting.

PLANT LIST

Qty	Botanical Name	Common Name	Scheduled Size	Remarks
	Trees			
3	Ilex x Nellie R Stevens	Nellie Stevens Holly	5'-6' Hgt; B&B	
11	Juniperus virginiana 'Brodie'	Brodie Juniper	8'-10' Hgt.	
3	Lagerstroemia indica 'Whit II'	Dynamite Crepe Myrtle	7'-8' Hgt.	
1	Magnolia stellata	Star Magnolia	6'-8' Hgt.	
3	Nyssa sylvatica 'Wildfire'	Wildfire Black Gum	3" Cal; 10' Hgt.	B & B; single straight leader
5	Prunus 'Okame'	Okame Cherry	2" Cal.; 10' Hgt. Minimum	
5	Quercus nuttallii	Nuttall Oak	3" Cal; 10' Hgt.	B & B; single straight leader
4	Quercus phellos	Willow Oak	3" Cal; 10' Hgt.	B & B; single straight leader
4	Ulmus parvifolia 'Bosque'	Bosque Elm	3" Cal; 10' Hgt.	B & B
	Shrubs			
52	Abelia x chinensis 'Rose Creek'	Rose Creek Abelia	3 Gal.	
41	Distylium 'Vintage Jade'	Vintage Jade Distylium	3 Gal.	
4	Hydrangea paniculata 'Limelight'	Limelight Hydrangea	3 Gal.	
47	Ilex cornuta 'Needlepoint'	Needlepoint Holly	3 Gal.	
47	Illicum parviflorum	Yellow Anise	3 Gal.	
13	Itea virginica 'Henry's Garnet'	Henryís Garnet Sweetspire	3 Gal.	
27	Loropetalum chinense 'Ruby'	Ruby Loropetalum	3 Gal 24"-36" Hgt.	
26	Loropetalum chinense 'Shang White'	Emerald Snow Loropetalum	3 Gal.	
73	Miscanthus sinensis 'Adagio'	Adagio Grass	3 Gal.	
37	Muhlenbergia capillaris	Pink Muhly Grass	3 Gal.	
	Groundcovers			
258	Euonymus coloratus	Wintercreeper	1 Gal.	Plant 18" O.C.
186	Liriope muscari 'Big Blue'	Big Blue Liriope	1 Gal.	Plant 18" O.C.
	Other			

NOTE:

This Lanscape Plan is Preliminary in nature and is subject to change





Manley Land Design, Inc. 51 Old Canton Street Alpharetta, Georgia 30009

manleylanddesign.com

FIL-A ID HILLS DTO

FSU# 4846

DEVISION SCHEDULE

REVISION SCHEDULE

NO. DATE DESCRIPTION

MLD PROJECT# 2021062
PRINTED FOR

DATE 4.9.21

DRAWN BY MB

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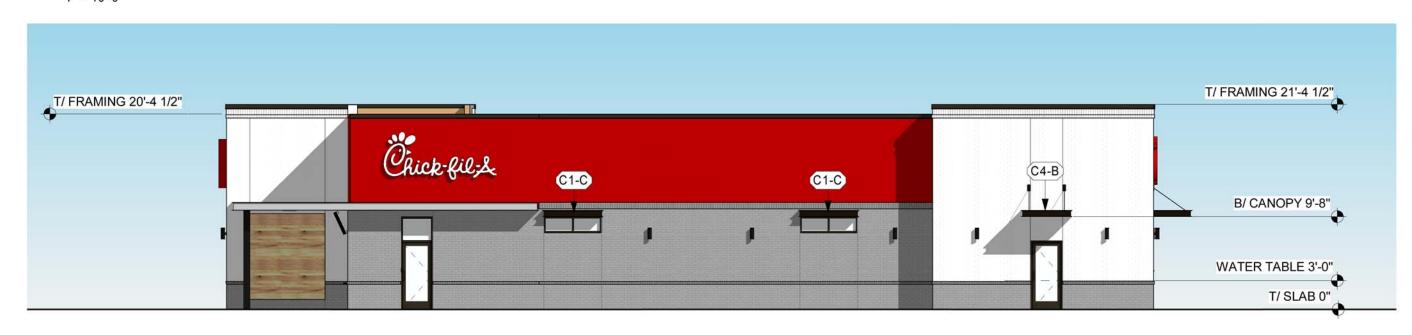
Landscape Plan

L-100

EXHIBIT F



EXTERIOR ELEVATION
1" = 10'-0"



EXTERIOR ELEVATION
1" = 10'-0"



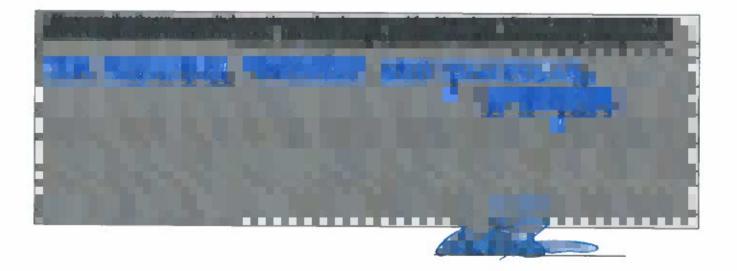
EXTERIOR ELEVATION
1" = 10'-0"



EXTERIOR ELEVATION
1" = 10'-0"



= 4



Zoning Comments

N1 & N2 (Z-21-1244885 & SLUP-21-1244886) - North Druid Hills is classified as a major arterial. Required to add a left turn lane onto Birch. Drive on North Druid Hills limited to Right in/right out only. Please see chapter 5 of the zoning code and chapter 14-190 of the land development code for infrastructure requirements. Requires 10 foot landscape strip, bike lanes and 6 foot sidewalks or 10 foot multiuse path (preferred), street lighting on back of sidewalk. Right of way dedication of 50 feet from centerline or such that all public infrastructure is within county right of way, whichever greater. Birch Road is classified as a local street. Requires a 27.5 foot right of way dedication from the centerline or such that all public infrastructure is on county right of way. Due to the proximity of the mall, potential for redevelopment and connectivity the residential areas- a 10 foot multiuse trail to be included in the sight design along Birch. Requires a 6 foot landscape strip. Streetlights required on back of path.

N3. (Z-21-1244892) Flakes Mill Road is classified as a minor arterial. Access point on Flakes Mill Road must meet intersection sight distance prior to permitting and verified prior to occupancy by the engineer of record. Please see chapter 5 of the zoning code and chapter 14-190 of the land development code for infrastructure requirements. Requires 10 foot landscape strip, bike lanes and 6 foot sidewalks or 10 foot multiuse path (preferred), street lighting on back of sidewalk. Right of way dedication of 40 feet from centerline or such that all public infrastructure is within county right of way, whichever greater. New residential streets will be local with a right of way of 55 feet, 24 feet of pavement, curb and gutter, 6 foot landscape strip, a 6 foot sidewalk, street lighting required behind sidewalk.

N4. (Z-21-1244893) Norman Road is classified as a collector road. Please see chapter 5 of the zoning code and chapter 14-190 of the land development code for infrastructure requirements. Requires 10 foot landscape strip, bike lanes and 6 foot sidewalks or 10 foot multiuse path (preferred), street lighting on back of sidewalk. Right of way dedication of 35 feet from centerline or such that all public infrastructure is within county right of way, whichever greater. New residential streets will be local with a right of way of 55 feet, 24 feet of pavement, curb and gutter, 6 foot landscape strip, a 6 foot sidewalk, street lighting required on back of sidewalk. Continue at least 2 traffic calming features (splitter islands) similar to the ones installed in the City of Clarkton along frontage.

N5. (SLUP-21-1244895) No comment

N6. (SLUP-21-1244899) No comment

DEKALB COUNTY

Board of Health

06/21/2021

To: Mr. John Reid, Senior Planner

From: Ryan Cira, Environmental Health Manager Cc: Alan Gaines, Technical Services Manager

Re: Rezone Application Review

General Comments:

DeKalb County Health Regulations prohibit use of on-site sewage disposal systems for

- multiple dwellings
- · food service establishments
- · hotels and motels
- commercial laundries
- funeral homes
- schools
- · nursing care facilities
- personal care homes with more than six (6) clients
- child or adult day care facilities with more than six (6) clients
- residential facilities containing food service establishments

If proposal will use on-site sewage disposal, please contact the Land Use Section (404) 508-7900.

Any proposal, which will alter wastewater flow to an on-site sewage disposal system, must be reviewed by this office prior to construction.

This office must approve any proposed food service operation or swimming pool prior to starting construction.

Public health recommends the inclusion of sidewalks to continue a preexisting sidewalk network or begin a new sidewalk network. Sidewalks can provide safe and convenient pedestrian access to a community-oriented facility and access to adjacent facilities and neighborhoods.

For a public transportation route, there shall be a 5ft. sidewalk with a buffer between the sidewalk and the road. There shall be enough space next to sidewalk for bus shelter's concrete pad installation. Recommendation: Provide trash can with liner at each bus stop with bench and monitor for proper removal of waste.

Since DeKalb County is classified as a Zone 1 radon county, this office recommends the use of radon resistant construction.

Board of Health

N.1 Z-21-1244885 2021-2678/18-100-04-019

District 02 Super District 06

3795 North Druid Hills Road, Decatur, GA 30033

Acres: 1.04

- Please review general comments.
- NS to C-1
- N.2 SLUP-21-1244886 2021-2638/18-100-04-019

District 02 Super District 06

3795 North Druid Hills Road, Decatur, GA 30033

Acres: 1.04

- Please review general comments.
- SLUP to allow drive through facility through in Town Center Character area.
- N.3 Z-21-1244892 2021-2640/12-253-03-002

District 03 Super District 07

5035 Flakes Mills Road, Ellenwood, GA 30294

Acres: 27

- Septic system installed on several surrounding properties in the past.
- Please review general comments.
- R-100 to R-60
- N.4 Z-21-1244893 2021-2641/18-095-03-005, 18-095-03-006, 18-095-03-008, 18-095-03-009, 18-

095-03-090, 18-095-03-094

District 04 Super District 06

3943 Norman Road, Stone Mountain, GA 30083

Acres: 35

- Septic system installed on several surrounding properties in the past.
- Please review general comments.
- R-85 to RSM
- N.5 SLUP-21-1244895 2021-2641/15-137-03-028

District 03 Super Districts 06

2445 Candler Road, Decatur, GA 30032

Acres: 0.3

- Dental Building at time septic installed on 12/4/1962.
- Please review general comments.
- SLUP to Housing Facility

DeKalb County Board of Health

445 Winn Way – Box 987 Decatur, GA 30031

404.294.3700 • www.dekalbhealth.net

DEKALB COUNTY

Board of Health

- N.6 SLUP-21-1244899 2021/2643/15-084-05-068 District 03 Super District 06 3008 Rollingwood Lane, Atlanta, GA 30316 Acres: 0.35
 - Septic system installed 8/19/1960.
 - Please review general comments.
- N.7 TA-21-1244945 2021-2644
 Districts 03 & 05 Super District 07
 Please review general comments.
 - TA-21-1244999 2021-2645 Districts 03 & 05 Super District 07
 - Please review general comments.

N.8





DEKALB COUNTY GOVERNMENT PLANNING DEPARTMENT DISTRIBUTION FORM

NOTE: PLEASE RETURN ALL COMMENTS VIA EMAIL OR FAX TO EXPEDITE THE PROCESS TO MICHELLE ALEXANDER mmalexander@dekalbcountvga.gov AND/OR LASONDRA HILL lahill@dekalbcountvga.gov

COMMENTS FORM: PUBLIC WORKS TRAFFIC ENGINEERING

		11 021 1222111 0
Case No.: Z-21-1	24488 Parcel I.D. #: /	8-100-04-019
Address: 3795	_	,
	und Hills Rd	
N 5 mm	6 -	
D. CATO	900	
	Adjacent	Roadway (s):
	(classification)	(classification)
	Capacity (TPD)	Capacity (TPD)
	Latest Count (TPD) Hourly Capacity (VPH)	Latest Count (TPD) Hourly Capacity (VPH)
	Peak Hour, Volume (VPH)	Peak Hour. Volume (VPH)
	Existing number of traffic lanes	Existing number of traffic lanes
	Existing right of way width Proposed number of traffic lanes	
	Proposed right of way width	Proposed right of way width
Please provide addition:	al information relating to the following st	atement.
generate an average of factor. Based on the ab with approximately	ifteen (15) vehicle trip end (VTE) per 1, (ove formula, thesquare foot plac peak hour vehicle trip ends. , on the other hand, would generate ten (eers (ITE) 6/7 TH Edition (whichever is applicable), churches 000 square feet of floor area, with an eight (8%) percent peak here of worship building would generate
a maximum ofunit	s per acres, and the given fact that the pi	(Single Family Residential) District designation which allo roject site is approximatelyacres in land area,daily nerated with residential development of the parcel.
COMMENTS:		
Field p	INITERLEST TRANS	Fic Flans.





DEKALB COUNTY GOVERNMENT PLANNING DEPARTMENT DISTRIBUTION FORM

NOTE: PLEASE RETURN ALL COMMENTS VIA EMAIL OR FAX TO EXPEDITE THE PROCESS TO MICHELLE ALEXANDER mmalexander@dekalbcountvga.gov AND/OR LASONDRA HILL lahill@dekalbcountvga.gov

COMMENTS FORM: PUBLIC WORKS TRAFFIC ENGINEERING

10001077		, (31, (31, 31, 11, 11, 11, 11, 11, 11, 11, 11,	
Case No.: 5 LUP-21-13	2448 Earcel I.D. #: 18	1-100-04-019	
Address: <u>3795</u>		,	
N. Drund Hil	lc /ld		
DECMUY, GE	}_		
·	Adjacent Ro	padway (s):	
	(classification)	(classification)	
Capacity (1	ГРD)	Capacity (TPD)	
Latest Cou Hourly Car	int (TPD) pacity (VPH)	Latest Count (TPD) Hourly Capacity (VPH)	
Peak Hour.	. Volume (VPH) imber of traffic lanes	Peak Hour. Volume (VPH) Existing number of traffic lanes	
Existing rig	ght of way width	Existing right of way width	
	number of traffic lanes ight of way width	Proposed number of traffic lanes Proposed right of way width	
	85 25 25		
Please provide additional information			
generate an average of fifteen (15) v	chicle trip end (VTE) per 1, 00, thesquare foot place	rs (ITE) <u>6/7TH</u> Edition (whichever is applical 0 square feet of floor area, with an eight (8% of worship building would generate) percent peak hous
peak hour factor. Based on the abo a maximum ofunits per acres,	ve referenced formula, the and the given fact that the pro	VTE's per day per dwelling unit, with a ter (Single Family Residential) District design ject site is approximatelyacres in land a grated with residential development of the particular particular acres in the	nation which allows area,daily
COMMENTS:			
Field And pl	AND RELIEWE	PATTERN/ HOW:	d Shay
		(A.S 1)	
			1.11
		Signature:	LMRoss

NEXT STEPS

Following an approval of this zoning action, one or several of the following may be required:

Land Disturbance Permit (Required for of new building construction on non-residential properties, or land disturbance/improvement such as storm water detention, paving, digging, or landscaping.)

Building Permit (New construction or renovation of a building (interior or exterior) may require full plan submittal or other documentation. zoning, site development, watershed and health department standards will be checked for compliance.)

Certificate of Occupancy (Required prior to occupation of a commercial space and for use of property for any business type. The issuance follows the review of submitted plans if required based on the type occupancy.)

Plat Approval (Required if any parcel is being subdivided, re-parceled, or combined. Issued "administratively"; no public hearing required.)

Sketch Plat Approval (Required for the subdivision of property into three lots or more. Requires a public hearing by the Planning Commission.)

Overlay Review (Required review of development and building plans for all new construction or exterior modification of building(s) located within a designated overlay district.)

Historic Preservation (Certificate of Appropriateness required for any proposed changes to building exteriors or improvements to land when located within the Druid Hills or the Soapstone Geological Historic Districts. Historic Preservation Committee public hearing may be required.)

Variance or Special Exception (Required seeking relief from any development standards of the Zoning Ordinance. A public hearing and action by the Board of Appeals are required for most variances.)

Minor Modification (Required if there are any proposed minor changes to zoning conditions that were approved by the Board of Commissioners. The review is administrative if the changes are determined to be minor as described by Zoning Code.)

Major Modification (Required submittal of a complete zoning application for a public hearing if there are any proposed changes to zoning conditions approved by the Board of Commissioner on a prior rezoning.)

Business License (Required for any business or non-residential enterprise operating in Unincorporated DeKalb County, including in-home occupations).

Alcohol License (Required permit to sell alcohol for consumption on-site or packaged for off-site consumption. Signed and sealed distance survey is required. Background checks will be performed.)

<u>Each of the approvals and permits listed above require submittal of application, fees and supporting documents.</u> Please consult with the appropriate department/division.

From: <u>Valerie Manson</u>

To: Patrick, Robert J.; Rader, Jeff; Johnson, Larry L.; Bradshaw, Stephen R.; Johnson, Mereda D.; Terry, Ted;

Cochran-Johnson, Lorraine; White, Brandon L.; djackson@dekalbsountyga.gov

Subject: Rezoning of 3795 North Druid Hills Road- Chick-Fil-A

Date: Friday, October 22, 2021 10:13:00 AM
Attachments: CFA Final Survey Summary.pdf

To: DeKalb County Board of Zoning

Fr: Valerie Manson

Re: Rezoning of 3795 North Druid Hills Road- rezoning for Chick-Fil-A

Date: October 22, 2021

Dear Planning Department and County Commissioners:

I am writing to express my <u>strong support</u> for the rezoning of the Chick-fil-A at 3795 North Druid Hills Road from Neighborhood Shopping (NS) to Local Commercial (C-1) and the approval of a Special Land Use Permit (SLUP) to allow a drive-through. I live in the area represented by the Laurel Ridge Shamrock Civic Association (LRSCA) and which is most impacted by the current Chick-Fil-A location and the proposed rezoning location.

The LRSCA communication with the Board of Zoning dated September 24, 2021 states that the Board "strongly opposes" the rezoning. Based on the results of their survey (see attached), I do not believe this position reflects the opinion of the respondents. A total of 151 people completed the survey and 43% support the move while 48% oppose it with 9% neutral. I do not think their data reflect "strong opposition" on the part of the residents.

To address some of their other claims:

- 1. Increased traffic: I do not think moving Chick-Fil-a will increase traffic at the entrance to North Druid Woods. It is already backed up to that point now because there are three fast food restaurants further up (McDonald's, Chick-Fil-a and Checkers) and Shamrock Plaza. Actually, I think moving the Chick-Fil-a will spread the entrances and exits out and make the traffic flow more smoothly. Having Zaxby's further down North Druid Hills Road does not present any addition to the congestion in large part because it IS further down the road.
- 2. Installation of a dedicated left turn lane: This has nothing to do specifically with where Chick-Fil-a is located and has everything to do with blocking one of the two lanes when a vehicle is turning left on Birch.

Furthermore, the Chick-Fil-a in the current location along with the other restaurants and Shamrock Plaza create a traffic safety nightmare. I am confident that an accident report would show a high number of accidents due to the high volume of vehicles entering, leaving or just trying to get up or down North Druid Hills Road. I believe moving the Chick-Fil-A out of that immediate vicinity would decrease the number of accidents and create a safer traffic situation.

I ask the Board to please consider the results of the LRSCA survey (not the Board's interpretation), the accident reports of the current configuration and adding a dedicated left turn lane onto Birch Drive in making a decision about rezoning and approving a SLUP for the Chick-Fil-A. I believe the move would make this area safer for residents and those passing through the area.

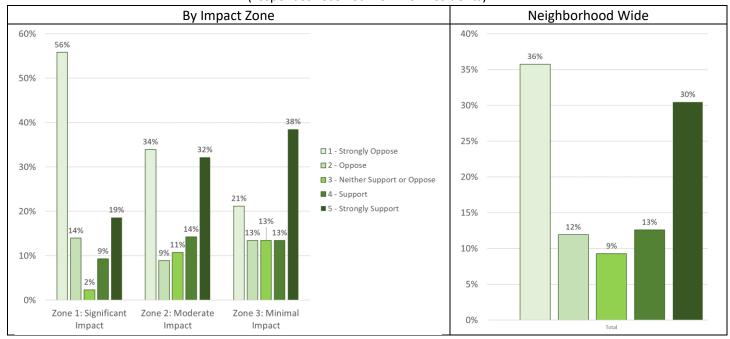
Sincerely,

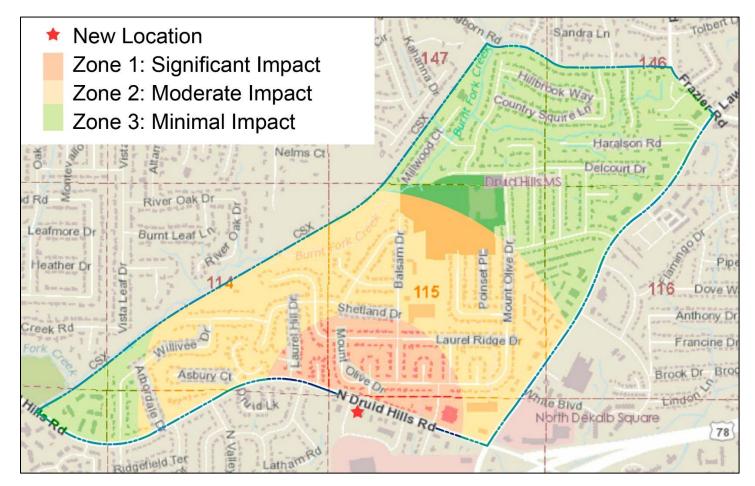
Valerie Manson 1179 Laurel Hill Dr.

CHICK-FIL-A RELOCATION SURVEY RESULTS



Neighborhood Support for Chick-fil-A Proposal (responses received from 151 residents)





OVERALL VOTE	Zone	1	Zon	e 2	Zon	ie 3	Tot	al
Support	12	28%	26	46%	27	52%	65	43%
Neutral	1	2%	6	11%	7	13%	14	9%
Oppose	30	70%	24	43%	18	35%	72	48%
Total	43		56		52		151	
TOP CONCERNS	Zone	1	Zon	e 2	Zon	ie 3	Tot	al
Traffic (Total)	34	79%	31	55%	20	38%	85	56%
- More congestion / traffic back-ups	19		11		7		37	
- More cars on N Druid Hills Rd	14		9		5		28	
- No left turn lane / middle lane	7		9		5		21	
- Car accidents / pedestrian safety	6		10		1		17	
- Congestion at N Druid Woods entrance	6		7		3		16	
- More cut through traffic in neighborhood	6		5		5		16	
Pollution (Total)	10	23%	8	14%	3	6%	21	14%
- Air pollution / car exhaust	9		7		3		19	
- Noise / Light / Etc	2		1		0		3	
Chick-fil-A Business Practices (Total)	5	12%	5	9%	4	8%	14	9%
- Fast food / drive thru	3		3		4		10	
- Moral / religious objections	1		1		0		2	
- Chain / not local	1		0		0		1	
- Already too many CFA's nearby	0		1		0		1	
Those Who Have No Concerns	9	21%	19	34%	27	52%	55	36%
TOP BENEFITS	Zone	2 1	Zon	e 2	Zon	ie 3	Tot	:al
Traffic (Total)	8	19%	18	32%	24	46%	50	33%
- Overall improvement on N Druid Hills Rd	4		7		12		23	
- Safer for pedestrians / fewer car accidents	1		6		13		20	
- Improvement at current CFA location	3		6		4		13	
- Improvement at L'ville Hwy / NDH Rd Intersection	0		6		7		13	
Better Business Location (Total)	8	19%	16	29%	13	25%	37	25%
- More convenient / easier to access	3		6		9		18	
- Bigger store / building	4		3		2		9	
- Bigger lot / more parking / faster drive thru	1		5		3		9	
- Cleaner / newer store / better curb appeal	1		1		3		5	
Pier 1 Store No Longer Abandoned	6	14%	9	16%	5	10%	20	13%
		2274						
Those Who See No Benefits	27	63%	21	38%	15	29%	63	42%
TOP DESIRED IMPROVEMENTS	700	. 1	Zon	- 3	Zor		Tal	
	Zone 12	28%	20n 10	e 2 18%	_	15%	Tot 30	20%
- Add left turn lane / center lane		20%	5	10%	8 7		16	20%
- Add a mall / rear facing entry/exit	3		8		4		15	
- Traffic study / unspecified mitigation measures	7						13	
- No direct entry/exit on N Druid Hills Rd	7		5 3		2		12	
- Make N Druid Hills entry/exit right turn only	0		2		2		4	
Other (Total)	5	12%	7	13%	6	12%	18	12%
		12%	2	15%				1270
- Add indoor seating - Fewer drive thru lanes	0				2		6 5	
- Re-zone old FCA location (no fast food)	1 1		3		0		4	
	17	40%	16	29%	-			200/
Opposed Even With Improvements	1/	40%	10	29%	9	17%	42	28%

From: Kellie Brownlow

To: Patrick, Robert J.; Johnson, Larry L.; Bradshaw, Stephen R.; Johnson, Mereda D.; Cochran-Johnson, Lorraine;

Rader, Jeff; Terry, Ted; White, Brandon L., Jackson, Dustin

Subject: CFA on North Druid Hills

Date: Friday, October 22, 2021 12:32:24 PM
Attachments: CFA Final Survey Summary.pdf

Good afternoon,

I am writing again to voice my support for the relocation of the Chick Fil A to the abandoned Pier 1 building. I know from reading the neighborhood page that there are about a dozen very loud voices in opposition to this project. I understand their opposition based on the proximity of their homes to the site and I am also aware that they oppose basically everything that is proposed to be developed within proximity of their homes.

The current site is unsafe and triggers constant traffic by the entrance. Clearly, the demand for CFA is much higher than the capacity of the current site. The attached pictures were taken on a random Friday and are typical of what you will find during breakfast, lunch, and dinner hours. I have heard opponents talk about traffic at the new site. The new site has the capacity to ensure that what you see in this attached picture does not happen. I have also heard opponents talk about emissions which is ludicrous given that all of us chose to buy a house in a neighborhood that has 2 schools and sits at the intersection of 2 of the busiest corridors in the county.

Please see the attached neighborhood survey. I often hear the opponents say "everyone" in the neighborhood is opposed to the project. This survey makes it clear that this statement is false. Unfortunately, as you know all too well, most of the time, the only people that speak up are the ones that are against projects.

As I mentioned in my earlier email, I have worked in public service most of my career. I know how hard it is to see the forest from the trees when it comes to a very loud minority. I also know that it takes leadership to separate the noise from reason. I am asking each of you to try and focus on the latter and think of what is best in the long term for this neighborhood, especially because the mall has finally been purchased. Yay!

I know a few other neighbors have written to you in support of this project. Ultimately, it will be an improvement to the corridor, the mall development, our neighborhood, and the community at-large.

Thank you so much for your leadership and thoughtfulness on this an all projects about which you have to make decisions.

With gratitude,

Kellie Brownlow 1314 Atterberry Place, Decatur, GA 30033





From: <u>Dallas Ivey</u>

To: Patrick, Robert J.; Johnson, Larry L.; Bradshaw, Stephen R.; Johnson, Mereda D.; Cochran-Johnson, Lorraine;

Rader, Jeff; Terry, Ted; White, Brandon L.; Jackson, Dustin

Subject:CFA Relocation on North Druid Hills RoadDate:Saturday, October 23, 2021 9:49:46 AM

Dear Commissioners,

I am writing in support of the proposed re-location of the Chick Fil A restaurant on North Druid Hills Road. Our family lives in the Pine Glen / North Druid Woods area located on the other side of North Druid Hills Road. The proposed location is better than the current location and has greater capacity to handle the existing flow of traffic and customers. CFA provides a great service to the community and the new location will be better for our area.

Thanks for considering this message and for your work for this community.

Yours truly,

Dallas Ivey

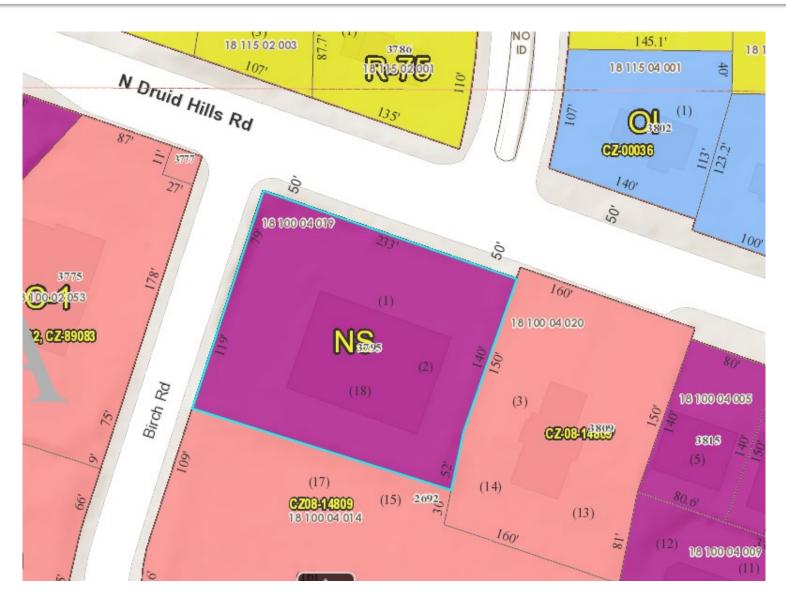
Case Overview

REQUEST:

To rezone property from NS (Neighborhood Shopping) to C-1 (Local Commercial) to allow for the development of a drive-through restaurant.

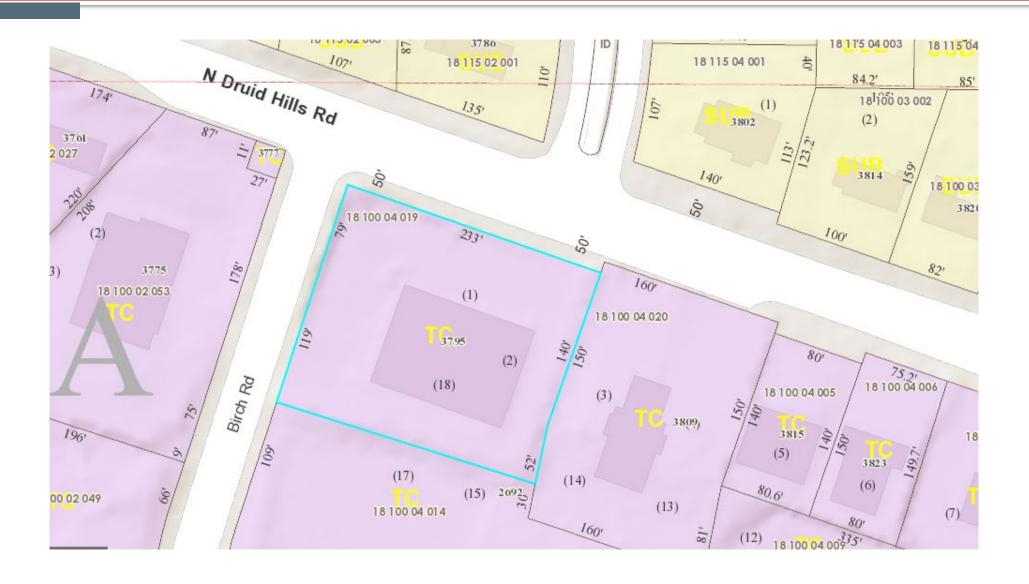
Location: 3795 North Druid Hills Road

Commission District: 2
Super District: 6



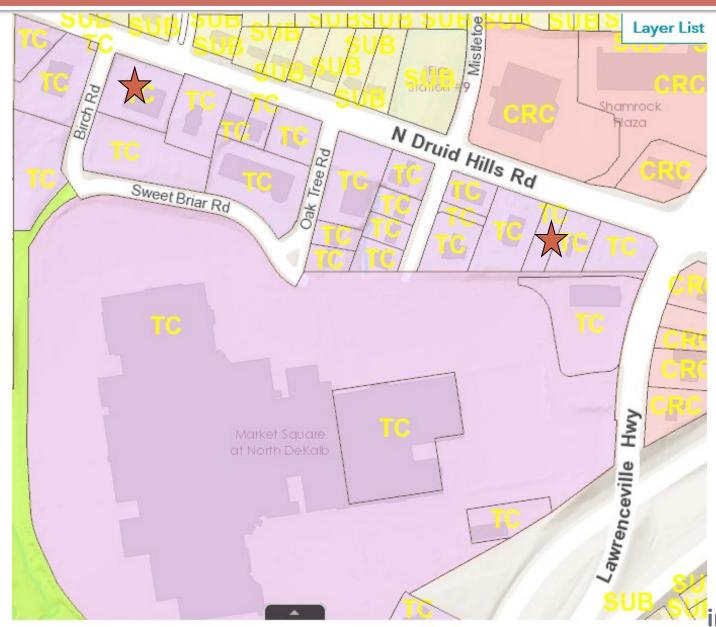
Department of Planning and Sustainability

Future Land Use



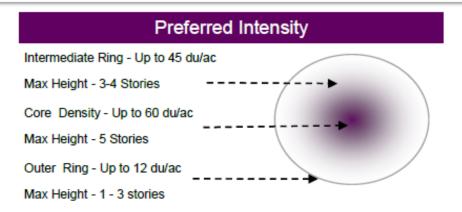
Department of Planning and Sustainability

Future Land Use

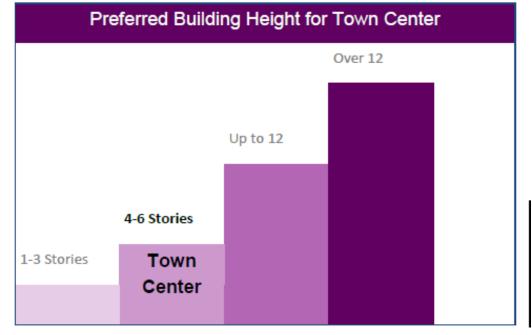


ing and Sustainability

Future Land Use



This is a guideline policy to protect single family housing which is supported by the new zoning code







July 26, 2021

Mrs. Patrece Keeter Dekalb County Public Works Transportation Division 1300 Commerce Drive Decatur, GA 30030

Turn Lane Warrant Memo:

North Druid Hills Road at Birch Road Chick-fil-a Relocation Turn Lane Warrant Analysis - Decatur, GA

Traffic Volumes

The existing Chick-fil-a restaurant located at 3905 N Druid Hills Rd in Decatur, GA, is proposing to relocate to the southeast corner of Birch Road at N Druid Hills Rd intersection. The proposed site location was previously occupied by a Pier 1 Imports retail store. This memo analyzes the need for a westbound left turn lane on N Druid Hills Rd for inbound traffic onto Birch Road. The data analyzed is based on the existing conditions traffic data.

Traffic count data for this project was collected on Tuesday July 13, 2021. Peak hour turning movement counts were collected at the intersections of Birch Road at N Druid Hills Rd and N Druid Hills Rd at both existing Chick-fil-a driveway locations. 24-HR Bi-directional counts were also collected for the locations of Birch Road and North Druid Hills Rd on Tuesday July 13, 2021. Listed below in Table 1 are the ADT volumes collected on July 13, 2021.

Table 1: Daily Traffic Volumes

Road	Location	Direction	Daily Approach Traffic	Daily Two-Way Traffic			
North Druid Hills Rd	East of N Druid Hills Ct	EB	15,945	33,742			
North Diala Hills Ka	East of N Didia Hills Ct	WB	17,797	33,742			
Dirch Dood	South of North Druid Hills Rd	NB	639	1 550			
Birch Road	South of North Druid Hills Rd	SB	920	1,559			

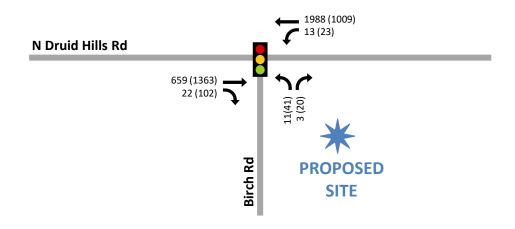
Peak hour turning movement counts were also collected at the Birch Rd at N Druid Hills Rd intersection. Turning movement counts at both existing Chick-fil-a driveway locations were collected from 6:00 am to 8:00 pm. The peak hour volumes at the Birch Rd intersection are provided in Figure 1. The full set of traffic data is provided in Appendix A. Collected volumes have not been adjusted for collection during the summer when school is not in session.

Figure 1: Existing Traffic Volumes

(##) AM (PM) Traffic Volumes



Existing Turning Movement Counts
N Druid Hills Rd at Birch Rd Intersection





Trip Generation & Data Collection

Typically, to estimate the trips that are anticipated to be generated by a proposed development a trip generation for the development is calculated using equations contained in the Institute of Transportation Engineers' (ITE) latest *Trip Generation Manual*, 10th Ed, 2017. In this case we have collected traffic volumes at both existing site driveway locations, servicing the existing Chick-fil-a facility, over a 14-hour period. This traffic data is expected to be representative of the traffic generated by this existing Chick-fil-a restaurant location on a typical weekday and can be utilized to provide some insight into volume demand, trip generation rates and traffic patterns associated with fast food restaurants in this area. The traffic count data for the existing Chick-fil-a is also provided in Appendix A.

Turn Lane Warrant Analysis:

Turn lane warrants were assessed per the *Georgia Department of Transportation Regulations for Driveway & Encroachment Control*. Turn lane warrants that were analyzed were left turn from the major road approach for the existing conditions.

North Druid Hills Rd at Birch Road was evaluated for conditions as a four-lane road, ADT greater than 10,000 vpd and a roadway at 40 mph:

- Total daily inbound volumes on Birch Road at 920 vpd.
- A Left-turn lane is required if there are 250 left turning vehicles (LTV) per day.
- At 30-50% distribution for inbound lefts from N Druid Hills Rd onto Birch Rd there are expected to be more than 250 LTV per day.
- A westbound Left-turn lane is Warranted at N Druid Hills Rd and Birch Rd in the existing conditions.

In addition to GDOT requirements, turn lane warrants were also analyzed for the need for a left turn lane, in the existing conditions, from N Druid Hills Rd onto Birch Rd per NCHRP Report 457: Evaluating Intersection Improvement recommendations which evaluates the necessity of a turn lane comparing the amount of turning movements with advancing and opposing volumes. Turn lane warrants that were analyzed were the westbound left-turn from major road.

• Westbound Left-turn Lane: Warranted (PM peak period) in the existing conditions.

The full turn-lane warrant analysis sheets are attached in Appendix B to this memo.



Conclusions and Recommendations

As a result of this analysis, it is determined that a westbound left-turn lane on North Druid Hills Rd at Birch Rd is warranted in the existing traffic conditions. The relocation of the Chick-fil-a restaurant to the southeast corner of the N Druid Hills Rd at Birch Rd intersection would be expected to add additional volumes to the already warranted left-turn lane at this intersection.

Please contact me or Jack Johnson at 770-368-1399 if you have any questions or need additional information.

Sincerely,

FORESITE GROUP, LLC

Stevie Berryman Project Manager



Appendix A: Traffic Counts



Prepared by National Data & Surveying Services

VOLUME

N Druid Hills Rd E/O N Druid Woods Ct

Day: Tuesday **Date:** 7/13/2021

City: Decatur
Project #: GA21_180190_001

	DAILY TOTAL	.S		NB		SB		EB	WB	_						tal
				0		0		15,945	17,79							742
AM Period 00:00	NB SB	EB 28		WB 22		50	TAL	PM Period 12:00	NB	SB	201		WB 228		429	TAL
00:15		23		19		42		12:15			236		259		495	
00:30 00:45		28 36	115	26 22	89	54 58	204	12:30 12:45			240 263	940	235 239	961	475 502	1901
01:00		42	113	32	03	74	204	13:00			242	340	239	901	481	1901
01:15		32		21		53		13:15			215		261		476	
01:30 01:45		27 29	130	21 16	90	48 45	220	13:30 13:45			246 250	953	278 249	1027	524 499	1980
02:00		30	130	9	30	39	220	14:00			281	333	224	1027	505	1500
02:15		26		16		42		14:15			305 362		244		549	
02:30 02:45		31 23	110	16 10	51	47 33	161	14:30 14:45			319	1267	283 259	1010	645 578	2277
03:00		23		11		34		15:00			356		249		605	
03:15 03:30		19 16		13 11		32 27		15:15 15:30			344 357		286 237		630 594	
03:45		16	74	23	58	39	132	15:45			332	1389	236	1008	568	2397
04:00		14		11		25		16:00			341		273		614	
04:15 04:30		16 15		28 45		44 60		16:15 16:30			337 339		275 236		612 575	
04:45		12	57	46	130	58	187	16:45			341	1358	245	1029	586	2387
05:00		19		59		78		17:00			343		245		588	
05:15 05:30		22 31		83 131		105 162		17:15 17:30			311 326		243 254		554 580	
05:45		49	121	159	432	208	553	17:45			355	1335	232	974	587	2309
06:00 06:15		46 72		166 259		212 331		18:00 18:15			344 330		237 212		581 542	
06:30		80		301		381		18:30			323		230		553	
06:45		103	301	324	1050	427	1351	18:45			269	1266	217	896	486	2162
07:00 07:15		102 113		347 384		449 497		19:00 19:15			268 267		170 194		438 461	
07:30		148		437		585		19:30			255		179		434	
07:45		157	520	505	1673	662	2193	19:45			232	1022	149	692	381	1714
08:00 08:15		169 163		479 482		648 645		20:00 20:15			231 214		142 113		373 327	
08:30		160		427		587		20:30			161		136		297	
08:45 09:00		177 188	669	441 343	1829	618 531	2498	20:45 21:00			182 147	788	139 123	530	321 270	1318
09:15		164		354 354		518		21:00 21:15			102		111		213	
09:30		154		330		484		21:30			110		129		239	
09:45 10:00		185 157	691	319 237	1346	504 394	2037	21:45 22:00			125 135	484	118 85	481	243	965
10:15		183		241		424		22:15			133		76		209	
10:30		195	740	263	072	458	4602	22:30			138	504	84	220	222	024
10:45 11:00		175 193	710	231 229	972	406 422	1682	22:45 23:00			98 83	504	75 52	320	173 135	824
11:15		214		235		449		23:15			82		55		137	
11:30 11:45		216 207	830	261 229	954	477 436	1784	23:30 23:45			74 72	311	45 43	195	119 115	506
TOTALS		207	4328	229	8674	430	13002	TOTALS			12	11617	43	9123	113	20740
SPLIT %			33.3%		66.7%		38.5%	SPLIT %				56.0%		44.0%		61.5%
				NID.		SB			WP						-T-	
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								13,343							- 33,	7-72
AM Peak Hour			11:45		07:30		07:45	PM Peak Hour				15:00		14:30		14:30
AM Pk Volume Pk Hr Factor			884 0.921		1903 0.942		2542 0.960	PM Pk Volume Pk Hr Factor				1389 0.973		1077 0.941		2458 0.953
7 - 9 Volume	0	0	1189		3502		4691	4 - 6 Volume	0	C)	2693		2003		4696
7 - 9 Peak Hour			08:00		07:30		07:45	4 - 6 Peak Hour				16:15		16:00		16:00
7 - 9 Pk Volume			669		1903		2542	4 - 6 Pk Volume				1360		1029		2387
Pk Hr Factor	0.000	0.000	0.945		0.942		0.960	Pk Hr Factor	0.000	0.0	000	0.991		0.935		0.972

VOLUME

Birch Rd S/O N Druid Hills Rd

Day: Tuesday **Date:** 7/13/2021

City: Decatur

City: Decatur **Project #:** GA21_180190_002

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00:30	0		0					0		12:30	16		13						29	
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01:00 01:15	0 2		0 2					0 4		13:00 13:15	11 21		9 19						20 40	
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05:45 06:00	3	2	9 6	11				10 9	13	17:45 18:00	12 16	64	24 22	130					36 38	194
06:15	3		7					10		18:15	12		26						38	
06:30	2	45	4	24				6	20	18:30	8	4.4	24	01					32	125
06:45 07:00	<u>7</u> 5	15	4	21				11 9	36	18:45 19:00	8 15	44	<u>9</u> 7	81					17 22	125
07:15	6		6					12		19:15	10		14						24	
07:30 07:45	6 1	18	2 12	24				8 13	42	19:30 19:45	15 7	47	15 6	42					30 13	89
08:00	2	10	7	24				9	42	20:00	9	47	12	42					21	09
08:15	6		10					16		20:15	8		4						12	
08:30 08:45	13 10	31	9 6	32				22 16	63	20:30 20:45	1 2	20	5 8	29					6 10	49
09:00	4	- 31	5	32				9	- 03	21:00	5		4	23					9	43
09:15	3		7					10		21:15	3		1						4	
09:30 09:45	5 13	25	4 7	23				9 20	48	21:30 21:45	2 1	11	2 1	8					4 2	19
10:00	6	23	7	23				13		22:00	1		2						3	13
10:15	8		10					18		22:15	2		3						5	
10:30 10:45	4 4	22	10 11	38				14 15	60	22:30 22:45	0 1	4	3 1	9					3 2	13
11:00	9		11	30				20		23:00	0		3						3	
11:15	11		8					19		23:15	1		1						2	
11:30 11:45	7 9	36	10 11	40				17 20	76	23:30 23:45	0	1	1 1	6					1 1	7
TOTALS		161		202					363	TOTALS	J	478	_	718					_	1196
SPLIT %		44.4%		55.6%					23.3%	SPLIT %		40.0%		60.0%						76.7%
			-0	1.0		NB		SB		EB		WB							To	otal
	D/	AILY 1	IOTA	11.5		639		920		0		0								559
AM Peak Hour		11:45		11:45					11:45	PM Peak Hour		16:45		16:45						16:45
AM Pk Volume		57		62					119	PM Pk Volume		80		137						217
Pk Hr Factor		0.792		0.705					0.826	Pk Hr Factor		0.714		0.835						0.904
7 - 9 Volume 7 - 9 Peak Hour		49 08:00		56 07:45					105	4 - 6 Volume 4 - 6 Peak Hour		131		248						379 16:45
7 - 9 Peak Hour 7 - 9 Pk Volume		31		07:45 38					08:00 63	4 - 6 Peak Hour 4 - 6 Pk Volume		16:45 80		16:45 137						16:45 217
Pk Hr Factor		0.596		0.792	0.00	0	0.000		0.716	Pk Hr Factor		0.714		0.835		0.000		0.000		0.904

National Data & Surveying Services Intersection Turning Movement Count

Location: Birch Rd & N Druid Hills Rd City: Decatur Control: Signalized

Project ID: 21-180189-001 Date: 7/13/2021

Data - Totals	5
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NS/EW Streets:	Birch Rd Birch Rd NORTHBOUND SOUTHBOUND								N Druid Hills Rd N Druid Hills Rd								1
		NORTH	BOUND			SOUTI	HBOUND			EASTB	OUND			WESTB	OUND		
AM	1	0	1	0	0	0	0	0	0	2	1	0	0	2	0	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
6:30 AM	2	0	0	0	0	0	0	0	0	81	0	0	2	314	0	0	399
6:45 AM	5	0	2	0	0	0	0	0	0	103	3	0	3	334	0	0	450
7:00 AM	4	0	1	0	0	0	0	0	0	103	3	0	1	356	0	0	468
7:15 AM	5	0	1	0	0	0	0	0	0	117	1	0	1	402	0	0	527
7:30 AM	4	0	1	0	0	0	0	0	0	151	3	0	2	464	0	0	625
7:45 AM	0	0	1	0	0	0	0	0	0	166	10	0	3	522	0	0	702
8:00 AM	2	0	0	0	0	0	0	0	0	171	3	0	4	515	0	0	695
8:15 AM	5	0	1	0	0	0	0	0	0	171	6	0	4	487	0	0	674
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES:	27	0	7	0	0	0	0	0	0	1063	29	0	20	3394	0	0	4540
APPROACH %'s:	79.41%	0.00%	20.59%	0.00%					0.00%	97.34%	2.66%	0.00%	0.59%	99.41%	0.00%	0.00%	
PEAK HR:	(7:30 AM -	08:30 AM														TOTAL
PEAK HR VOL:	11	0	3	0	0	0	0	0	0	659	22	0	13	1988	0	0	2696
PEAK HR FACTOR :	0.550	0.000	0.750	0.000	0.000	0.000	0.000	0.000	0.000	0.963	0.550	0.000	0.813	0.952	0.000	0.000	0.960
		0.58	83							0.96	52			0.95	3		0.960
		NORTH	ROLIND			SOLITI	HROLIND			FASTR	OLIND			WESTR	ULIND		

		NORTH	BOUND			SOUTI	HBOUND			EASTE	BOUND			WESTE	OUND		
PM	1	0	1	0	0	0	0	0	0	2	1	0	0	2	0	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
4:00 F	M 7	0	6	0	0	0	0	0	0	347	30	0	4	244	0	0	638
4:15 F	M 11	0	4	0	0	0	0	0	0	328	23	0	8	276	0	0	650
4:30 F	M 5	0	3	0	0	0	0	0	0	340	23	0	5	257	0	0	633
4:45 F	M 18	0	7	0	0	0	0	0	0	348	26	0	6	232	0	0	637
5:00 F	M 12	0	6	0	0	0	0	0	0	337	26	0	7	235	0	0	623
5:15 F		0	3	0	0	0	0	0	0	304	35	0	5	257	0	0	619
5:30 F	M 14	0	2	0	0	0	0	0	0	330	28	0	4	234	0	0	612
5:45 F	M 8	0	2	0	0	0	0	0	0	379	19	0	1	218	0	0	627
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES		0	33	0	0	0	0	0	0	2713	210	0	40	1953	0	0	5039
APPROACH %'s	73.17%	0.00%	26.83%	0.00%					0.00%	92.82%	7.18%	0.00%	2.01%	97.99%	0.00%	0.00%	
PEAK HE	t :	04:00 PM -	05:00 PM														TOTAL
PEAK HR VO	-: 41	0	20	0	0	0	0	0	0	1363	102	0	23	1009	0	0	2558
PEAK HR FACTOR	0.569	0.000	0.714	0.000	0.000	0.000	0.000	0.000	0.000	0.979	0.850	0.000	0.719	0.914	0.000	0.000	0.984
		0.6	10	-	:'					0.9	71			0.9	08		0.964

${\tt National\ Data\ \&\ Surveying\ Services} \\ Intersection\ Turning\ Movement\ Count$

Location: Chick-fil-A W Dwy & N Druid Hills Rd City: Decatur Control: No Control

Project ID: 21-180189-002 Date: 7/13/2021

Control.	NO CONTROL					Data - Totals								Date. 7	/13/2021		
NS/EW Streets:		Chick-fil-A					A W Dwy			N Druid H	lills Rd			N Druid I			
AM	1	NORTH 1	BOUND 0	0	0	SOUTI 0	HBOUND 0	0	0	EASTB0	OUND 0	0	0	WESTB 3	OUND 0	0	
6:00 AM	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 47	ER 0	EU 0	WL 9	WT 187	WR 0	WU 0	TOTAL 243
6:15 AM 6:30 AM	1 2	0	1	0	0	0	0	0	0	76 83	7	0	6 15	274 315	0	0	365 419
6:45 AM 7:00 AM	4	0	3	0	0	0 0	0	0	0	112 107	5	0	11 19	339 363	0	0	474 497
7:15 AM	3	0	0	0	0	0	0	0	0	125	6	0	10	404	0	0	548
7:30 AM 7:45 AM	4 3	0	2	0	0	0	0	0	0 0	167 171	15 10	0	16 6	470 526	0 0	0 0	674 718
8:00 AM 8:15 AM	5 4	0 0	4 3	0 0	0	0 0	0 0	0 0	0	177 177	10 1	0	16 10	517 497	0 0	0 0	729 692
8:30 AM 8:45 AM	1 3	0 0	3 5	0	0	0	0	0	0	181 183	10 12	0	17 17	465 477	0 0	0	677 697
9:00 AM 9:15 AM	3	0	4	0	0	0	0	0	0	191 181	10 10	0	13 16	363 386	0	0	584 600
9:30 AM 9:45 AM	5	0	5	0	0	0	0	0	0	168 208	17 13	0	20 10	365 351	0	0	580 588
	NL	NT	NR 42	NU	SL 0	ST	SR	SU	EL	ET 2254	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES : APPROACH %'s : PEAK HR :	45 51.14%	0 0.00% 07:45 AM -	43 48.86% 08:45 AM	0.00%	U	0	0	0	0 0.00%	2354 94.65%	133 5.35%	0 0.00%	211 3.24%	6299 96.76%	0 0.00%	0 0.00%	9085 TOTAL
PEAK HR VOL : PEAK HR FACTOR :	13 0.650	0 0.000	12 0.750	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	706 0.975	31 0.775	0 0.000	49 0.721	2005 0.953	0 0.000	0 0.000	2816 0.966
		0.6				COLITI	IDOLIND			0.96				0.96			0.500
NOON	1	1	BOUND 0	0	0	0	HBOUND 0	0	0	EASTBO	0	0	0	WESTB 3	0	0	TOTAL
10:00 AM	NL 6	NT 0	NR 7	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	172	ER 7	EU 0	WL 16	WT 259	WR 0	WU 0	TOTAL 467
10:15 AM 10:30 AM	7 4	0 0	3 4	0	0	0 0	0 0	0 0	0 0	187 226	17 9	0	10 13	259 277	0 0	0 0	483 533
10:45 AM 11:00 AM	3 2	0	4	0	0	0	0	0	0	196 220	10 10	0	12 18	258 230	0	0	483 482
11:15 AM 11:30 AM	5 3	0	6 7	0	0	0	0	0	0	228 254	14 15	0	13 15	259 281	0 0	1 0	526 575
11:45 AM 12:00 PM	1 3	0	9 7	0	0	0	0	0	0	232 229	14 10	0	24 16	247 250	0	0	527 516
12:15 PM 12:30 PM	2	0	5	0	0	0	0	0	0	258 259	16 16	0	12 22	278 237	0	0	571 543
12:45 PM 1:00 PM	4 3	0	7	0	0	0	0	0	0	291 266	17 18	0	11 18	268 269	0	3	601 582
1:15 PM 1:30 PM	9	0	10 5	0	0	0	0	0	0	273 250	18 15	0	10 19	294 290	0	0	614 580
1:45 PM	3	0	8	0	0	0	0	0	0	274	20	0	8	280	0	0	593
TOTAL VOLUMES :	NL 59	NT 0	NR 97	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 3815	ER 226	EU 0	WL 237	WT 4236	WR 0	WU 6	TOTAL 8676
APPROACH %'s : PEAK HR :	37.82%	0.00% 12:45 PM -	62.18% 01:45 PM	0.00%					0.00%	94.41%	5.59%	0.00%	5.29%	94.57%	0.00%	0.13%	TOTAL
PEAK HR VOL : PEAK HR FACTOR :	17 0.472	0.000	29 0.725	0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	1080 0.928	68 0.944	0 0.000	58 0.763	1121 0.953	0 0.000	4 0.333	2377
		0.6	05							0.93				0.95	57		0.968
PM	1	1	BOUND 0	0	0	0	HBOUND 0	0	0	EASTB0	OUND 0	0	0	WESTE 3	0	0	
2:00 PM	NL 3	NT 0	NR 5	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	518	ER 8	EU 0	WL 18	WT 225	WR 0	WU 0	TOTAL 577
2:15 PM 2:30 PM	1 2	0	10 8	0	0	0	0	0	0 0	328 392	15 15	0	10 14	272 289	0	0	636 720
2:45 PM 3:00 PM	1 2	0	8	0	0	0	0	0	0	355 367	14 16	0	9 8	273 271	0	0	660 665
3:15 PM 3:30 PM	3 2	0	4 5	0	0	0	0	0	0	389 356	15 8	0	10 12	300 252	0	0 1	721 636
3:45 PM 4:00 PM	1 2	0	1 3	0	0	0	0	0	0	378 391	13 10	0	7	247 284	0	0	647 694
4:15 PM	1	Ō	5	0	Ō	0	0	0	0	373	9	1	9	287	Ō	0	685
4:30 PM 4:45 PM	0 4	0	4	0	0	0	0	0	0	366 381	12 9	0	9	249 258	0	0	641 659
5:00 PM 5:15 PM	1	0 0	4 4	0 0	0	0 0	0 0	0 0	0	394 352	9 16	0	7 8	277 270	0 0	0 0	693 651
5:30 PM 5:45 PM	2 0	0	4 0	0	0	0	0	0	0 0	374 394	8 13	0	5 4	257 256	0 0	0 0	650 667
6:00 PM 6:15 PM	0	0	3	0	0	0	0	0	0	369 353	12 10	0	4 7	238 234	0	0	626 609
6:30 PM 6:45 PM	4	0	4 2	0	0	0	0	0	0	378 322	14 11	0	6 14	241 229	0	0	647 581
7:00 PM 7:15 PM	1 3	0	3 2	0	0	0	0	0	0	290 286	19 10	0	14 6	179 208	0	0	506 515
7:30 PM 7:45 PM	3	0	6 2	0	0	0	0	0	0	276 257	13 12	0	13 11	193 169	0	1 0	505 451
TOTAL VOLUMES :	NL 43	NT 0	NR 94	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 8439	ER 291	EU 2	WL 213	WT 5958	WR 0	WU 2	TOTAL 15042
APPROACH %'s :	31.39%	0.00% 02:30 PM -	68.61%	0.00%	0	3		,	0.00%	96.64%	3.33%	0.02%	3.45%	96.52%	0.00%	0.03%	TOTAL
PEAK HR VOL : PEAK HR FACTOR :	8 0.667	0.000	21 0.656	0 0.000	0 0.000	0.000	0.000	0 0.000	0 0.000	1503 0.959	60 0.938	0.000	41 0.732	1133 0.944	0.000	0.000	2766 0.959
		0.7	23							0.96	U			0.94	1		

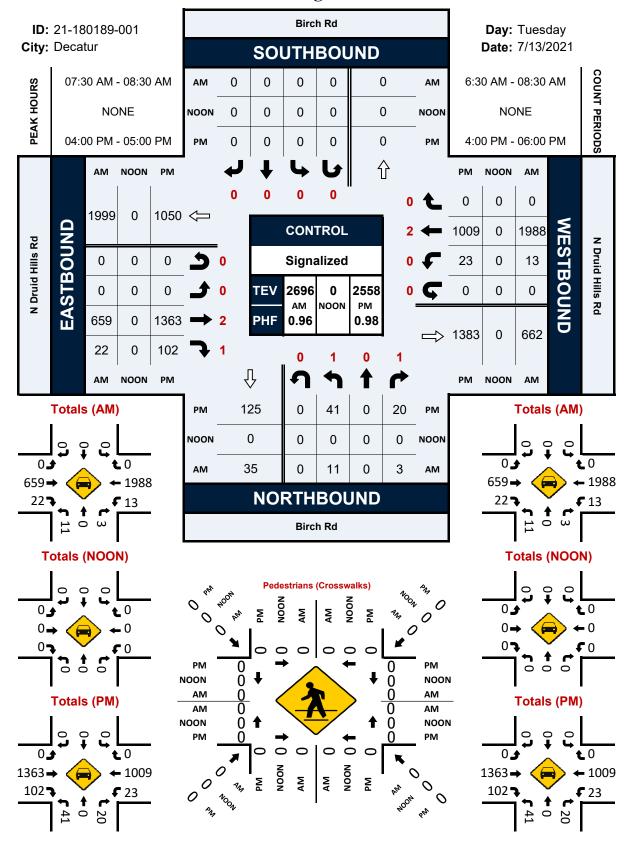
${\tt National\ Data\ \&\ Surveying\ Services} \\ Intersection\ Turning\ Movement\ Count$

Location: Chick-fil-A E Dwy & N Druid Hills Rd City: Decatur Control: 1-Way Stop(SB)

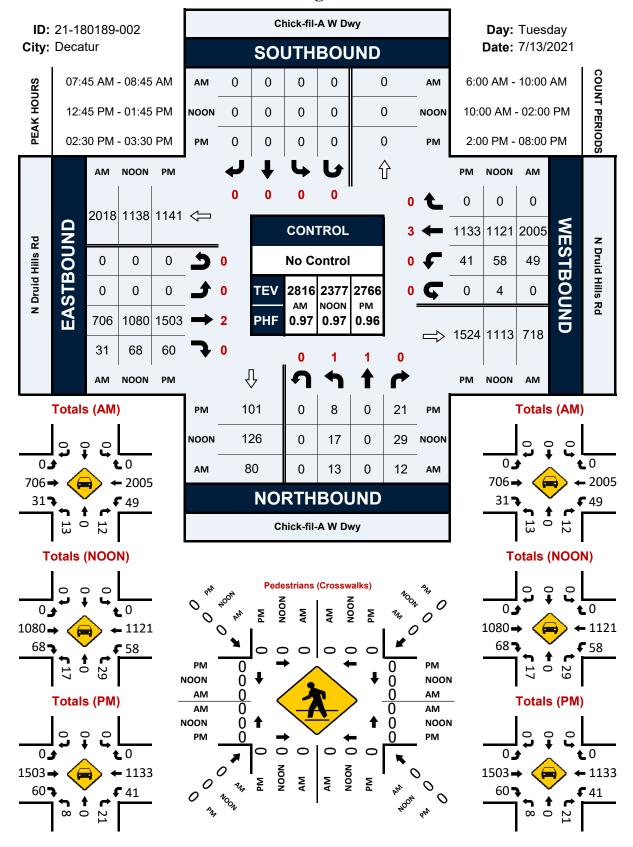
Project ID: 21-180189-003 Date: 7/13/2021

Control.	1-way Stop	(30)						Data -	Totals				Date. 7	/13/2021			
NS/EW Streets:		Chick-fil-A				Chick-fil-A				N Druid H				N Druid I			
AM	0.5	NORTH 0	BOUND 0.5	0	0	SOUTHI 2	BOUND 0	0	0	EASTB0	OUND 0	0	0	WESTB 3	OUND 0	0	
6:00 AM	NL 1	NT 0	NR 5	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 48	ER 0	EU 0	WL 0	WT 199	WR 1	WU 0	TOTAL 254
6:15 AM 6:30 AM	2	1	9	0	0	0	0	0	0	76 85	0	0	0	274 334	0	0	362 434
6:45 AM 7:00 AM	2	0	9	0	1 0	0	1 2	0	1 2	111 110	0	0	0	339 376	6 8	0	470 512
7:15 AM	4	0	13	0	0	0	0	0	0	126	Ō	0	0	420	6	0	569
7:30 AM 7:45 AM	3 1	0	12 14	0	0 2	0	3 5	0 0	2 1	163 176	0	0	0	470 526	8 9	1 0	662 734
8:00 AM 8:15 AM	3 1	1 0	10 9	0	1 0	0 0	3 0	0 0	0 1	181 179	0 0	0	0 0	529 504	12 10	0 1	740 705
8:30 AM 8:45 AM	1 2	0	16 13	0	0 0	0	1 2	0	1	181 188	0 0	0	0 0	480 491	10 15	0	690 712
9:00 AM 9:15 AM	2 2	0	18 15	0	2 2	0	6	0	0	196 185	0	0	0	367 396	16 16	0	607 621
9:30 AM 9:45 AM	3 1	0	10 19	0	2 2	0 0	5 3	1 0	2 4	170 208	0	0	0	381 353	8 21	0	582 611
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES : APPROACH %'s :	34 15.04%	3 1.33%	189 83.63%	0 0.00%	12 24.00%	0 0.00%	37 74.00%	1 2.00%	15 0.63%	2383 99.37%	0 0.00%	0 0.00%	0 0.00%	6439 97.69%	149 2.26%	3 0.05%	9265
PEAK HR : PEAK HR VOL :	6	07:45 AM - 1	08:45 AM 49	0	3	0	9	0	3	717	0	0	0	2039	41	1	TOTAL 2869
PEAK HR FACTOR :	0.500	0.250 0.8	0.766 24	0.000	0.375	0.000 0.42	0.450 29	0.000	0.750	0.990 0.98	0.000	0.000	0.000	0.964 0.96	0.854 52	0.250	0.969
NOON		NORTH				SOUTH				EASTBO				WESTE			
NOON	0.5 NL	0 NT	0.5 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	3 WT	0 WR	0 WU	TOTAL
10:00 AM 10:15 AM	1 2	0 0	17 13	0	5 3	0 0	2 5	0 0	0 1	174 193	1 0	0	0 0	271 262	21 19	0 0	492 498
10:30 AM 10:45 AM	4 4	1 0	16 13	0	7 6	0 0	8	0 0	2 0	228 200	0 0	0	0 0	278 263	17 13	0 0	561 502
11:00 AM 11:15 AM	2 1	0	8 22	0	3 1	0 0	7 5	0 0	1 3	214 236	0 0	0 2	0 0	239 274	26 17	0 0	500 561
11:30 AM 11:45 AM	2	0	21 21	0	5 1	0	6 10	0 1	1 2	261 239	0	0	0	279 261	18 21	0	593 557
12:00 PM 12:15 PM	4 2	0	13 28	0	7	0	2 7	0	2	233 262	0	0	0	268 274	32 28	0	561 610
12:30 PM 12:45 PM	2 2	0	15 15	0	6	0	7 15	0	1	264 289	0	0	0	251 264	21 21	0	567 613
1:00 PM	2	0	18	0	2	0	10	0	3	279	0	0	0	277	25	0	616
1:15 PM 1:30 PM	2	0	21 20	0	6	0	9 10	0	2	274 260	0	0	0	292 304	22 28	0	626 632
1:45 PM	1	0	18	0	6	0	5	0	3	278	1	0	0	275	33	0	620
TOTAL VOLUMES :	NL 33	NT 2	NR 279	O NU	SL 69	ST 0	SR 111	SU 2	EL 30	ET 3884	ER 2	EU 2	WL 0	WT 4332	WR 362	WU 1	TOTAL 9109
APPROACH %'s : PEAK HR :		0.64% 01:00 PM -		0.00%	37.91%	0.00%	60.99%	1.10%	0.77%	99.13%	0.05%	0.05%	0.00%	92.27%	7.71%	0.02%	TOTAL
PEAK HR VOL : PEAK HR FACTOR :	7 0.875	0 0.000	77 0.917	0.000	18 0.750	0.000	34 0.850	0 0.000	10 0.833	1091 0.978	1 0.250	0 0.000	0 0.000	1148 0.944	108 0.818	0 0.000	2494 0.987
		0.9				0.81				0.97				0.94			0.307
PM	0.5	NORTH 0	0.5	0	0	SOUTH!	0	0	0	EASTB0	0	0	0	WESTE 3	0	0	
2:00 PM	NL 0	NT 0	NR 20	NU 0	SL 4	ST 0	SR 8	SU 0	EL 1	320	ER 0	EU 0	WL 0	WT 231	WR 19	WU 1	TOTAL 604
2:15 PM 2:30 PM	0 0	1 0	12 12	0	6 4	0 0	10 8	0	2 1	338 399	0 0	0	0 0	275 294	18 13	0	662 731
2:45 PM 3:00 PM	0	0	14 20	0	4 2	0	<u>7</u>	0	0	360 370	0	0	0	272 281	19 28	0	678 707
3:15 PM 3:30 PM	1	0	20 19	0	2 6	0	7 10	0	0	393 360	0	0	0	294 255	21 14	0	738 666
3:45 PM 4:00 PM	0	0	16 18	0	1 7	0	4	0	5 3	367 399	0	0	0	250 281	31 25	0	674 745
4:15 PM 4:30 PM	1 0	0	10	0	7 7	0	2 7	0	1	377	0	Ō	0	288 251	28 28	0	714 672
4:45 PM	0	Ō	12 9	Ō	6	0	9	Ō	1	366 390	Ō	0	0	261	16	0	692
5:00 PM 5:15 PM	0	0	12 15	0	8	0	6 5	0	1	393 346	0	0	0	269 273	24 29	0	714 677
5:30 PM 5:45 PM	0	0	15 14	0	8	0	14 4	0	0	374 403	0	0	0	252 251	25 21	0	682 702
6:00 PM 6:15 PM	1 0	0 0	9 11	0	6 6	0 0	7 9	0	1	366 356	0 0	0	0 0	240 226	25 23	0	655 632
6:30 PM 6:45 PM	1 1	0 0	17 13	0	4 8	0 0	6 4	0 0	1 0	385 324	0 0	0 0	0	245 233	34 27	0 0	693 610
7:00 PM 7:15 PM	2 2	0	16 20	0	4 8	0	0 6	0	0 1	287 293	0	0	0	191 217	18 24	0	518 572
7:30 PM 7:45 PM	2	0	16 17	0	4 7	0	4 7	0	2	281 258	0	0	0	190 173	14 13	0	513 477
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES : APPROACH %'s : PEAK HR :	16 4.26%	3 0.80% 02:30 PM -	357 94.95%	0.00%	126 43.90%	0 0.00%	161 56.10%	0 0.00%	29 0.34%	8505 99.66%	0.00%	0 0.00%	0.00%	5993 91.76%	537 8.22%	1 0.02%	15728 TOTAL
PEAK HR VOL : PEAK HR FACTOR :	1 0.250	1 0.250	66 0.825	0 0.000	12 0.750	0 0.000	28 0.875	0 0.000	2 0.500	1522 0.954	0 0.000	0 0.000	0 0.000	1141 0.970	81 0.723	0 0.000	2854
		0.8			550	0.83				0.95				0.97		,	0.967

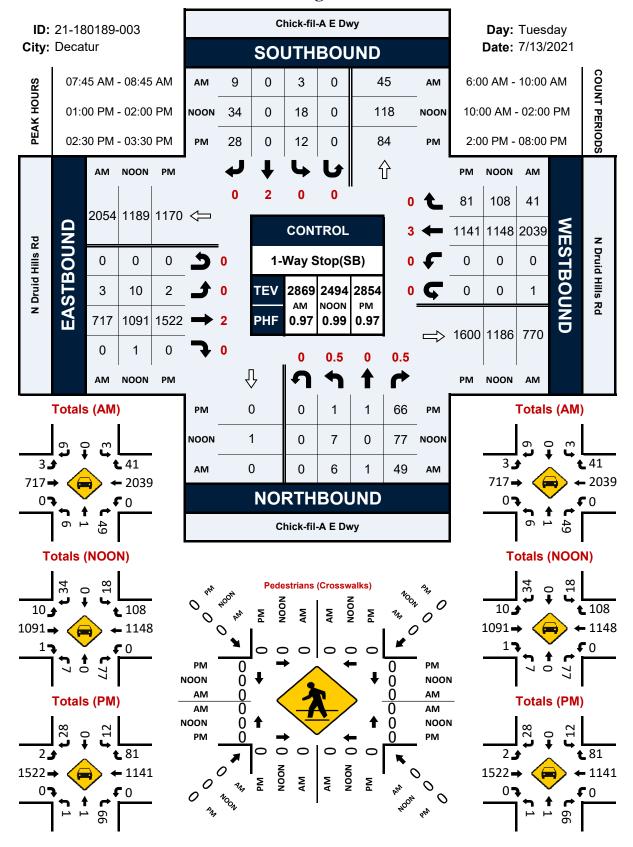
Birch Rd & N Druid Hills Rd



Chick-fil-A W Dwy & N Druid Hills Rd



Chick-fil-A E Dwy & N Druid Hills Rd



Appendix B: Turn Lane Warrant Worksheets



Note: This warrant is being applied to a 4-phase signal that controls the minor street approach but the main line left (N Druid Hills) is not signalized (protected phase) and still makes a permissive left turn when there are acceptable gaps in traffic.

Figure 2 - 5. Guideline for determining the need for a major-road left-turn bay at a two-way stop-controlled intersection.

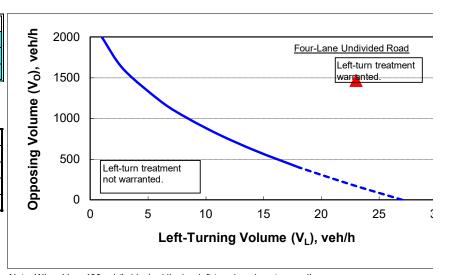
4-lane roadway

INPUT

Variable	Value
Left-turning volume (V _L), veh/h:	23
Advancing volume (V _A), veh/h:	1032
Opposing volume (V _O), veh/h:	1465

OUTPUT

Variable	Message									
Opposing volume (Vo) check:	O.K.									
Combined volume (V _A and V _O) check:	O.K.									
Guidance for determining the need for a major	Guidance for determining the need for a major-road left-turn bay:									
Left-turn treatment warranted.										



CALIBRATION CONSTANTS

Variable	Value
Average time for making left-turn, s:	4.0
Critical headway, s:	6.0

Note: When V_O < 400 veh/h (dashed line), a left-turn lane is not normally warranted unless the advancing volume (V_A) in the same direction as the left-turning traffic exceeds 400 veh/h (V_A > 400 veh/h).



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September 2, 2021

Mrs. Patrece Keeter
Dekalb County Public Works
Transportation Division
1300 Commerce Drive
Decatur, GA 30030

Intersection Analysis Memo:

North Druid Hills Road at Birch Road Chick-fil-a Relocation Intersection Analysis - Decatur, GA

Existing Traffic Volumes

The existing Chick-fil-a restaurant located at 3905 N Druid Hills Rd in Decatur, GA, is proposing to relocate to the southeast corner of Birch Road at N Druid Hills Rd intersection. The proposed site location was previously occupied by a Pier 1 Imports retail store. This memo analyzes the intersection LOS and queuing at North Druid Hills at Birch Road in the existing and proposed building conditions and the potential impacts on the operation of this intersection by the proposed relocated Chick-fil-a restaurant.

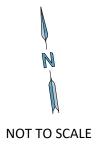
Traffic count data for this project was originally collected on Tuesday July 13, 2021. Peak hour turning movement counts were collected at the intersections of Birch Road at N Druid Hills Rd and N Druid Hills Rd at both existing Chick-fil-a driveway locations. Additional peak hour turning movement counts were recollected at the Birch Rd intersection on Tuesday August 10th, 2021, after the school year had started. The peak hour volumes at the Birch Rd intersection are provided in Figure 1. The full set of traffic data is provided in Appendix A. Previous collected volumes at the Chick-fila driveways have not been adjusted for collection during the summer when school was not in session.

Proposed Traffic Volumes

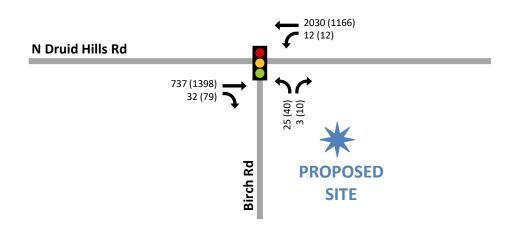
For the proposed traffic volumes, the existing Chick-fil-a trips were distributed to the Birch Rd intersection and the proposed right-in-right-out driveway on North Druid Hills Rd. Chick-fil-a estimates the new site location to generate 10% more trips, so this growth factor is applied to the redistributed trips. The proposed peak hour volumes at the Birch Road intersection are provided in Figures 2 and 3.

Figure 1: Existing Traffic Volumes

(##) AM (PM) Traffic Volumes

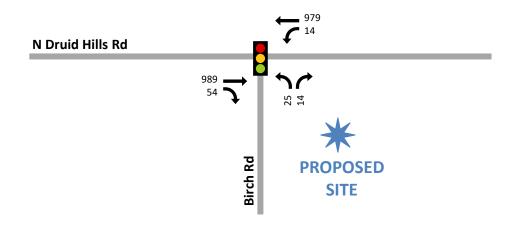


Existing Turning Movement Counts N Druid Hills Rd at Birch Rd Intersection



(##) MD Traffic Volumes (midday)

Existing Turning Movement Counts
N Druid Hills Rd at Birch Rd Intersection

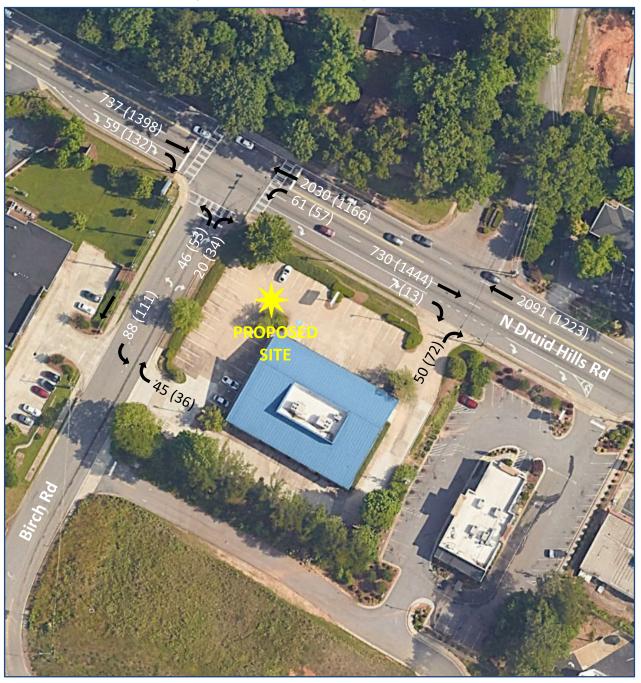




(##) AM (PM) Traffic Volumes

Volumes updated 8/10/2021 AM peak hour of Birch Rd intersection 7:30-8:30am PM peak hour of Birch Rd intersection 5:00-6:00pm

AM (PM) Peak Hour Turning movement counts – Birch Rd + Proposed Chick-fil-a site on North Druid Hills Rd

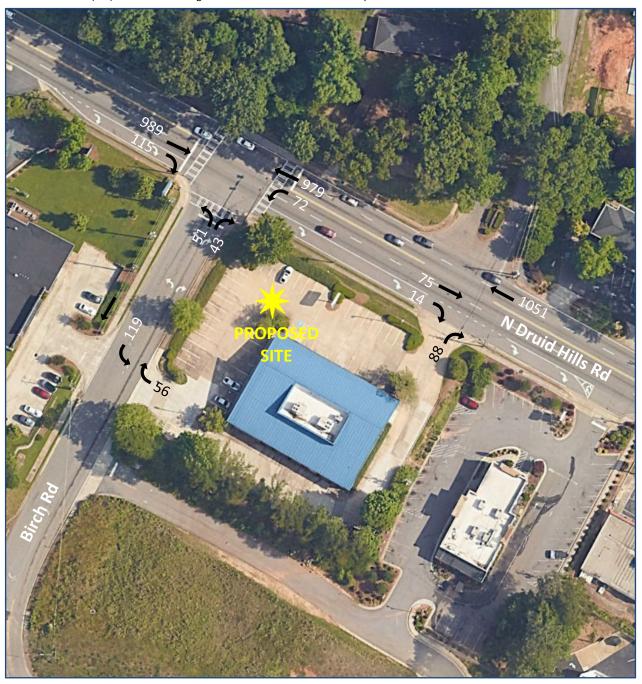




(##) MD Traffic Volumes

Volumes updated 8/10/2021 MD peak hour of Birch Rd intersection 1:00-2:00pm

AM (PM) Peak Hour Turning movement counts – Birch Rd + Proposed Chick-fil-a site on North Druid Hills Rd





Existing Conditions Capacity Analysis

Existing traffic volumes were entered into a *Synchro* 10 model to perform capacity analysis of existing conditions for the AM, MD and PM peak periods. The results of the capacity analysis are shown by lane group movement in Table 1. Average vehicular delays and level-of service, as defined by the Highway Capacity Manual (HCM) 6th Edition are presented, and 95th percentile queues from *SimTraffic* 10 are shown. Full *Synchro* output reports are included Appendix B. The signal timings for the intersection at Birch Rd were not available at the time of this analysis. The timings were estimated to be actuated coordinated with a 140-second cycle length with a side street split time of 25 seconds. These estimated timings were used for both the existing and proposed conditions analysis. For the purposes of this analysis North Druid H

AM Peak Hour MD Peak Hour PM Peak Hour Lane Group Intersection Control 95th% 95th% 95th% Movement Delay (s) LOS Delay (s) Delay (s) Queue (ft) Queue (ft) Queue (ft) EBT Α 1.4 2.0 Α 131 1.1 59 Α 81 EBR 0.8 Α 10 0.9 1.0 33 Α 12 Α WBL 4.4 157 1.7 112 2.1 198 Α Α North Druid Hills Rd Signal WBT 4.9 Α 130 1.7 Α 59 2.2 Α 148 at Birch Rd Control NBL 74.5 74 67 71.1 61 78.1 NRR 66.6 F 5 68.5 Ε 13 66.6 Ε 13

Table 1: Existing Conditions Capacity Analysis

The intersection of Birch Rd at North Druid Hills Rd (with estimated timings) is shown to operate at LOS A during the AM, MD and PM peak periods. Under these conditions the NB approaches from Birch Rd are shown to operate at LOS E. This approach LOS for the minor street is a result of the coordinated operation on the mainline.

In the existing conditions the shared WBT/L lane is shown to have 95th percentile queuing of 157-ft in the AM peak period, 112-ft in the MD peak period and 198-ft in the PM peak period. These queuing lengths are equivalent to approximately 5-8 vehicles queuing per cycle.



Build Conditions Capacity Analysis

The proposed redistributed traffic volumes were also entered into a *Synchro* 10 model to perform capacity analysis of existing conditions for the AM, MD and PM peak periods. The results of the capacity analysis are shown by lane group movement in Table 2. Average vehicular delays and level-of service, as defined by the Highway Capacity Manual (HCM) 6th Edition are presented, and 95th percentile queues from *SimTraffic 10* are shown. Full *Synchro* output reports are included Appendix B. The estimated signal timings for the intersection at Birch Rd remined the same as the existing conditions analysis.

Table 2: Build Conditions Capacity Analysis

		Lane Group		AM Peak Hou	r	N	/ID Peak Hou	r	ı	PM Peak Hou	r
Intersection	Control	Movement	Delay (s)	LOS	95 th % Queue (ft)	Delay (s)	LOS	95 th % Queue (ft)	Delay (s)	LOS	95 th % Queue (ft)
		EBT	1.3	Α	85	1.7	Α	92	2.2	Α	125
	Signal Control	EBR	1.0	Α	23	1.2	А	35	1.2	Α	35
		WBL	6.6	Α	262	2.4	Α	207	3.0	Α	289
North Druid Hills Rd at Birch Rd		WBT	5.9	Α	220	2.5	Α	165	3.0	Α	259
a c Bircii ila		NBL	77.9	E	95	75.6	E	88	76.1	E	90
		NBR	68.6	E	13	75.5	E	25	70.4	E	32
		Intersection	6.5	Α	-	5.1	Α	-	4.7	Α	-

The intersection of Birch Rd at North Druid Hills Rd (with estimated timings) is shown to continue operate at LOS A during the AM, MD and PM peak periods with minor increases to the overall intersection delay. Under these conditions the NB approaches from Birch Rd continue to operate at LOS E with minor increases in delay and queuing.

In the build conditions the shared WBT/L lane is shown to have 95th percentile queuing of 262-ft in the AM peak period, 207-ft in the MD peak period and 289-ft in the PM peak period. These queuing lengths are equivalent to approximately 9-12 vehicles queuing per cycle.



Conclusions and Recommendations

The increased turning movement volumes at the Birch Rd intersection generated by the Chick-fil-a relocation contribute to minor increases in delay and queuing at the intersection approaches. The overall intersection continues to operate at the same LOS A as in the existing conditions with minor increases to overall delay. The Birch Rd minor street approach also continues to operate at the same level of service as it does in the existing conditions. The shared WBT/L lane on North Druid Hills demonstrates increases in queuing equivalent to approximately 100-ft or 4 vehicles per cycle during the peak hour build conditions.

Please contact me or Jack Johnson at 770-368-1399 if you have any questions or need additional information.

Sincerely.

FORESITE GROUP, LLC

Stevie Berryman Project Manager



Appendix A: Traffic Counts



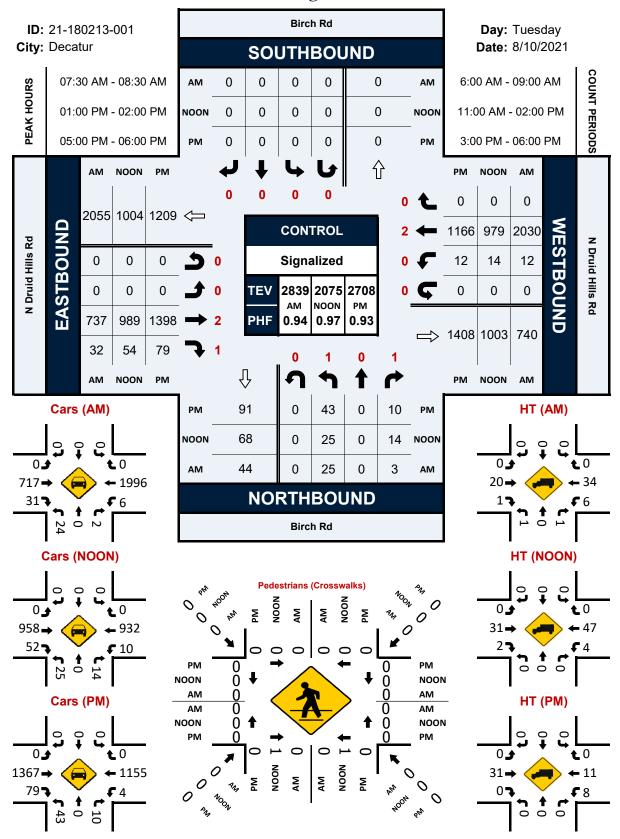
National Data & Surveying Services Intersection Turning Movement Count

Location: Birch Rd & N Druid Hills Rd City: Decatur

Project ID: 21-180213-001

City: Control:	Decatur Signalized												Pr	oject ID: 2 Date: 8	21-180213- 3/10/2021	001	
Γ								Data ·	- Total								
NS/EW Streets:		Birch					h Rd HBOUND			N Druid I				N Druid I			
AM	1 NL	NORTH 0 NT	1 NR	0 NU	0 SL	0 ST	O SR	0 SU	0 EL	EASTB 2 ET	1 ER	0 EU	0 WL	WESTE 2 WT	0 WR	0 WU	TOTAL
6:00 AM 6:15 AM	1 1	0	0	0	0	0	0	0	0	58 98	2	0	0 4	211 287	0	0	272 393
6:30 AM 6:45 AM	0	0	0	0	0	0	0	0	0	99 107	0	0	0	362 419	0	0	461 536
7:00 AM	1	0	0	0	0	0	0	0	0	127	4	0	0	405	0	0	537
7:15 AM 7:30 AM	3 2	Ö	0 2	0 0	0	0	0 0	0	0	153 192	2 9	0 0	2	443 549	0 0	0	603 756
7:45 AM 8:00 AM	7 4	0	0	0	0	0	0	0	0	181 167	7	0	5 3	535 475	0	0	737 656
8:15 AM 8:30 AM	12 6	0	1 0	0	0	0	0 0	0	0	197 191	7 9	0	2 1	471 525	0	0	690 732
8:45 AM	3	Ō	Ō	0	Ō	Ō	Ō	Ō	Ō	159	6	Ō	3	480	Ō	0	651
TOTAL VOLUMES :	NL 43	NT 0	NR 3	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 1729	ER 62	EU 0	WL 25	WT 5162	WR 0	WU 0	TOTAL 7024
APPROACH %'s : PEAK HR :		0.00% 07:30 AM -		0.00%					0.00%	96.54%	3.46%	0.00%	0.48%	99.52%	0.00%	0.00%	TOTAL
PEAK HR VOL : PEAK HR FACTOR :	25 0.521	0 0.000	3 0.375	0 0.000	0.000	0 0.000	0 0.000	0.000	0.000	737 0.935	32 0.889	0 0.000	12 0.600	2030 0.924	0 0.000	0.000	2839 0.939
		0.5								0.94				0.92			0.555
NOON	1	NORTH 0	BOUND 1	0	0	SOUTI 0	HBOUND 0	0	0	EASTB 2	OUND 1	0	0	WESTE 2	0	0	
11:00 AM	NL 8	NT 0	NR 2	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 192	ER 7	EU 0	WL 5	WT 263	WR 0	WU 0	TOTAL 477
11:15 AM 11:30 AM	7 6	0	2 2	0	0	0	0	0	0	231 195	10 7	0	1 4	236 245	0	0	487 459
11:45 AM 12:00 PM	12 7	0		0	0	0	0	0	0	215 215	7	0	6	270 222	0	0	512 462
12:15 PM	5	0	5	0	0	0	0	Ō	0	211	8 5	Ö	4	240	0	0	473
12:30 PM 12:45 PM	6 8	0	2	0	0	0	0	0	0	243 225	11	0	6 2	261 255	0	0	525 504
1:00 PM 1:15 PM	6 5	0 0	3 2	0 0	0	0 0	0 0	0 0	0	252 251	19 9	0 0	3 3	226 232	0 0	0 0	509 502
1:30 PM 1:45 PM	8 6	0	1 8	0	0	0	0	0	0	247 239	12 14	0 0	4 4	256 265	0	0 0	528 536
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES : APPROACH %'s :	84 69.42%	0 0.00%	37 30.58%	0 0.00%	0	0	0	0	0 0.00%	2716 95.73%	120 4.23%	1 0.04%	44 1.46%	2971 98.51%	0 0.00%	1 0.03%	5974
PEAK HR : PEAK HR VOL :	25	01:00 PM - 0	02:00 PM 14	0	0	0	0	0	0	989	54	0	14	979	0	0	TOTAL 2075
PEAK HR FACTOR :	0.781	0.000	0.438 96	0.000	0.000	0.000	0.000	0.000	0.000	0.981 0.96	0.711 52	0.000	0.875	0.924 0.92	0.000 23	0.000	0.968
		NORTH	BOUND				HBOUND			EASTB	OUND			WESTE			
PM	1 NL	0 NT	1 NR	0 NU	0 SL	0 ST	0 SR	0 SU	0 EL	2 ET	1 ER	0 EU	0 WL	2 WT	0 WR	0 WU	TOTAL
3:00 PM 3:15 PM	7 8	0	3 1	0	0	0	0	0	0	342 379	17 9	0	1 4	238 249	0	0	608 650
3:30 PM 3:45 PM	11 11	0	6	0	0	0	0	0	0	403 354	20 16	0	2	191 223	0	0	633 614
4:00 PM 4:15 PM	15 7	0	3	0	0	0	0	0	0	343 330	17 23	0	6	227 302	0	0	611 667
4:13 PM 4:30 PM 4:45 PM	11 15	0	4	0	0	0	0	0	0	331 352	23 23 24	0	3	234 239	0	1	607 637
5:00 PM	14	0	3	0	0	0	0	0	0	317	29	0	6	254	0	0	623
5:15 PM 5:30 PM	8 9	0	4 1	0	0	0 0	0 0	0 0	0	350 381	16 17	0 0	1 1	263 322	0 0	0 0	642 731
5:45 PM	12	0	2	0	0	0	0	0	0	350	17	0	4	327	0	0	712
TOTAL VOLUMES :	NL 128	NT 0	NR 41	NU 0 0.00%	SL 0	ST 0	SR 0	SU 0	EL 0	ET 4232	ER 228	EU 0 0.00%	WL 36 1.16%	WT 3069 98.81%	WR 0	WU 1 0.03%	TOTAL 7735
APPROACH %'s : PEAK HR :		0.00% 05:00 PM -			0	0	0	0	0.00%	94.89%	5.11%				0.00%		TOTAL
PEAK HR VOL : PEAK HR FACTOR :	43 0.768	0.000	10 0.625	0 0.000	0.000	0 0.000	0 0.000	0 0.000	0.000	1398 0.917	79 0.681	0 0.000	12 0.500	1166 0.891	0.000	0 0.000	2708 0.926
		0.7	/9							0.92	20			0.89	JU		

Birch Rd & N Druid Hills Rd



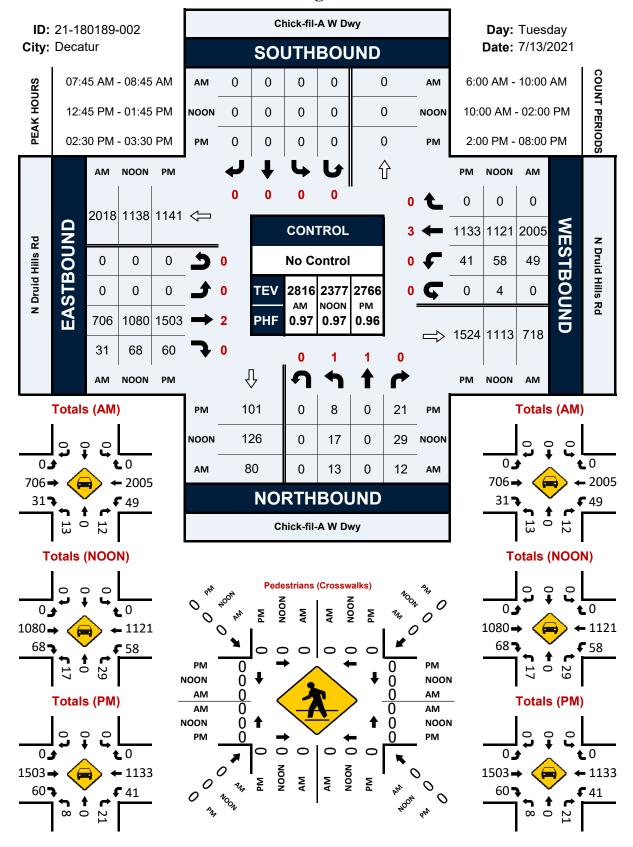
${\tt National\ Data\ \&\ Surveying\ Services} \\ Intersection\ Turning\ Movement\ Count$

Location: Chick-fil-A W Dwy & N Druid Hills Rd City: Decatur Control: No Control

Project ID: 21-180189-002 Date: 7/13/2021

Control.	NO CONTROL					Data - Totals								Date. 7	/13/2021		
NS/EW Streets:		Chick-fil-A					A W Dwy			N Druid H	lills Rd			N Druid I			
AM	1	NORTH 1	BOUND 0	0	0	SOUTI 0	HBOUND 0	0	0	EASTB0	OUND 0	0	0	WESTB 3	OUND 0	0	
6:00 AM	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 47	ER 0	EU 0	WL 9	WT 187	WR 0	WU 0	TOTAL 243
6:15 AM 6:30 AM	1 2	0	1	0	0	0	0	0	0	76 83	7	0	6 15	274 315	0	0	365 419
6:45 AM 7:00 AM	4	0	3	0	0	0 0	0	0	0	112 107	5	0	11 19	339 363	0	0	474 497
7:15 AM	3	0	0	0	0	0	0	0	0	125	6	0	10	404	0	0	548
7:30 AM 7:45 AM	4 3	0	2	0	0	0	0	0	0	167 171	15 10	0	16 6	470 526	0 0	0 0	674 718
8:00 AM 8:15 AM	5 4	0 0	4 3	0 0	0	0 0	0 0	0 0	0	177 177	10 1	0	16 10	517 497	0 0	0 0	729 692
8:30 AM 8:45 AM	1 3	0 0	3 5	0	0	0	0	0	0	181 183	10 12	0	17 17	465 477	0 0	0	677 697
9:00 AM 9:15 AM	3	0	4	0	0	0	0	0	0	191 181	10 10	0	13 16	363 386	0	0	584 600
9:30 AM 9:45 AM	5	0	5	0	0	0	0	0	0	168 208	17 13	0	20 10	365 351	0	0	580 588
	NL	NT	NR 42	NU	SL 0	ST	SR	SU	EL	ET 2254	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES : APPROACH %'s : PEAK HR :	45 51.14%	0 0.00% 07:45 AM -	43 48.86% 08:45 AM	0.00%	U	0	0	0	0 0.00%	2354 94.65%	133 5.35%	0 0.00%	211 3.24%	6299 96.76%	0 0.00%	0 0.00%	9085 TOTAL
PEAK HR VOL : PEAK HR FACTOR :	13 0.650	0 0.000	12 0.750	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	706 0.975	31 0.775	0 0.000	49 0.721	2005 0.953	0 0.000	0 0.000	2816 0.966
		0.6				COLITI	IDOLIND			0.96				0.96			0.500
NOON	1	1	BOUND 0	0	0	0	HBOUND 0	0	0	EASTBO	0	0	0	WESTB 3	0	0	TOTAL
10:00 AM	NL 6	NT 0	NR 7	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	172	ER 7	EU 0	WL 16	WT 259	WR 0	WU 0	TOTAL 467
10:15 AM 10:30 AM	7 4	0 0	3 4	0	0	0 0	0 0	0 0	0 0	187 226	17 9	0	10 13	259 277	0 0	0 0	483 533
10:45 AM 11:00 AM	3 2	0	4	0	0	0	0	0	0	196 220	10 10	0	12 18	258 230	0	0	483 482
11:15 AM 11:30 AM	5 3	0	6 7	0	0	0	0	0	0	228 254	14 15	0	13 15	259 281	0 0	1 0	526 575
11:45 AM 12:00 PM	1 3	0	9 7	0	0	0	0	0	0	232 229	14 10	0	24 16	247 250	0	0	527 516
12:15 PM 12:30 PM	2	0	5	0	0	0	0	0	0	258 259	16 16	0	12 22	278 237	0	0	571 543
12:45 PM 1:00 PM	4 3	0	7	0	0	0	0	0	0	291 266	17 18	0	11 18	268 269	0	3	601 582
1:15 PM 1:30 PM	9	0	10 5	0	0	0	0	0	0	273 250	18 15	0	10 19	294 290	0	0	614 580
1:45 PM	3	0	8	0	0	0	0	0	0	274	20	0	8	280	0	0	593
TOTAL VOLUMES :	NL 59	NT 0	NR 97	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 3815	ER 226	EU 0	WL 237	WT 4236	WR 0	WU 6	TOTAL 8676
APPROACH %'s : PEAK HR :	37.82%	0.00% 12:45 PM -	62.18% 01:45 PM	0.00%					0.00%	94.41%	5.59%	0.00%	5.29%	94.57%	0.00%	0.13%	TOTAL
PEAK HR VOL : PEAK HR FACTOR :	17 0.472	0.000	29 0.725	0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	1080 0.928	68 0.944	0 0.000	58 0.763	1121 0.953	0 0.000	4 0.333	2377
		0.6	05							0.93				0.95	57		0.968
PM	1	1	BOUND 0	0	0	0	HBOUND 0	0	0	EASTB0	OUND 0	0	0	WESTE 3	0	0	
2:00 PM	NL 3	NT 0	NR 5	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	518	ER 8	EU 0	WL 18	WT 225	WR 0	WU 0	TOTAL 577
2:15 PM 2:30 PM	1 2	0	10 8	0	0	0	0	0	0 0	328 392	15 15	0	10 14	272 289	0	0	636 720
2:45 PM 3:00 PM	1 2	0	8	0	0	0	0	0	0	355 367	14 16	0	9 8	273 271	0	0	660 665
3:15 PM 3:30 PM	3 2	0	4 5	0	0	0	0	0	0	389 356	15 8	0	10 12	300 252	0	0 1	721 636
3:45 PM 4:00 PM	1 2	0	1 3	0	0	0	0	0	0	378 391	13 10	0	7	247 284	0	0	647 694
4:15 PM	1	Ō	5	0	Ō	0	0	0	0	373	9	1	9	287	Ō	0	685
4:30 PM 4:45 PM	0 4	0	4	0	0	0	0	0	0	366 381	12 9	0	9	249 258	0	0	641 659
5:00 PM 5:15 PM	1	0 0	4 4	0 0	0	0 0	0 0	0 0	0	394 352	9 16	0	7 8	277 270	0 0	0 0	693 651
5:30 PM 5:45 PM	2 0	0	4 0	0	0	0	0	0	0 0	374 394	8 13	0	5 4	257 256	0 0	0 0	650 667
6:00 PM 6:15 PM	0	0	3	0	0	0	0	0	0	369 353	12 10	0	4 7	238 234	0	0	626 609
6:30 PM 6:45 PM	4	0	4 2	0	0	0	0	0	0	378 322	14 11	0	6 14	241 229	0	0	647 581
7:00 PM 7:15 PM	1 3	0	3 2	0	0	0	0	0	0	290 286	19 10	0	14 6	179 208	0	0	506 515
7:30 PM 7:45 PM	3	0	6 2	0	0	0	0	0	0	276 257	13 12	0	13 11	193 169	0	1 0	505 451
TOTAL VOLUMES :	NL 43	NT 0	NR 94	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 8439	ER 291	EU 2	WL 213	WT 5958	WR 0	WU 2	TOTAL 15042
APPROACH %'s :	31.39%	0.00% 02:30 PM -	68.61%	0.00%	0	3	•	,	0.00%	96.64%	3.33%	0.02%	3.45%	96.52%	0.00%	0.03%	TOTAL
PEAK HR VOL : PEAK HR FACTOR :	8 0.667	0.000	21 0.656	0 0.000	0 0.000	0.000	0.000	0 0.000	0 0.000	1503 0.959	60 0.938	0.000	41 0.732	1133 0.944	0.000	0.000	2766 0.959
		0.7	23							0.96	U			0.94	1		

Chick-fil-A W Dwy & N Druid Hills Rd



${\tt National\ Data\ \&\ Surveying\ Services} \\ Intersection\ Turning\ Movement\ Count$

Location: Chick-fil-A E Dwy & N Druid Hills Rd City: Decatur Control: 1-Way Stop(SB)

Project ID: 21-180189-003 Date: 7/13/2021

Control.	1-way Stop	(30)						Data -	Totals				Date. 7	/13/2021			
NS/EW Streets:		Chick-fil-A				Chick-fil-A				N Druid H				N Druid I			
AM	0.5	NORTH 0	BOUND 0.5	0	0	SOUTHI 2	BOUND 0	0	0	EASTB0	OUND 0	0	0	WESTB 3	OUND 0	0	
6:00 AM	NL 1	NT 0	NR 5	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 48	ER 0	EU 0	WL 0	WT 199	WR 1	WU 0	TOTAL 254
6:15 AM 6:30 AM	2	1	9	0	0	0	0	0	0	76 85	0	0	0	274 334	0	0	362 434
6:45 AM 7:00 AM	2	0	9	0	1 0	0	1 2	0	1 2	111 110	0	0	0	339 376	6 8	0	470 512
7:15 AM	4	0	13	0	0	0	0	0	0	126	Ō	0	0	420	6	0	569
7:30 AM 7:45 AM	3 1	0	12 14	0	0 2	0	3 5	0 0	2 1	163 176	0	0	0	470 526	8 9	1 0	662 734
8:00 AM 8:15 AM	3 1	1 0	10 9	0	1 0	0 0	3 0	0 0	0 1	181 179	0 0	0	0 0	529 504	12 10	0 1	740 705
8:30 AM 8:45 AM	1 2	0	16 13	0	0 0	0	1 2	0	1	181 188	0 0	0	0 0	480 491	10 15	0	690 712
9:00 AM 9:15 AM	2 2	0	18 15	0	2 2	0	6	0	0	196 185	0	0	0	367 396	16 16	0	607 621
9:30 AM 9:45 AM	3 1	0	10 19	0	2 2	0 0	5 3	1 0	2 4	170 208	0	0	0	381 353	8 21	0	582 611
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES : APPROACH %'s :	34 15.04%	3 1.33%	189 83.63%	0 0.00%	12 24.00%	0 0.00%	37 74.00%	1 2.00%	15 0.63%	2383 99.37%	0 0.00%	0 0.00%	0 0.00%	6439 97.69%	149 2.26%	3 0.05%	9265
PEAK HR : PEAK HR VOL :	6	07:45 AM - 1	08:45 AM 49	0	3	0	9	0	3	717	0	0	0	2039	41	1	TOTAL 2869
PEAK HR FACTOR :	0.500	0.250 0.8	0.766 24	0.000	0.375	0.000 0.42	0.450 29	0.000	0.750	0.990 0.98	0.000	0.000	0.000	0.964 0.96	0.854 52	0.250	0.969
NOON		NORTH				SOUTH				EASTBO				WESTE			
NOON	0.5 NL	0 NT	0.5 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	3 WT	0 WR	0 WU	TOTAL
10:00 AM 10:15 AM	1 2	0 0	17 13	0	5 3	0 0	2 5	0 0	0 1	174 193	1 0	0	0 0	271 262	21 19	0 0	492 498
10:30 AM 10:45 AM	4 4	1 0	16 13	0	7 6	0 0	8	0 0	2 0	228 200	0 0	0	0 0	278 263	17 13	0 0	561 502
11:00 AM 11:15 AM	2 1	0	8 22	0	3 1	0 0	7 5	0 0	1 3	214 236	0 0	0 2	0 0	239 274	26 17	0 0	500 561
11:30 AM 11:45 AM	2	0	21 21	0	5 1	0	6 10	0 1	1 2	261 239	0	0	0	279 261	18 21	0	593 557
12:00 PM 12:15 PM	4 2	0	13 28	0	7	0	2 7	0	2	233 262	0	0	0	268 274	32 28	0	561 610
12:30 PM 12:45 PM	2 2	0	15 15	0	6	0	7 15	0	1	264 289	0	0	0	251 264	21 21	0	567 613
1:00 PM	2	0	18	0	2	0	10	0	3	279	0	0	0	277	25	0	616
1:15 PM 1:30 PM	2	0	21 20	0	6	0	9 10	0	2	274 260	0	0	0	292 304	22 28	0	626 632
1:45 PM	1	0	18	0	6	0	5	0	3	278	1	0	0	275	33	0	620
TOTAL VOLUMES :	NL 33	NT 2	NR 279	O NU	SL 69	ST 0	SR 111	SU 2	EL 30	ET 3884	ER 2	EU 2	WL 0	WT 4332	WR 362	WU 1	TOTAL 9109
APPROACH %'s : PEAK HR :		0.64% 01:00 PM -		0.00%	37.91%	0.00%	60.99%	1.10%	0.77%	99.13%	0.05%	0.05%	0.00%	92.27%	7.71%	0.02%	TOTAL
PEAK HR VOL : PEAK HR FACTOR :	7 0.875	0 0.000	77 0.917	0.000	18 0.750	0.000	34 0.850	0 0.000	10 0.833	1091 0.978	1 0.250	0 0.000	0 0.000	1148 0.944	108 0.818	0 0.000	2494 0.987
		0.9				0.81				0.97				0.94			0.307
PM	0.5	NORTH 0	0.5	0	0	SOUTH!	0	0	0	EASTB0	0	0	0	WESTE 3	0	0	
2:00 PM	NL 0	NT 0	NR 20	NU 0	SL 4	ST 0	SR 8	SU 0	EL 1	320	ER 0	EU 0	WL 0	WT 231	WR 19	WU 1	TOTAL 604
2:15 PM 2:30 PM	0 0	1 0	12 12	0	6 4	0 0	10 8	0	2 1	338 399	0 0	0	0 0	275 294	18 13	0	662 731
2:45 PM 3:00 PM	0	0	14 20	0	4 2	0	<u>7</u>	0	0	360 370	0	0	0	272 281	19 28	0	678 707
3:15 PM 3:30 PM	1	0	20 19	0	2 6	0	7 10	0	0	393 360	0	0	0	294 255	21 14	0	738 666
3:45 PM 4:00 PM	0	0	16 18	0	1 7	0	4	0	5 3	367 399	0	0	0	250 281	31 25	0	674 745
4:15 PM 4:30 PM	1 0	0	10	0	7 7	0	2 7	0	1	377	0	Ō	0	288 251	28 28	0	714 672
4:45 PM	0	Ō	12 9	Ō	6	0	9	Ō	1	366 390	Ō	0	0	261	16	0	692
5:00 PM 5:15 PM	0	0	12 15	0	8	0	6 5	0	1	393 346	0	0	0	269 273	24 29	0	714 677
5:30 PM 5:45 PM	0	0	15 14	0	8	0	14 4	0	0	374 403	0	0	0	252 251	25 21	0	682 702
6:00 PM 6:15 PM	1 0	0 0	9 11	0	6 6	0 0	7 9	0	1	366 356	0 0	0	0 0	240 226	25 23	0	655 632
6:30 PM 6:45 PM	1 1	0 0	17 13	0	4 8	0 0	6 4	0 0	1 0	385 324	0 0	0 0	0	245 233	34 27	0 0	693 610
7:00 PM 7:15 PM	2 2	0	16 20	0	4 8	0	0 6	0	0 1	287 293	0	0	0	191 217	18 24	0	518 572
7:30 PM 7:45 PM	2	0	16 17	0	4 7	0	4 7	0	2	281 258	0	0	0	190 173	14 13	0	513 477
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES : APPROACH %'s : PEAK HR :	16 4.26%	3 0.80% 02:30 PM -	357 94.95%	0.00%	126 43.90%	0 0.00%	161 56.10%	0 0.00%	29 0.34%	8505 99.66%	0.00%	0 0.00%	0.00%	5993 91.76%	537 8.22%	1 0.02%	15728 TOTAL
PEAK HR VOL : PEAK HR FACTOR :	1 0.250	1 0.250	66 0.825	0 0.000	12 0.750	0 0.000	28 0.875	0 0.000	2 0.500	1522 0.954	0 0.000	0 0.000	0 0.000	1141 0.970	81 0.723	0 0.000	2854
		0.8			550	0.83				0.95				0.97		,	0.967

Chick-fil-A E Dwy & N Druid Hills Rd

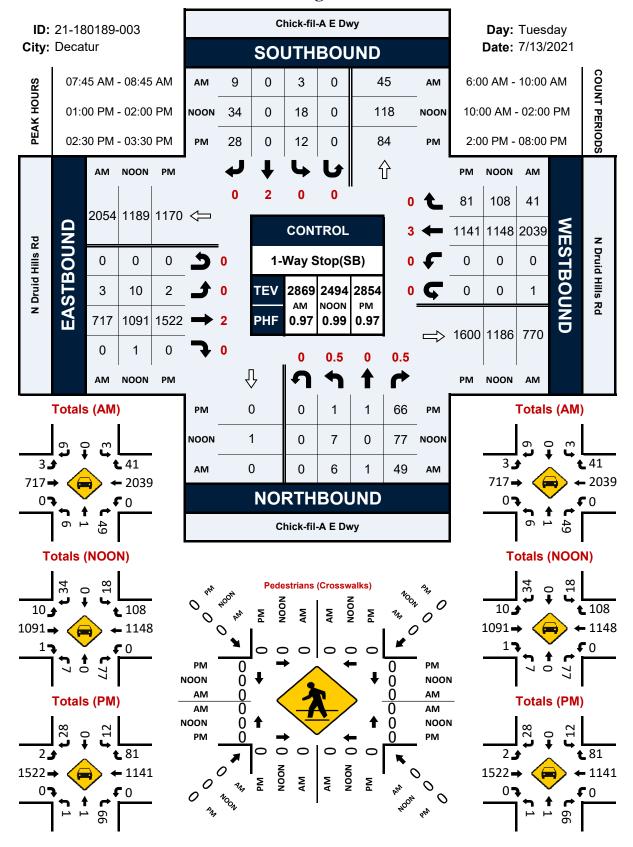


Figure 1: Peak Hour Turning Movement Counts

(##) AM (PM) Traffic Volumes

AM peak hour of driveways 7:45-8:45am PM peak hour of driveways 2:30-3:30pm



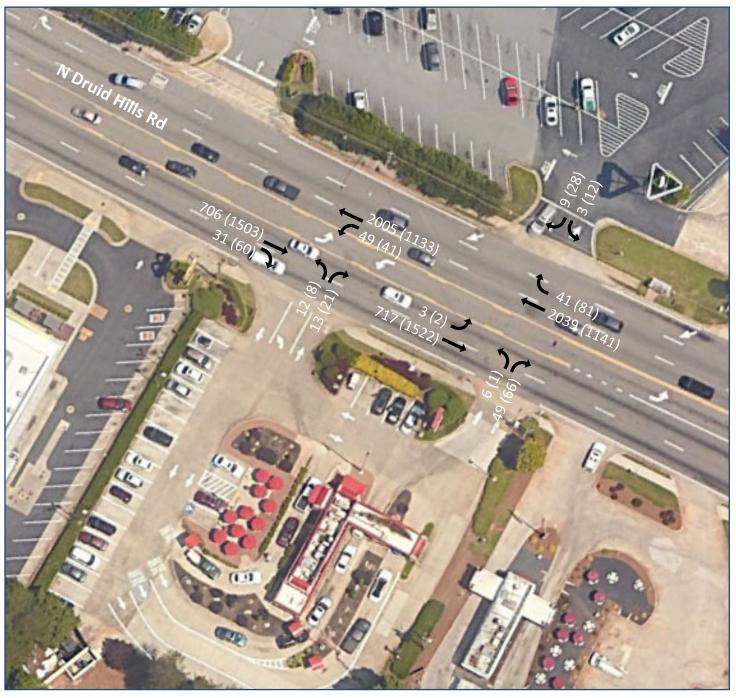


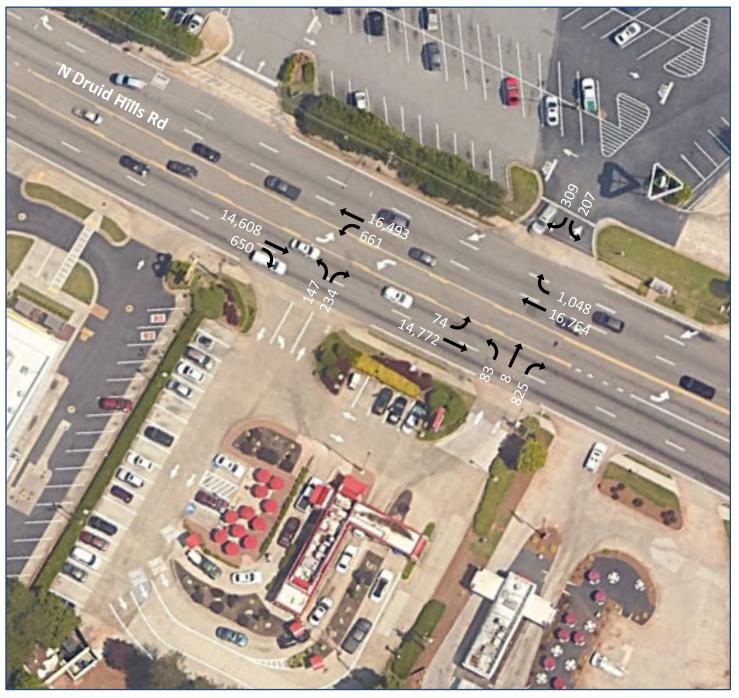


Figure 2: 14-Hour Turning Movement Counts

(##) AM (PM) Traffic Volumes

14-hour counts collected 6:00am to 8:00pm







Appendix B: Synchro Analysis



	→	•	•	•	4	/
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414	ሻ	7
Traffic Volume (veh/h)	737	32	12	2030	25	3
Future Volume (veh/h)	737	32	12	2030	25	3
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	801	35	13	2207	27	3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3195	1425	34	3113	53	47
Arrive On Green	0.90	0.90	0.90	0.90	0.03	0.03
Sat Flow, veh/h	3647	1585	9	3548	1781	1585
Grp Volume(v), veh/h	801	35	1191	1029	27	3
Grp Sat Flow(s), veh/h/ln	1777	1585	1854	1617	1781	1585
Q Serve(g_s), s	4.1	0.3	0.0	24.7	2.1	0.3
Cycle Q Clear(g_c), s	4.1	0.3	24.8	24.7	2.1	0.3
Prop In Lane		1.00	0.01	_ 1.1	1.00	1.00
Lane Grp Cap(c), veh/h	3195	1425	1693	1454	53	47
V/C Ratio(X)	0.25	0.02	0.70	0.71	0.51	0.06
Avail Cap(c_a), veh/h	3195	1425	1693	1454	254	226
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	0.9	0.7	2.0	2.0	66.9	66.1
Incr Delay (d2), s/veh	0.9	0.0	2.5	2.0	7.6	0.6
Initial Q Delay(d3),s/veh	0.2	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.0	3.1	2.9	1.1	0.0
Unsig. Movement Delay, s/veh		0.0	J. I	2.3	1.1	0.1
LnGrp Delay(d),s/veh	1.1	0.8	4.4	4.9	74.5	66.6
LnGrp Delay(d),s/ven LnGrp LOS	1.1 A				74.5 E	00.0 E
		A	A	A		
Approach Vol, veh/h	836			2220	30	
Approach Delay, s/veh	1.1			4.7	73.7	
Approach LOS	Α			Α	Е	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		130.9		9.1		130.9
Change Period (Y+Rc), s		5.0		5.0		5.0
Max Green Setting (Gmax), s		110.0		20.0		110.0
Max Q Clear Time (g_c+l1), s		26.8		4.1		6.1
Green Ext Time (p_c), s		43.5		0.0		6.4
Intersection Summary						
HCM 6th Ctrl Delay			4.4			
HCM 6th LOS						
HOW BUILDS			Α			

Existing AM 08/10/2021 Synchro 10 Report Page 1

Intersection: 1: Birch Rd & North Druid Hills

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	T	L	R
Maximum Queue (ft)	96	52	22	205	179	90	12
Average Queue (ft)	17	5	1	59	44	24	1
95th Queue (ft)	59	26	10	157	130	67	5
Link Distance (ft)	804	804		1048	1048	311	311
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		0					
Queuing Penalty (veh)		0					

Network Summary

Network wide Queuing Penalty: 0

Existing AM SimTraffic Report
Page 1

	→	•	•	←	•	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414		7
Traffic Volume (veh/h)	989	54	14	979	25	14
Future Volume (veh/h)	989	54	14	979	25	14
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	1075	59	15	1064	27	15
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3177	1417	48	3037	61	55
Arrive On Green	0.89	0.89	0.89	0.89	0.03	0.03
Sat Flow, veh/h	3647	1585	24	3482	1781	1585
Grp Volume(v), veh/h	1075	59	569	510	27	15
Grp Sat Flow(s), veh/h/ln	1777	1585	1804	1617	1781	1585
Q Serve(g_s), s	6.4	0.6	0.0	6.8	2.1	1.3
Cycle Q Clear(g_c), s	6.4	0.6	6.5	6.8	2.1	1.3
Prop In Lane		1.00	0.03		1.00	1.00
Lane Grp Cap(c), veh/h	3177	1417	1639	1446	61	55
V/C Ratio(X)	0.34	0.04	0.35	0.35	0.44	0.27
Avail Cap(c_a), veh/h	3177	1417	1639	1446	254	226
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	1.1	0.8	1.1	1.1	66.3	65.9
Incr Delay (d2), s/veh	0.3	0.1	0.6	0.7	4.9	2.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	0.1	0.9	0.9	1.0	0.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	1.4	0.9	1.7	1.8	71.1	68.5
LnGrp LOS	Α	A	A	A	E	E
Approach Vol, veh/h	1134			1079	42	_
Approach Delay, s/veh	1.4			1.8	70.2	
Approach LOS	A			A	E	
	А	•				
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		130.2		9.8		130.2
Change Period (Y+Rc), s		5.0		5.0		5.0
Max Green Setting (Gmax), s		110.0		20.0		110.0
Max Q Clear Time (g_c+I1), s		8.8		4.1		8.4
Green Ext Time (p_c), s		8.7		0.1		10.0
Intersection Summary						
HCM 6th Ctrl Delay			2.8			
HCM 6th LOS			A			
			/ \			

Existing MD 08/10/2021 Synchro 10 Report Page 1

Intersection: 1: Birch Rd & North Druid Hills

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	T	L	R
Maximum Queue (ft)	109	76	21	146	109	84	25
Average Queue (ft)	27	14	2	41	13	23	2
95th Queue (ft)	81	51	12	112	59	61	13
Link Distance (ft)	804	804		1048	1048	311	311
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		0					
Queuing Penalty (veh)		0					

Network Summary

Network wide Queuing Penalty: 0

Existing MD SimTraffic Report
Page 1

	-	•	•	•	^	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414		7
Traffic Volume (veh/h)	1398	79	12	1166	43	10
Future Volume (veh/h)	1398	79	12	1166	43	10
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	1520	86	13	1267	47	11
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3164	1411	39	3029	68	61
Arrive On Green	0.89	0.89	0.89	0.89	0.04	0.04
Sat Flow, veh/h	3647	1585	14	3488	1781	1585
Grp Volume(v), veh/h	1520	86	676	604	47	11
Grp Sat Flow(s), veh/h/ln	1777	1585	1800	1617	1781	1585
Q Serve(g_s), s	11.5	0.9	0.0	9.2	3.6	0.9
Cycle Q Clear(g_c), s	11.5	0.9	8.7	9.2	3.6	0.9
Prop In Lane	. 1.0	1.00	0.02	7.2	1.00	1.00
Lane Grp Cap(c), veh/h	3164	1411	1629	1439	68	61
V/C Ratio(X)	0.48	0.06	0.41	0.42	0.69	0.18
Avail Cap(c_a), veh/h	3164	1411	1629	1439	254	226
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	1.5	0.9	1.3	1.3	66.5	65.2
Incr Delay (d2), s/veh	0.5	0.1	0.8	0.9	11.6	1.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.5	0.1	1.3	1.3	1.9	0.4
Unsig. Movement Delay, s/vel		0.1	1.0	1.0	1.0	0.1
LnGrp Delay(d),s/veh	2.0	1.0	2.1	2.2	78.1	66.6
LnGrp LOS	Α	A	A	A	E	E
Approach Vol, veh/h	1606			1280	58	
Approach Delay, s/veh	1.9			2.2	75.9	
	Α			Α.Δ	7 J. 9	
Approach LOS	А			А		
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		129.6		10.4		129.6
Change Period (Y+Rc), s		5.0		5.0		5.0
Max Green Setting (Gmax), s		110.0		20.0		110.0
Max Q Clear Time (g_c+l1), s	;	11.2		5.6		13.5
Green Ext Time (p_c), s		11.9		0.1		19.1
Intersection Summary						
HCM 6th Ctrl Delay			3.5			
HCM 6th LOS			3.5 A			
I IOW OUI LOS			А			

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Intersection: 1: Birch Rd & North Druid Hills

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	Т	T	R	LT	T	L	R
Maximum Queue (ft)	159	158	52	278	240	85	25
Average Queue (ft)	56	36	7	72	43	37	2
95th Queue (ft)	131	103	33	198	148	74	13
Link Distance (ft)	804	804		1048	1048	311	311
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		1					
Queuing Penalty (veh)		1					

Network Summary

Network wide Queuing Penalty: 1

Existing PM SimTraffic Report
Page 1

Movement
Lane Configurations
Traffic Volume (veh/h) 737 59 61 2030 46 20 Future Volume (veh/h) 737 59 61 2030 46 20 Initial Q (Qb), veh 0 0 0 0 0 0 Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 Work Zone On Approach No No No No Adj Sat Flow, veh/h/ln 1870 1870 1870 1870 1870 Adj Flow Rate, veh/h 801 64 66 2207 50 22 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 Percent Heavy Veh, % 2<
Future Volume (veh/h) 737 59 61 2030 46 20 Initial Q (Qb), veh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Q (Qb), veh
Ped-Bike Adj(A_pbT) 1.00 </td
Work Zone On Approach No No No No Adj Sat Flow, veh/h/ln 1870 22 2
Adj Sat Flow, veh/h/ln 1870 22 22 22 22 0.92 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04
Adj Sat Flow, veh/h/ln 1870 22 22 22 22 0.92 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04
Peak Hour Factor 0.92 0.93 0.89 0.84 64 420 0.84 64 1.08 0.80 0.89 0.89 0.84 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04
Peak Hour Factor 0.92 0.94 0.02 0.04 0.04 0.04 0.04 0.04 0.04
Cap, veh/h 3157 1408 95 2898 72 64 Arrive On Green 0.89 0.89 0.89 0.89 0.04 0.04 Sat Flow, veh/h 3647 1585 77 3347 1781 1585 Grp Volume(v), veh/h 801 64 1220 1053 50 22 Grp Sat Flow(s), veh/hIn 1777 1585 1722 1617 1781 1585 Q Serve(g_s), s 4.5 0.7 12.8 29.2 3.9 1.9 Cycle Q Clear(g_c), s 4.5 0.7 12.8 29.2 3.9 1.9 Prop In Lane 1.00 0.05 1.00 1.00 Lane Grp Cap(c), veh/h 3157 1408 1557 1436 72 64 V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00
Arrive On Green 0.89 0.89 0.89 0.89 0.04 0.04 Sat Flow, veh/h 3647 1585 77 3347 1781 1585 Grp Volume(v), veh/h 801 64 1220 1053 50 22 Grp Sat Flow(s), veh/h/ln 1777 1585 1722 1617 1781 1585 Q Serve(g_s), s 4.5 0.7 12.8 29.2 3.9 1.9 Cycle Q Clear(g_c), s 4.5 0.7 32.8 29.2 3.9 1.9 Prop In Lane 1.00 0.05 1.00 1.00 Lane Grp Cap(c), veh/h 3157 1408 1557 1436 72 64 V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Arrive On Green 0.89 0.89 0.89 0.89 0.04 0.04 Sat Flow, veh/h 3647 1585 77 3347 1781 1585 Grp Volume(v), veh/h 801 64 1220 1053 50 22 Grp Sat Flow(s), veh/h/ln 1777 1585 1722 1617 1781 1585 Q Serve(g_s), s 4.5 0.7 12.8 29.2 3.9 1.9 Cycle Q Clear(g_c), s 4.5 0.7 32.8 29.2 3.9 1.9 Prop In Lane 1.00 0.05 1.00 1.00 Lane Grp Cap(c), veh/h 3157 1408 1557 1436 72 64 V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Uniform Delay
Grp Volume(v), veh/h 801 64 1220 1053 50 22 Grp Sat Flow(s),veh/h/ln 1777 1585 1722 1617 1781 1585 Q Serve(g_s), s 4.5 0.7 12.8 29.2 3.9 1.9 Cycle Q Clear(g_c), s 4.5 0.7 32.8 29.2 3.9 1.9 Prop In Lane 1.00 0.05 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 3157 1408 1557 1436 72 64 V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Uniform Delay (d), s/veh 1.1 0.9 2.6 2.5 66
Grp Volume(v), veh/h 801 64 1220 1053 50 22 Grp Sat Flow(s),veh/h/ln 1777 1585 1722 1617 1781 1585 Q Serve(g_s), s 4.5 0.7 12.8 29.2 3.9 1.9 Cycle Q Clear(g_c), s 4.5 0.7 32.8 29.2 3.9 1.9 Prop In Lane 1.00 0.05 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 3157 1408 1557 1436 72 64 V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Uniform Delay (d), s/veh 1.1 0.9 2.6 2.5 66
Grp Sat Flow(s),veh/h/ln 1777 1585 1722 1617 1781 1585 Q Serve(g_s), s 4.5 0.7 12.8 29.2 3.9 1.9 Cycle Q Clear(g_c), s 4.5 0.7 32.8 29.2 3.9 1.9 Prop In Lane 1.00 0.05 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 3157 1408 1557 1436 72 64 V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00
Q Serve(g_s), s 4.5 0.7 12.8 29.2 3.9 1.9 Cycle Q Clear(g_c), s 4.5 0.7 32.8 29.2 3.9 1.9 Prop In Lane 1.00 0.05 1.00 1.00 Lane Grp Cap(c), veh/h 3157 1408 1557 1436 72 64 V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00 1.0
Cycle Q Clear(g_c), s 4.5 0.7 32.8 29.2 3.9 1.9 Prop In Lane 1.00 0.05 1.00 1.00 Lane Grp Cap(c), veh/h 3157 1408 1557 1436 72 64 V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00
Prop In Lane 1.00 0.05 1.00 1.00 Lane Grp Cap(c), veh/h 3157 1408 1557 1436 72 64 V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00 <
Lane Grp Cap(c), veh/h 3157 1408 1557 1436 72 64 V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00 1.
V/C Ratio(X) 0.25 0.05 0.78 0.73 0.70 0.34 Avail Cap(c_a), veh/h 3157 1408 1557 1436 254 226 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.
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HCM Platoon Ratio 1.00
Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 1.00 Uniform Delay (d), s/veh 1.1 0.9 2.6 2.5 66.3 65.4 Incr Delay (d2), s/veh 0.2 0.1 4.0 3.4 11.5 3.2 Initial Q Delay(d3),s/veh 0.0 0.8 0.8 0.8
Uniform Delay (d), s/veh 1.1 0.9 2.6 2.5 66.3 65.4 Incr Delay (d2), s/veh 0.2 0.1 4.0 3.4 11.5 3.2 Initial Q Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%),veh/ln 0.6 0.1 5.3 4.4 2.0 0.8 Unsig. Movement Delay, s/veh 1.3 1.0 6.6 5.9 77.9 68.6 LnGrp Delay(d),s/veh 1.3 1.0 6.6 5.9 77.9 68.6 LnGrp LOS A A A A E E Approach Vol, veh/h 865 2273 72 72 75.0 75.0 A A E Timer - Assigned Phs 2 4 6 6 75.0 A A E Timer - Assigned Phs 2 4 6 6 75.0 5.0 5.0 5.0
Incr Delay (d2), s/veh 0.2 0.1 4.0 3.4 11.5 3.2 Initial Q Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%),veh/ln 0.6 0.1 5.3 4.4 2.0 0.8 Unsig. Movement Delay, s/veh 1.3 1.0 6.6 5.9 77.9 68.6 LnGrp Delay(d),s/veh 1.3 1.0 6.6 5.9 77.9 68.6 LnGrp LOS A A A A E E Approach Vol, veh/h 865 2273 72 Approach LOS A A E Timer - Assigned Phs 2 4 6 Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
Initial Q Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%),veh/ln 0.6 0.1 5.3 4.4 2.0 0.8 Unsig. Movement Delay, s/veh LnGrp Delay(d),s/veh 1.3 1.0 6.6 5.9 77.9 68.6 LnGrp LOS A A A A E E Approach Vol, veh/h 865 2273 72 Approach Delay, s/veh 1.3 6.2 75.0 Approach LOS A A E E Timer - Assigned Phs 2 4 6 Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
%ile BackOfQ(50%),veh/ln 0.6 0.1 5.3 4.4 2.0 0.8 Unsig. Movement Delay, s/veh 1.3 1.0 6.6 5.9 77.9 68.6 LnGrp Delay(d),s/veh 1.3 1.0 6.6 5.9 77.9 68.6 LnGrp LOS A A A A E E Approach Vol, veh/h 865 2273 72 Approach Delay, s/veh 1.3 6.2 75.0 Approach LOS A A E Timer - Assigned Phs 2 4 6 Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
Unsig. Movement Delay, s/veh LnGrp Delay(d),s/veh 1.3 1.0 6.6 5.9 77.9 68.6 LnGrp LOS A A A A E E Approach Vol, veh/h 865 2273 72 Approach Delay, s/veh 1.3 6.2 75.0 Approach LOS A A E Timer - Assigned Phs 2 4 6 Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
LnGrp Delay(d),s/veh 1.3 1.0 6.6 5.9 77.9 68.6 LnGrp LOS A A A A E E Approach Vol, veh/h 865 2273 72 Approach Delay, s/veh 1.3 6.2 75.0 Approach LOS A A E Timer - Assigned Phs 2 4 6 Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
LnGrp LOS A A A A E E Approach Vol, veh/h 865 2273 72 Approach Delay, s/veh 1.3 6.2 75.0 Approach LOS A A E Timer - Assigned Phs 2 4 6 Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
Approach Vol, veh/h 865 2273 72 Approach Delay, s/veh 1.3 6.2 75.0 Approach LOS A A E Timer - Assigned Phs 2 4 6 Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
Approach Delay, s/veh 1.3 6.2 75.0 Approach LOS A A E Timer - Assigned Phs 2 4 6 Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
Approach LOS A A E Timer - Assigned Phs 2 4 6 Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
Timer - Assigned Phs 2 4 6 Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
Phs Duration (G+Y+Rc), s 129.4 10.6 129.4 Change Period (Y+Rc), s 5.0 5.0 5.0
Change Period (Y+Rc), s 5.0 5.0 5.0
Max Green Setting (Gmax), s 110.0 20.0 110.0
Max Q Clear Time (g_c+l1), s 34.8 5.9 6.5
Green Ext Time (p_c), s 45.3 0.1 6.5
Intersection Summary
HCM 6th Ctrl Delay 6.5
HCM 6th LOS A

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Intersection: 1: Birch Rd & North Druid Hills

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	T	L	R
Maximum Queue (ft)	101	56	32	304	278	111	23
Average Queue (ft)	32	11	5	127	86	41	2
95th Queue (ft)	85	44	23	262	220	95	13
Link Distance (ft)	804	804		1048	1048	311	311
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		0					
Queuing Penalty (veh)		0					

Network Summary

Network wide Queuing Penalty: 0

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Page 1

	→	•	•	•	•	/
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414	*	7
Traffic Volume (veh/h)	989	115	72	979	51	43
Future Volume (veh/h)	989	115	72	979	51	43
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	1075	125	78	1064	55	47
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3139	1400	184	2460	81	72
Arrive On Green	0.88	0.88	0.88	0.88	0.05	0.05
Sat Flow, veh/h	3647	1585	175	2870	1781	1585
Grp Volume(v), veh/h	1075	125	521	621	55	47
Grp Sat Flow(s), veh/h/ln	1777	1585	1343	1617	1781	1585
Q Serve(g_s), s	7.1	1.4	0.0	10.2	4.3	4.1
Cycle Q Clear(g_c), s	7.1	1.4	6.3	10.2	4.3	4.1
Prop In Lane	1.1	1.00	0.15	13.2	1.00	1.00
Lane Grp Cap(c), veh/h	3139	1400	1216	1428	81	72
V/C Ratio(X)	0.34	0.09	0.43	0.43	0.68	0.66
Avail Cap(c_a), veh/h	3139	1400	1216	1428	254	226
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	1.4	1.00	1.3	1.5	65.8	65.8
Incr Delay (d2), s/veh	0.3	0.1	1.1	1.0	9.7	9.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	0.0	1.3	1.6	2.2	1.9
Unsig. Movement Delay, s/vel		0.2	1.0	1.0	۷.۷	1.9
LnGrp Delay(d),s/veh	1.7	1.2	2.4	2.5	75.6	75.5
LnGrp LOS	1.7 A	1.2 A	2.4 A	2.5 A	75.0 E	75.5 E
	1200	Α	^	1142	102	
Approach Vol, veh/h					75.5	
Approach LOS	1.6			2.5	75.5 E	
Approach LOS	Α			Α	E	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		128.7		11.3		128.7
Change Period (Y+Rc), s		5.0		5.0		5.0
Max Green Setting (Gmax), s		110.0		20.0		110.0
Max Q Clear Time (g_c+l1), s		12.2		6.3		9.1
Green Ext Time (p_c), s		11.1		0.2		10.4
Intersection Summary						
HCM 6th Ctrl Delay			5.1			
HCM 6th LOS			J. 1			
HOW OUT LOS			А			

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Intersection: 1: Birch Rd & North Druid Hills

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	Т	L	R
Maximum Queue (ft)	104	78	47	275	230	110	38
Average Queue (ft)	37	16	11	104	52	40	7
95th Queue (ft)	92	55	35	207	165	88	25
Link Distance (ft)	804	804		1048	1048	311	311
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		0					
Queuing Penalty (veh)		0					

Network Summary

Network wide Queuing Penalty: 0

Build MD SimTraffic Report
Page 1

	→	•	•	•	4	/
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414	*	7
Traffic Volume (veh/h)	1398	132	57	1166	53	34
Future Volume (veh/h)	1398	132	57	1166	53	34
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	1520	143	62	1267	58	37
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3135	1398	122	2489	83	74
Arrive On Green	0.88	0.88	0.88	0.88	0.05	0.05
Sat Flow, veh/h	3647	1585	106	2907	1781	1585
Grp Volume(v), veh/h	1520	143	608	721	58	37
Grp Sat Flow(s), veh/h/ln	1777	1585	1311	1617	1781	1585
Q Serve(g_s), s	12.3	1.6	0.0	13.3	4.5	3.2
Cycle Q Clear(g_c), s	12.3	1.6	8.0	13.3	4.5	3.2
Prop In Lane		1.00	0.10		1.00	1.00
Lane Grp Cap(c), veh/h	3135	1398	1185	1426	83	74
V/C Ratio(X)	0.48	0.10	0.51	0.51	0.70	0.50
Avail Cap(c_a), veh/h	3135	1398	1185	1426	254	226
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	1.7	1.1	1.4	1.8	65.8	65.2
Incr Delay (d2), s/veh	0.5	0.1	1.6	1.3	10.3	5.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.9	0.2	1.6	2.1	2.3	1.4
Unsig. Movement Delay, s/veh		V. L				
LnGrp Delay(d),s/veh	2.2	1.2	3.0	3.0	76.1	70.4
LnGrp LOS	A	A	Α	A	7 O.1	E
Approach Vol, veh/h	1663	, , , , , , , , , , , , , , , , , , ,	,,	1329	95	
Approach Delay, s/veh	2.2			3.0	73.9	
Approach LOS	Z.Z A			3.0 A	7 5.9 E	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		128.5		11.5		128.5
Change Period (Y+Rc), s		5.0		5.0		5.0
Max Green Setting (Gmax), s		110.0		20.0		110.0
Max Q Clear Time (g_c+I1), s		15.3		6.5		14.3
Green Ext Time (p_c), s		15.6		0.2		19.7
Intersection Summary						
HCM 6th Ctrl Delay			4.7			
HCM 6th LOS			A			
00. 200			, ,			

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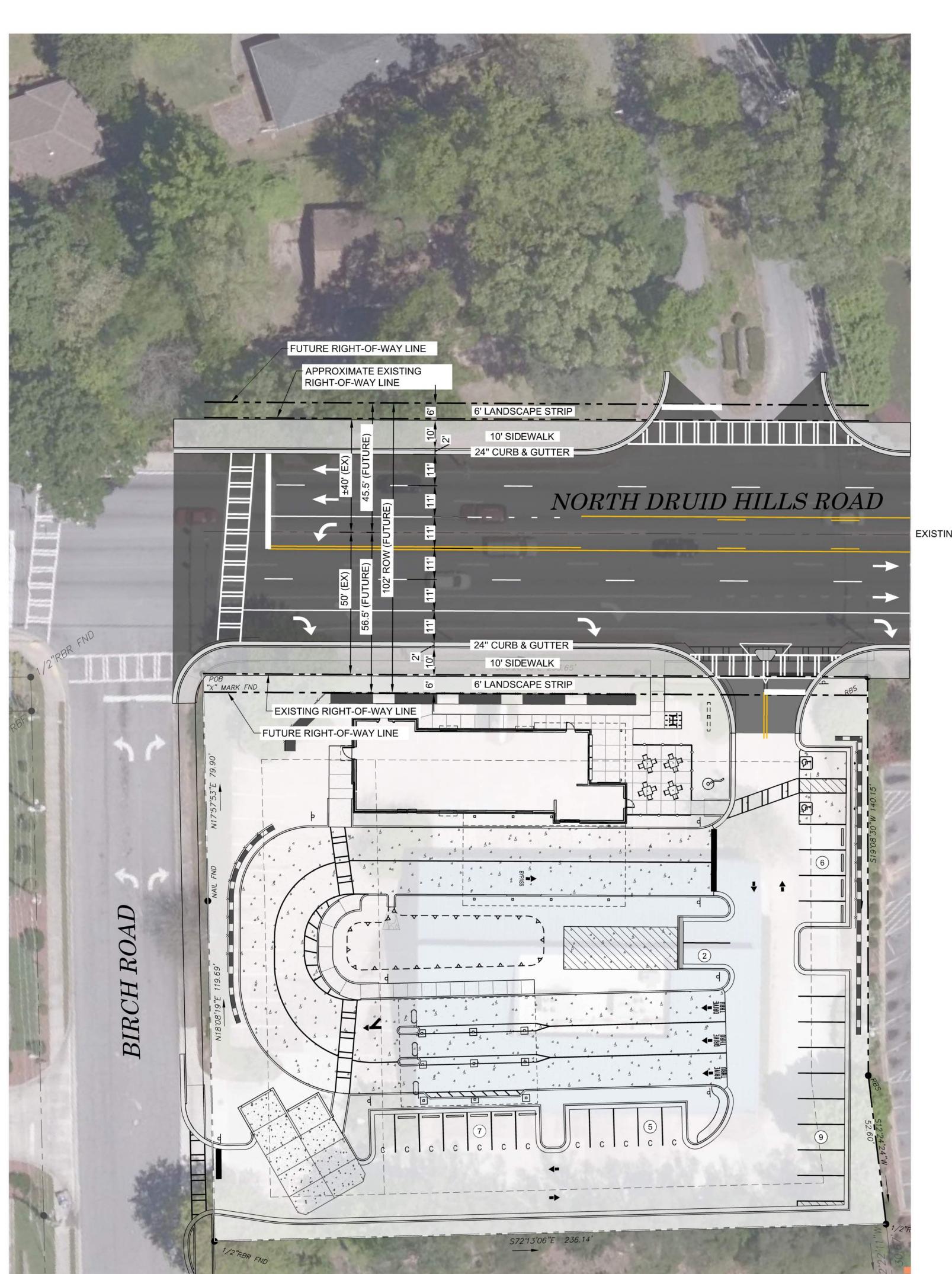
Intersection: 1: Birch Rd & North Druid Hills

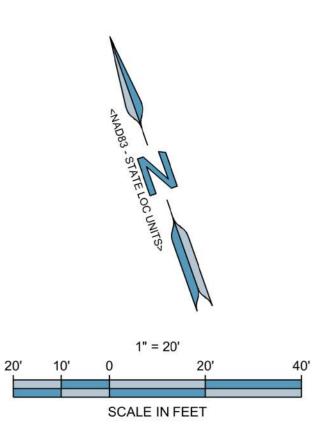
Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	Т	L	R
Maximum Queue (ft)	132	128	48	317	280	104	48
Average Queue (ft)	56	31	11	150	104	43	10
95th Queue (ft)	125	87	35	289	259	90	32
Link Distance (ft)	804	804		1048	1048	311	311
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		1	0				
Queuing Penalty (veh)		1	0				

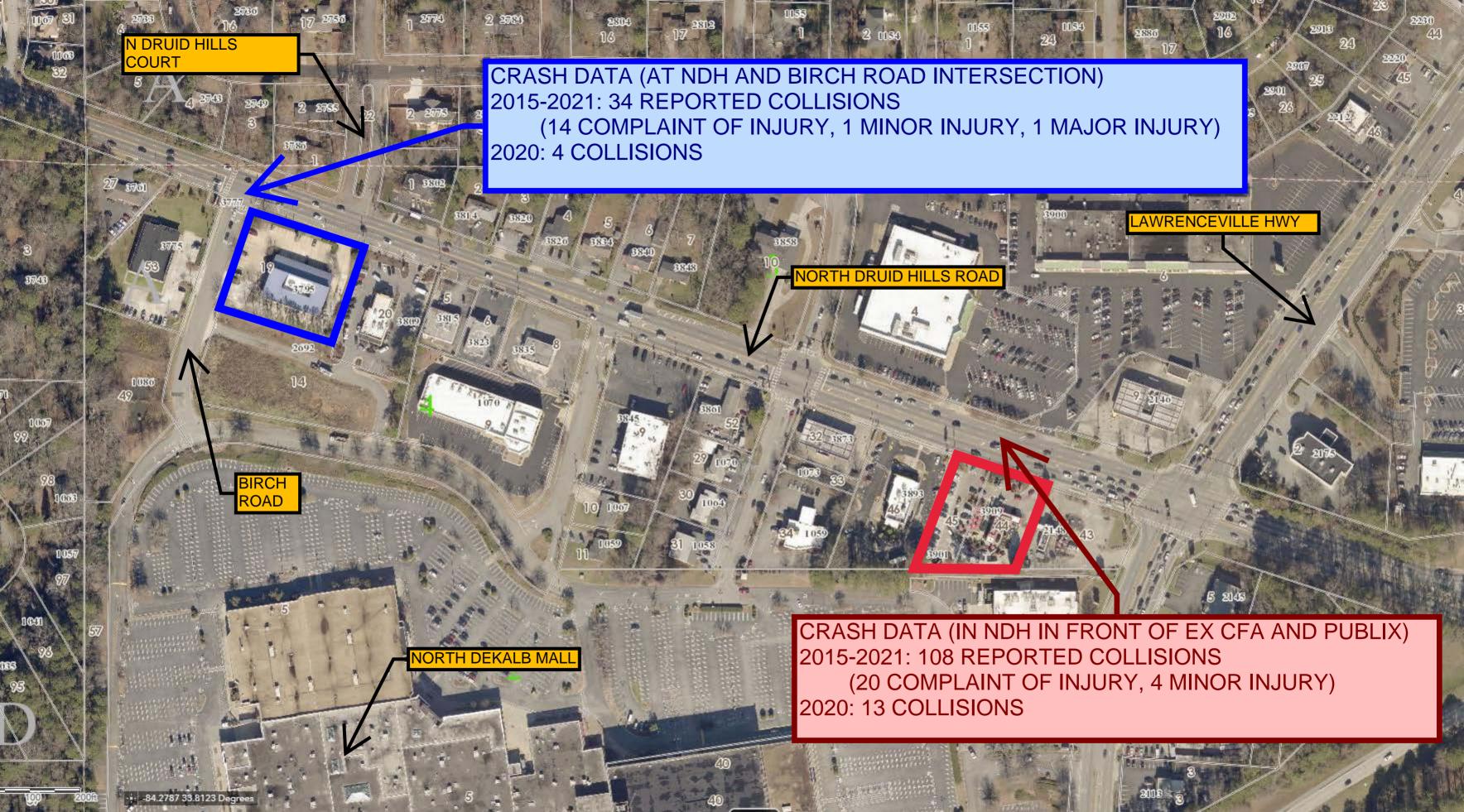
Network Summary

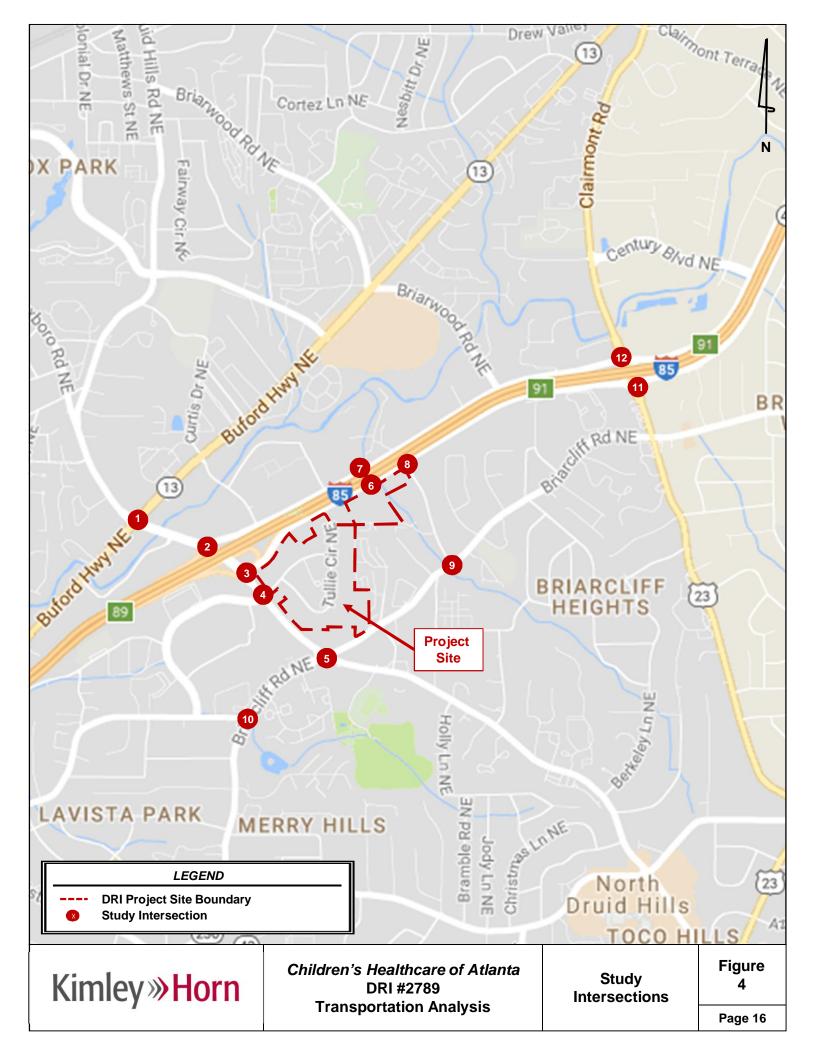
Network wide Queuing Penalty: 1

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Page 1













November 29, 2021

Ms. Patrece Keeter
Dekalb County Public Works
Transportation Division
1300 Commerce Drive
Decatur, GA 30030

Intersection Analysis Memo:

North Druid Hills Road at Birch Road Chick-fil-A Relocation Intersection Analysis - Decatur, GA

Existing Traffic Volumes

The existing Chick-fil-A restaurant located at 3905 N Druid Hills Rd in Decatur, GA, is proposing to relocate to the southeast corner of the Birch Road at N Druid Hills Rd intersection. The proposed site location was previously occupied by a Pier 1 Imports retail store. This memo analyzes the intersection LOS and queuing at North Druid Hills at Birch Road in the existing and proposed building conditions and the potential impacts on the operation of this intersection by the proposed relocation of the Chick-fil-A restaurant.

Traffic count data for this project was originally collected on Tuesday July 13, 2021. Peak hour turning movement counts were collected at the intersections of Birch Road at N Druid Hills Rd and N Druid Hills Rd at both existing Chick-fil-A driveway locations. Additional peak hour turning movement counts were collected again at the Birch Rd intersection on Tuesday August 10th, 2021, after the school year had started. The peak hour volumes at the Birch Rd intersection are provided in Figure 1. The full set of traffic data is provided in Appendix A. Previous collected volumes at the Chick-fil-A driveways have not been adjusted for collection during the summer when school was not in session.

Proposed Traffic Volumes

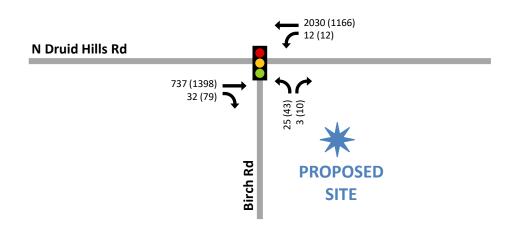
For the proposed traffic volumes, the existing Chick-fil-A trips were distributed to the Birch Rd intersection, the proposed right-in-right-out driveway on North Druid Hills Rd and the full access driveway located on Birch Rd. Chick-fil-A estimates the new site location to generate 10% more trips, so this growth factor is applied to the redistributed existing Chick-fil-A trips. In addition, with the new location proposed at the corner of Birch Rd at North Druid Hills Rd, 10% of the existing left-turning inbound trips have been distributed to arrive internal to the mall site via Sweetbriar Rd to northbound onto Birch Road. The proposed peak hour volumes at the Birch Road intersection are provided in Figures 2 and 3.

Figure 1: Existing Traffic Volumes

(##) AM (PM) Traffic Volumes

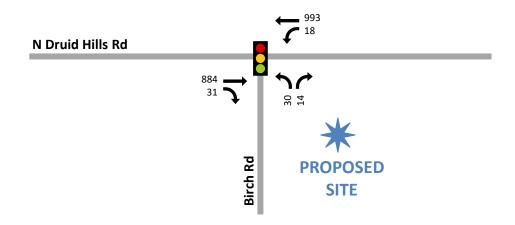


Existing Turning Movement Counts N Druid Hills Rd at Birch Rd Intersection



(##) MD Traffic Volumes (midday)

Existing Turning Movement Counts
N Druid Hills Rd at Birch Rd Intersection

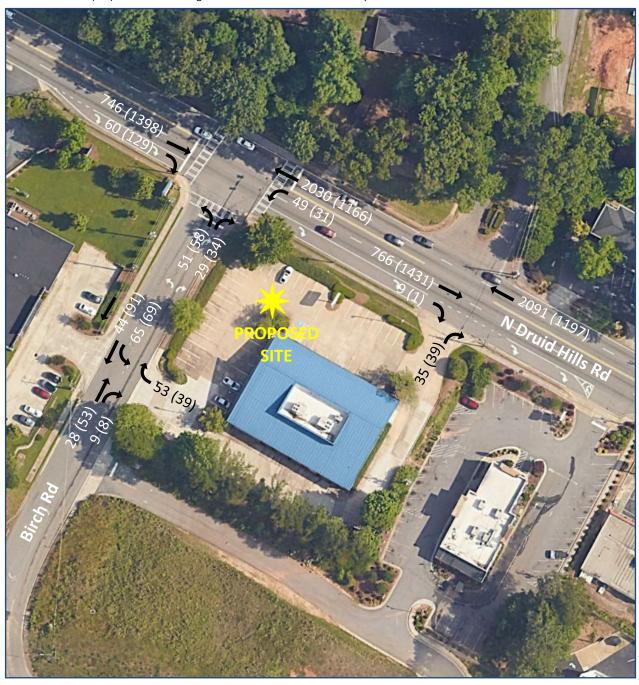




(##) AM (PM) Traffic Volumes

Volumes updated 8/10/2021 AM peak hour of Chick-fil-A volumes 7:30-8:30am PM peak hour of Chick-fil-A volumes 5:00-6:00pm





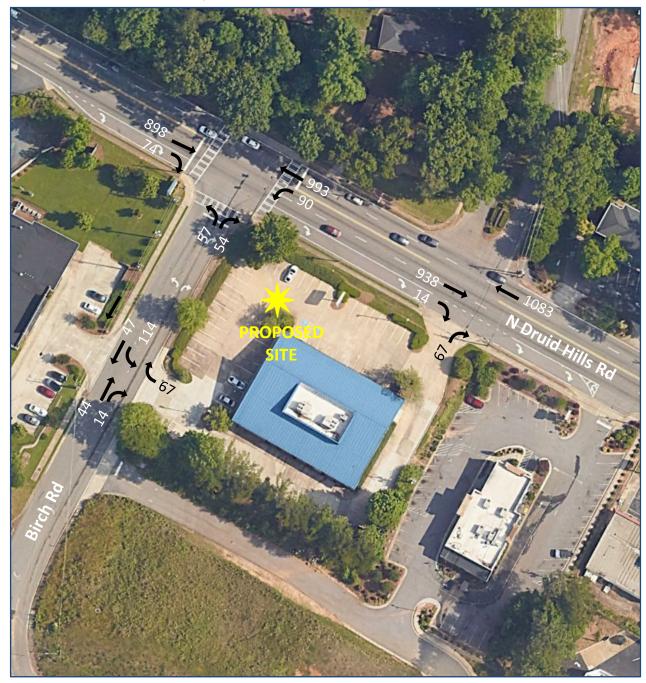


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(##) MD Traffic Volumes

Volumes updated 8/10/2021 MD peak hour of Chick-fil-a volumes 11:45am-12:45pm







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Existing Signal Timings and Field Observation

DeKalb County provided the signal timings for the intersections of Birch Road at North Druid Hills Road and Mistletoe Road. This corridor was observed in the field on Wednesday November 17th, 2021, from 7am to 6pm for all the peak and non-peak hour operations. The observation of this corridor also included the signalized intersections on North Druid Hills Road from US 29/Lawrenceville Hwy to the east and to Willivee Drive to the west of Birch Rd.

During the AM peak period this corridor runs Pattern 10 with a 150 second cycle length. The Midday (MD) period runs Pattern 20, also with a 150 second cycle length. The PM peak period runs Pattern 30 with a 170 second cycle length. On the section of the corridor between Birch Rd and Lawrenceville Hwy, the westbound traffic generally flows better because as vehicles travel westbound from Lawrenceville Hwy the amount of green time to get cars through each intersection on North Druid Hills Rd increases as vehicles go through each intersection towards Birch Rd. Similarly, the green time, or the window of time to get through the intersection on North Druid Hills Rd decreases as vehicles travel eastbound from Birch Rd to Mistletoe Rd to US 29/Lawrenceville Hwy. In the PM peak period this is the primary reason for the extreme backup, delay, and queuing. During the PM peak period the volumes are heavier going eastbound, and the narrowing of the time allowed for throughput reduces at each intersection to the east. The AM peak has a heavier westbound movement and the MD peak volumes are more balanced but increasingly gets heavier eastbound later in the day.

It should be noted that through field observations it was determined that the Birch Rd approach, or the Birch Rd phase of the signal, operates on a constant call. The signal was observed to serve the side street approach, or signal phase, from Birch Rd whether cars were present or not. This signal will serve the full Birch Rd phase split time every cycle during the day. This was also determined to be the case for side street and all left turn phases at the intersections of Mistletoe Rd and US 29/Lawrenceville Hwy. With these phases set on constant call, it was determined that this section of the corridor operates essentially as fixed time signals when setting up our corridor modeling.

During the AM peak the intersection at Birch Rd was observed to operate with no major issues. The AM peak traffic flow is predominately westbound during this time. There were observed to be distinct platoons arriving from Mistletoe Rd that generally always made it through the intersection at Birch Rd. It was observed that cars queuing westbound at this intersection typically arrived from the side streets at Mistletoe Rd or the adjacent neighborhood. There were large gaps in the eastbound traffic that allowed plenty of gaps for cars turning left from North Druid Hills Rd onto Birch Rd to make that turn without much associated delay. This movement was observed for several MARTA buses and large trucks serving the mall area.

The MD peak was observed to operate similarly to the AM peak period. The traffic volumes on North Druid Hills Rd become more balanced directionally, but the operation of the intersection continues to function with very little delay and queuing. The platoons from the east continue to clear the intersection. The eastbound traffic continues to have sizeable gaps in traffic allowing for left turn maneuvers to continue onto Birch Rd without much associated delay.

The PM peak period experiences heaving queuing delay for eastbound traffic that is compounded later in the peak period. The traffic volumes were observed to intensify during the four o'clock hour that coincides with a school release period. During the peak period the queuing originates at US 29/Lawrenceville Hwy intersection and causes delays back at the Mistletoe Rd intersection. This queuing eventually spills back further west impacting the delay at the Birch Rd intersection. The queueing from Birch Rd was observed to average around 2,000-ft+ to the west. It was observed at this intersection when queuing was extreme, drivers would leave the intersection open to allow vehicles turning left from North Druid Hills to make that maneuver. This same type of operation happens at the existing Chick-fil-A driveway location during the PM peak period. This observed queuing was utilized to calibrate the timings for this section of the corridor by oversaturating the demand to match the average queuing in the SimTraffic simulation modeling.

Existing Conditions Capacity Analysis

Existing traffic volumes were entered into a *Synchro 10* model to perform capacity analysis of existing conditions for the AM, MD and PM peak periods. The timings were calibrated to the field conditions to match existing conditions. The results of the capacity analysis are shown by lane group movement in Table 1. Average vehicular delays and level-of service, as defined by the Highway Capacity Manual (HCM) 6th Edition are presented, and 95th percentile queues from *SimTraffic 10* are shown. Full *Synchro* output reports are included Appendix B.

AM Peak Hour MD Peak Hour PM Peak Hour Lane Group Intersection Control 95th% 95th% 95th% LOS Movement LOS Delay (s) Delay (s) LOS Delay (s) Queue (ft) Queue (ft) Queue (ft) EBT 7.5 Α 173 8.9 Α 220 36.8 D 4539 EBR 5.8 Α 29 6.4 Α 29 5.5 Α 168 WBL 2.5 144 0.9 95 73.2 160 North Druid Hills Rd Signal 3.1 Α WBT 167 1.1 70 2.6 166 at Birch Rd Control NBL 49.6 48.2 D Ε 77 48.5 22 Ε 27 D 10 47.5 D 59.0 Intersection

Table 1: Existing Conditions Capacity Analysis

The intersection of Birch Rd at North Druid Hills Rd is shown to operate at LOS A during the AM and MD peak periods. Under these conditions the NB approach lanes from Birch Rd are shown to operate at LOS D. The northbound approach of the signal is on constant recall.

The PM peak period is shown to operate at LOS D with 37.1 seconds of overall intersection delay. The northbound approaches are shown to operate at LOS E. The existing WBL movement is shown to operate at LOS F. The 95th percentile queuing for the eastbound approach is shown to be 4,539-ft and the average queuing is shown to be at 2,100-ft+. The delay at this intersection is largely due to the extreme peak hour queuing from upstream intersections to the east and less from the side street volume demands.

Build Conditions Capacity Analysis

The proposed redistributed traffic volumes were also entered into a *Synchro 10* model to perform capacity analysis of existing conditions for the AM, MD and PM peak periods. The results of the capacity analysis are shown by lane group movement in Table 2. Average vehicular delays and level-of service, as defined by the Highway Capacity Manual (HCM) 6th Edition are presented, and 95th percentile queues from *SimTraffic 10* are shown. This analysis also include the additional proposed Chick-fil-A driveways located on North Druid Hills Rd and Birch Rd. Full *Synchro* output reports are included Appendix B. The estimated signal timings for the intersection at Birch Rd remain the same as the existing conditions analysis.

Table 2: Build Conditions Capacity Analysis

		l	Į.	AM Peak Hou	r	N	/ID Peak Hou	r	F	M Peak Hou	r
Intersection	Control	Lane Group Movement	Delay (s)	LOS	95 th % Queue (ft)	Delay (s)	LOS	95 th % Queue (ft)	Delay (s)	LOS	95 th % Queue (ft)
		EBT	7.6	Α	186	8.5	А	221	36.8	D	5446
		EBR	6.0	Α	49	6.3	А	64	5.9	Α	170
		WBL	13.6	В	183	3.5	Α	176	426.2	F	207
North Druid Hills Rd at Birch Rd	Signal Control	WBT	10.4	В	200	1.9	А	173	5.3	Α	246
at Birch Na	control	NBL	50.9	D	84	49.0	D	123	63.3	E	100
		NBR	50.6	D	37	49.3	D	57	64.0	E	96
		Intersection	11.9	В	-	7.6	Α	-	91.5	F	-
		EBT	-	-	-	-	-	-	-	-	228
	Side Street	EBR	-	ı	-	-	ı	-	-	ı	-
North Druid Hills Rd at DW1	Stop	WBT	-	-	134	-	-	88	-	-	142
atbwi	Controlled	NBR	11.6	В	38	13.3	В	47	23.9	С	96
		Intersection	0.1	-	-	0.4	-	-	0.1	-	-
		WB	8.6	Α	50	8.7	Α	54	8.8	Α	45
Birch Rd at DW2	Side Street	NB	-	-	3	-	-	-	-	-	5
BIICII NU at DW2	Stop Controlled	SB	7.4	Α	21	7.5	Α	38	7.5	Α	24
	22	Intersection	4.7	Α	-	5.7	Α	-	3.9	1	-

The intersection of Birch Rd at North Druid Hills Rd is shown to continue to operate at acceptable levels of service during the AM and MD peak periods with minor increases to the overall intersection delay. Under these conditions the NB approaches from Birch Rd continues to operate at LOS D with minor increases in delay and queuing. The queuing shown during these time periods does not spill back and block the driveway located to the south on Birch Rd. The additional driveways located on North Druid Hills Rd and Birch Rd are shown to operate at acceptable levels of service during the AM and MD peak periods.

The PM peak period is shown to operate at LOS F with 91.5 seconds of overall intersection delay. The northbound approaches are shown to operate at LOS E. The WBL movement continues to operate at LOS F with increased delay. This increased delay for this approach contributes to the overall delay of the intersection degrading to LOS F. The average queuing in this scenario is shown to be at 2,600-ft+.

This intersection was also evaluated by allowing the signal to operate as actuated, removing the constant recall, and with an optimized split time for the Birch Road approach. The impacts of those changes are provided below in Table 3.

Table 3: Build Conditions Capacity Analysis (Optimized)

		Lane Group	Į.	AM Peak Hou	r	N	/ID Peak Hou	r	F	PM Peak Hou	r
Intersection	Control	Movement	Delay (s)	LOS	95 th % Queue (ft)	Delay (s)	LOS	95 th % Queue (ft)	Delay (s)	LOS	95 th % Queue (ft)
		EBT	1.7	Α	105	1.7	Α	221	9.7	Α	5718
		EBR	1.2	Α	31	1.2	Α	64	1.8	Α	160
	6: 1	WBL	3.9	Α	132	1.2	Α	176	20.5	С	210
North Druid Hills Rd at Birch Rd	Signal Control	WBT	3.5	Α	134	1.0	Α	173	2.8	Α	238
at Bircii Nu	Control	NBL	80.1	F	115	78.5	D	123	91.7	F	92
		NBR	74.0	E	20	81.8	D	57	96.1	F	155
		Intersection	5.1	Α	=	5.4	Α	=	12.6	В	=

This analysis indicates there is potential opportunity to improve the signal operation with some minor adjustments to the existing timings and functionality of the intersection vehicle detection.

Build Conditions Alternative Analysis

The intersection at Birch and North Druid Hills Rd has also been analyzed with minor changes to the signal timing or intersection configuration with focus during the PM peak operation. The first alternate analysis considers allowing the side street approach (Birch Rd) phase of the signal to be detection actuated instead of being on constant recall. The result of the implemented actuated detection is provided below in Table 4.

Table 4: Build Conditions Alternate Analysis 1

		Lana Graun	ı	PM Peak Hou	r
Intersection	Control	Lane Group Movement	Delay (s)	LOS	95 th % Queue (ft)
		EBT	9.3	Α	5601
		EBR	1.8	Α	164
	a	WBL	20.8	С	212
North Druid Hills Rd at Birch Rd	Signal Control	WBT	2.8	А	228
at Bireii Ka	Control	NBL	91.1	F	114
		NBR	95.3	F	121
		Intersection	12.7	В	-

These results indicate the intersection can potentially operate at LOS B in the PM peak period. The actuated detection does increase delay for the northbound approaches to degrade to LOS F. This adjustment also improves the WBL approach delay which most benefits the reduction of the overall delay at the intersection.

The intersection at Birch and North Druid Hills Rd has also been analyzed as a half-cycle operation with just a minimum recall placed on the side street approach. This shortened the cycle time at this intersection to 85 seconds, but the intersection remains in coordination with the other intersections on the corridor. The result of the implemented timings changes is provided below in Table 5.

Table 5: Build Conditions Alternate Analysis 2

		Lana Graun	ſ	PM Peak Hou	r
Intersection	Control	Lane Group Movement	Delay (s)	LOS	95 th % Queue (ft)
		EBT	17.6	В	5527
		EBR	2.4	А	165
N B	6. 1	WBL	27.4	С	203
North Druid Hills Rd at Birch Rd	Signal Control	WBT	3.6	А	238
at Bireii Ka	Control	NBL	44.3	D	65
		NBR	46.1	D	65
		Intersection	17.0	В	-

These results indicate the intersection can potentially operate at LOS B in the PM peak period with a reduced half cycle. The minor street approach in this scenario was placed on minimum recall. This adjustment also improves the WBL approach delay which most benefits the reduction of the overall delay at the intersection.

The intersection at Birch and North Druid Hills Rd has also been analyzed to consider a future potential left turn lane with left turn signal phasing operation. The side street approach has been placed in a maximum constant recall, as in the existing conditions, but the split time has been reduced. A left turn phase and timing has also been included with actuated detection. The result of the implemented timings changes is provided below in Table 6.

Table 6: Build Conditions Alternate Analysis 3

		Lana Graun	ı	PM Peak Hou	r
Intersection	Control	Lane Group Movement	Delay (s)	LOS	95 th % Queue (ft)
		EBT	34.3	С	5482
		EBR	5.6	А	168
North Dood didth Dd	C: I	WBL	55.4	E	73
North Druid Hills Rd at Birch Rd	Signal Control	WBT	1.0	А	61
a c Bireii iid	control	NBL	76.7	E	104
		NBR	78.4	E	101
		Intersection	23.3	С	-

These results indicate the intersection can potentially operate at LOS C in the PM peak period with a reduced side street approach phase time and the inclusion of a westbound left turn lane and signal phase. The minor street approach in this scenario was placed on maximum recall with a reduced split time. This adjustment also improves the WBT operation and approach. The WBL queuing is also reduced.

Conclusions and Recommendations

The intersection of Birch Rd at North Druid Hills Rd operates at acceptable levels of service, with minor delay and queuing during the AM and MD peak periods in the existing conditions. This intersection continues to operate at acceptable levels of service, with minor delay and queuing, when the volumes

generated by Chick-fil-A are distributed to this intersection.

During the PM peak period the intersection of Birch Rd at North Druid Hills Rd experiences delay and queuing that can be attributed to delay and queuing originating from intersections located to the east. The Birch Rd intersection is shown to operate at LOS D during the PM peak period with average queuing observed to be 2,000-ft+ to the west. When the Chick-fil-A traffic is distributed to the intersection in the PM peak period, and simulated with the existing timing configuration, the intersection delay is shown to increase, degrading the overall intersection level of service. However, there are potentially a few minor timing adjustments then could be implemented to help reduce delay and improve the overall intersection

level of service.

Because of the extreme queuing and backup occurring on the corridor during the PM peak period, the driveways for the Chick-fil-A at the existing location are experiencing the same type of delays shown when those volumes are distributed to the Birch Rd intersection. By relocating the restaurant, the Birch Rd intersection can potentially allow for improved traffic operations by providing a signal operation for the outbound vehicles turning left from Chick-fil-A. In addition, having the option for customers to arrive from south of the site via Birch Rd may potentially reduce traffic demands of customers arriving via North

Druid Hills Rd.

The simulation modeling indicates that the intersection operation can potentially be improved with some minor timing adjustments, even with the addition of traffic volumes generated by the Chick-fil-A. These timing and signal adjustments could be implemented relatively quickly if necessary. For the long-term operation of this intersection, it is recommended for westbound left turn lane and left turn signal phasing be added to this intersection.

Please contact me or Jack Johnson at 770-368-1399 if you have any questions or need additional

information.

Sincerely,

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FORESITE GROUP, LLC

Stevie Berryman Project Manager

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Appendix A: Traffic Counts

${\tt National\ Data\ \&\ Surveying\ Services} \\ Intersection\ Turning\ Movement\ Count$

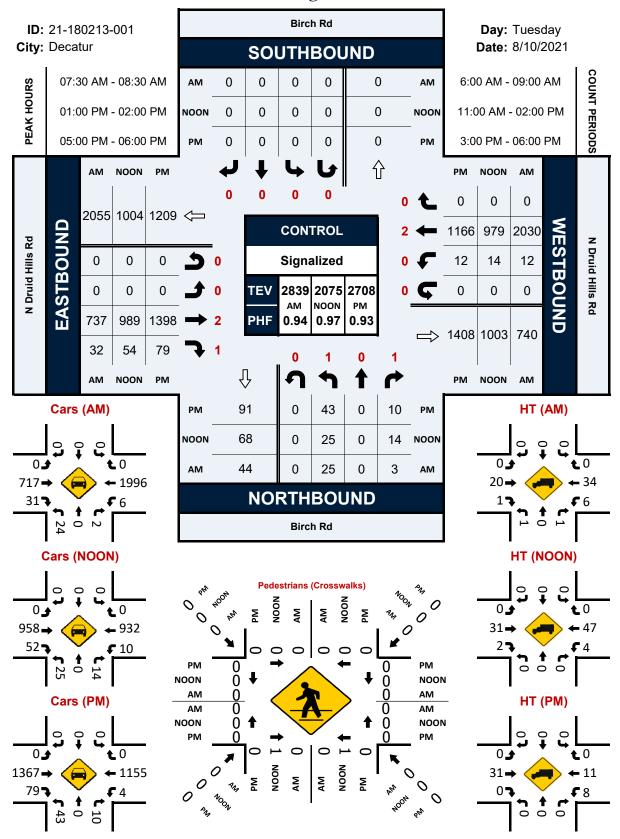
Location: Birch Rd & N Druid Hills Rd City: Decatur

Project ID: 21-180213-001 Date: 8/10/2021

City: Control:	Decatur Signalized												Pro	Date: 8	:1-180213- 3/10/2021	J01	
ı								Data ·	- Total								ı
NS/EW Streets:		Birch	Rd			Birc	h Rd			N Druid F	lills Rd			N Druid H	lills Rd		l
AM	1	NORTHE 0	BOUND 1	0	0	SOUTI 0	HBOUND 0	0	0	EASTB0	OUND 1	0	0	WESTB 2	OUND 0	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
6:00 AM 6:15 AM	1 1	0	0 0	0	0	0	0	0	0	58 98	2	0	0 4	211 287	0 0	0	272 393
6:30 AM	0	0	0	0	0	0	0	0	0	99	0	Ō	0	362	0	0	461
6:45 AM 7:00 AM	3 1	0	0	0	0	0	0	0	0	107 127	4	0	3 0	419 405	0	0	536 537
7:15 AM	3	0	0	0	0	0	0	0	0	153	2	0	2	443	0	0	603
7:30 AM 7:45 AM	2 7	0	2 0	0	0	0	0	0	0	192 181	9 9	0	2 5	549 535	0	0	756 737
8:00 AM 8:15 AM	4 12	0	0	0	0	0	0	0	0	167 197	7	0	3 2	475 471	0	0	656 690
8:30 AM	6	0	ō	Ō	0	Ō	Ō	Ō	0	191	ģ	0	1	525	0	0	732
8:45 AM	3	0	0	0	0	0	0	0	0	159	6	0	3	480	0	0	651
	NL 42	NT	NR 3	NU 0	SL	ST	SR	SU	EL 0	ET	ER	EU 0	WL	WT	WR 0	WU 0	TOTAL
TOTAL VOLUMES : APPROACH %'s :	43 93.48%	0 0.00%	6.52%	0.00%	0	0	0	0	0.00%	1729 96.54%	62 3.46%	0.00%	25 0.48%	5162 99.52%	0.00%	0.00%	7024
PEAK HR : PEAK HR VOL :	25	07:30 AM -	08:30 AM 3	0	0	0	0	0	0	737	32	0	12	2030	0	0	TOTAL 2839
PEAK HR FACTOR :	0.521	0.000	0.375	0.000	0.000	0.000	0.000	0.000	0.000	0.935	0.889	0.000	0.600	0.924	0.000	0.000	0.939
		0.53								0.94				0.92			
NOON	1	NORTHE 0	BOUND 1	0	0	SOUTI 0	HBOUND 0	0	0	EASTB0	OUND 1	0	0	WESTB 2	OUND 0	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ĒR	EU	WL	WT	WR	WU	TOTAL
11:00 AM 11:15 AM	8 7	0	2	0	0	0	0	0	0	192 231	7 10	0	5 1	263 236	0 0	0	477 487
11:30 AM	6	0	2	0	0	0	0	0	0	195 215	7	0	4	245 270	0	0	459
11:45 AM 12:00 PM	7	0	5	0	0	0	0	0	0	215	11	0	2	222	0	0	512 462
12:15 PM 12:30 PM	5 6	0	5 2	0	0	0	0	0	0	211 243	8 5	0 1	4 6	240 261	0	0 1	473 525
12:45 PM	8	0	3	0	0	0	0	0	0	225	11	0	2	255	0	0	504
1:00 PM 1:15 PM	6 5	0	3 2	0	0 0	0	0	0	0	252 251	19 9	0	3 3	226 232	0	0	509 502
1:30 PM	8	0	1 8	0	0	0	0	0	0	247	12 14	0	4	256 265	0	0	528 536
1:45 PM	_	_						-	_	239		-	•		-	_	
TOTAL VOLUMES :	NL 84	NT 0	NR 37	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 2716	ER 120	EU 1	WL 44	WT 2971	WR 0	WU 1	TOTAL 5974
APPROACH %'s : PEAK HR :	69.42%	0.00% 01:00 PM -	30.58%	0.00%					0.00%	95.73%	4.23%	0.04%	1.46%	98.51%	0.00%	0.03%	TOTAL
PEAK HR VOL :	25	0	14	0	0	0	0	0	0	989	54	0	14	979	0	0	2075
PEAK HR FACTOR :	0.781	0.000	0.438 96	0.000	0.000	0.000	0.000	0.000	0.000	0.981 0.96	0.711 2	0.000	0.875	0.924 0.92	0.000 !3	0.000	0.968
		NORTHE	BOUND			SOUTH	HBOUND			EASTBO	OUND			WESTB	OUND		
PM	1	0	1	0	0	0	0	0	0	2	1	0	0	2	0	0	TOTAL
3:00 PM	NL 7	NT 0	NR 3	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 342	ER 17	EU 0	WL 1	WT 238	WR 0	WU 0	608
3:15 PM 3:30 PM	8 11	0	1 6	0	0	0	0	0	0	379 403	9 20	0	4 2	249 191	0	0	650 633
3:45 PM	11	0	7	0	0	0	0	0	0	354	16	0	3	223	0	0	614
4:00 PM 4:15 PM	15 7	0	3 1	0	0	0	0	0	0	343 330	17 23	0	6 4	227 302	0	0	611 667
4:30 PM	11	0	4	0	0	0	0	0	0	331	23	Ō	3	234	Ō	1	607
4:45 PM 5:00 PM	15 14	0	3	0	0	0	0	0	0	352 317	24 29	0	6	239 254	0	0	637 623
5:15 PM 5:30 PM	8 9	0	4	0	0 0	0	0	0	0	350 381	16 17	0	1	263 322	0	0	642 731
5:45 PM	12	0	2	0	0	0	0	0	0	350	17	0	4	327	0	0	712
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES : APPROACH %'s :	128 75.74%	0 0.00%	41 24.26%	0 0.00%	0	0	0	0	0 0.00%	4232 94.89%	228 5.11%	0 0.00%	36 1.16%	3069 98.81%	0 0.00%	1 0.03%	7735
PEAK HR :		05:00 PM -	06:00 PM														TOTAL
PEAK HR VOL : PEAK HR FACTOR :	43 0.768	0.000	10 0.625	0.000	0.000	0.000	0.000	0.000	0.000	1398 0.917	79 0.681	0 0.000	12 0.500	1166 0.891	0.000	0.000	2708
	2 30	0.000	79			2.200	2.200	2.300		0.517	18			0.80	20		0.926

Birch Rd & N Druid Hills Rd

Peak Hour Turning Movement Count



${\tt National\ Data\ \&\ Surveying\ Services} \\ Intersection\ Turning\ Movement\ Count$

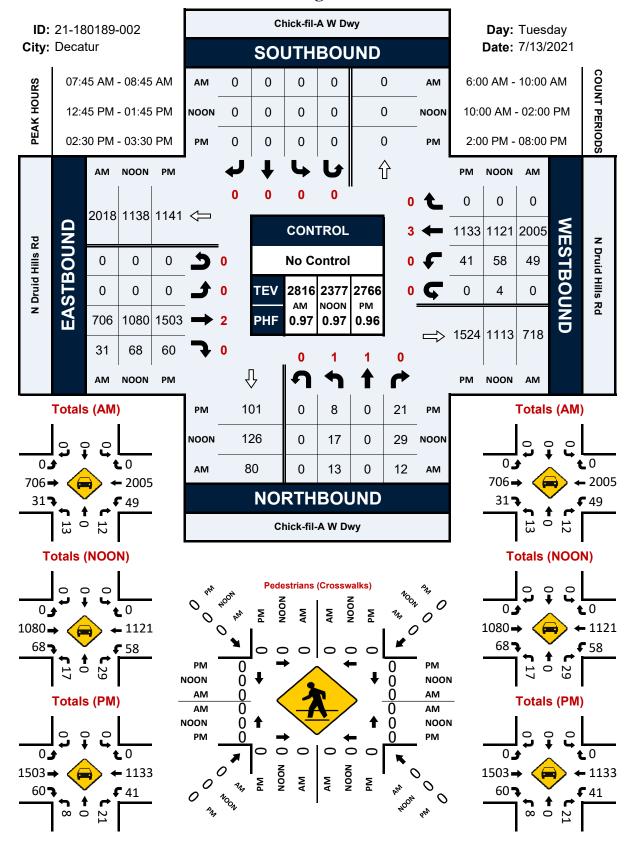
Location: Chick-fil-A W Dwy & N Druid Hills Rd City: Decatur Control: No Control

Project ID: 21-180189-002 Date: 7/13/2021

Control:	No Control							Data -	Totals					Date: 7	//13/2021		
NS/EW Streets:		Chick-fil-A	W Dwy			Chick-fil-	A W Dwy	Dutu	locais	N Druid H	lills Rd			N Druid H	Hills Rd		
		NORTH	BOUND			SOUT	HBOUND			EASTB	OUND			WESTB	OUND		
AM	1 NL	1 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	3 WT	0 WR	0 WU	TOTAL
6:00 AM	0	0	0	0	0	0	0	0	0	47	0 7	0	9	187	0	0	243
6:15 AM 6:30 AM	1 2	0	1	0 0	0	0 0	0 0	0	0	76 83	3	0 0	6 15	274 315	0	0	365 419
6:45 AM	4	0	3	0	0	0	0	0	0	112	5	0	11	339	0	0	474
7:00 AM 7:15 AM	1 3	0	3 0	0	0	0	0	0	0	107 125	4 6	0	19 10	363 404	0	0	497 548
7:30 AM	4	0	2	0	0	Ö	0	0	0	167	15	0	16	470	0	0	674
7:45 AM 8:00 AM	3 5	0	2 4	0	0	0	0	0	0	171 177	10 10	0	6 16	526 517	0	0	718 729
8:15 AM	4	0	3	0	0	0	0	0	0	177	1	0	10	497	0	0	692
8:30 AM	1	0	3	0	0	0	0	0	0	181	10	0	17	465	0	0	677
8:45 AM 9:00 AM	3	0	5 4	0	0	0	0	0	0	183 191	12 10	0	17 13	477 363	0	0	697 584
9:15 AM	3	0	4	0	0	0	0	0	0	181	10	0	16	386	0	0	600
9:30 AM 9:45 AM	5 3	0	5 3	0	0	0	0	0	0	168 208	17 13	0	20 10	365 351	0	0	580 588
															11100		
TOTAL VOLUMES :	NL 45	NT 0	NR 43	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 2354	ER 133	O CI	WL 211	WT 6299	WR 0	O	TOTAL 9085
APPROACH %'s:	51.14%	0.00%	48.86%	0.00%					0.00%	94.65%	5.35%	0.00%	3.24%	96.76%	0.00%	0.00%	
PEAK HR : PEAK HR VOL :	13	07:45 AM - 0	08:45 AM 12	0	0	0	0	0	0	706	31	0	49	2005	0	0	TOTAL 2816
PEAK HR FACTOR :	0.650	0.000	0.750	0.000	0.000	0.000	0.000	0.000	0.000	0.975	0.775	0.000	0.721	0.953	0.000	0.000	0.966
		0.6	94							0.96	55			0.96	i3		0.500
NOON		NORTH			_		HBOUND			EASTB		_	_	WESTB		_	
NOON	1 NL	1 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	3 WT	0 WR	0 WU	TOTAL
10:00 AM	6	0	7	0	0	0	0	0	0	172	7	0	16	259	0	0	467
10:15 AM 10:30 AM	7 4	0	3 4	0	0	0	0	0	0	187 226	17 9	0	10 13	259 277	0	0	483 533
10:45 AM	3	0	4	0	0	0	0	0	0	196	10	Ō	12	258	0	0	483
11:00 AM 11:15 AM	2 5	0	2 6	0	0	0	0	0	0	220 228	10 14	0	18 13	230 259	0	0 1	482 526
11:30 AM	3	0	7	0	0	Ö	Ö	Ö	0	254	15	0	15	281	0	Ō	575
11:45 AM 12:00 PM	1 3	0	9 7	0	0	0	0	0	0	232 229	14 10	0	24 16	247 250	0	0	527
12:15 PM	2	0	5	0	0	0	0	0	0	258	16	0	12	278	0	0	516 571
12:30 PM	3	0	6 7	0	0	0	0	0	0	259	16	0	22	237	0	0	543
12:45 PM 1:00 PM	3	0	7	0	0	0	0	0	0	291 266	17 18	0	11 18	268 269	0	3	601 582
1:15 PM	9	Ō	10	0	0	Ö	0	0	0	273	18	Ö	10	294	Ō	0	614
1:30 PM 1:45 PM	1 3	0	5 8	0	0	0	0 0	0	0	250 274	15 20	0	19 8	290 280	0	0	580 593
TOTAL VOLUMES :	NL 59	NT 0	NR 97	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 3815	ER 226	O CI	WL 237	WT 4236	WR 0	WU 6	TOTAL 8676
APPROACH %'s:	37.82%	0.00%	62.18%	0.00%	-			-	0.00%	94.41%	5.59%	0.00%	5.29%	94.57%	0.00%	0.13%	
PEAK HR : PEAK HR VOL :	17	12:45 PM - 0	01:45 PM 29	0	0	0	0	0	0	1080	68	0	58	1121	0	4	TOTAL 2377
PEAK HR FACTOR :	0.472	0.000	0.725	0.000	0.000	0.000	0.000	0.000	0.000	0.928	0.944	0.000	0.763	0.953	0.000	0.333	0.968
		0.6	05							0.93	32			0.95	o/		
PM			BOUND	0	0	SOUTI	HBOUND 0	0	0	EASTB	OUND 0	0	0	WESTB 3		0	
PIVI	1 NL	1 NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	0 WR	WU	TOTAL
2:00 PM	3	0	5	0	0	0	0	0	0	318	8	0	18	225	0	0	577
2:15 PM 2:30 PM	1 2	0 0	10 8	0	0	0 0	0 0	0	0	328 392	15 15	0 0	10 14	272 289	0 0	0	636 720
2:45 PM	1	0	8	0	0	0	0	0	0	355	14	0	9	273	0	0	660
3:00 PM 3:15 PM	2	0 0	1 4	0 0	0	0 0	0 0	0	0	367 389	16 15	0 0	8 10	271 300	0 0	0 0	665 721
3:30 PM	2	0	5	0	0	0	0	0	0	356	8	0	12	252	0	1	636
3:45 PM 4:00 PM	2	0	3	0	0	0	0	0	0	378 391	13 10	0	7	247 284	0	0	647 694
4:15 PM	1	0	5	0	0	0	0	0	0	373	9	1	9	287	0	0	685
4:30 PM 4:45 PM	0 4	0 0	4 3	0	0	0 0	0 0	0	0	366 381	12 9	1 0	9 4	249 258	0 0	0	641 659
5:00 PM	2	0	4	0	0	0	0	0	0	394	9	0	7	277	0	0	693
5:15 PM 5:30 PM	1 2	0 0	4 4	0	0	0 0	0 0	0	0	352 374	16 8	0 0	8 5	270 257	0 0	0	651 650
5:45 PM	0	0	0	0	0	0	0	0	0	394	13	0	4	256	0	0	667
6:00 PM 6:15 PM	0 2	0	3	0 0	0	0	0	0	0	369 353	12 10	0 0	4 7	238 234	0	0 0	626 609
6:30 PM	4	Ō	4	0	0	0	0	0	0	378	14	0	6	241	0	0	647
6:45 PM 7:00 PM	3	0	3	0	0	0	0	0	0	322 290	11 19	0	14 14	229 179	0	0	581 506
7:15 PM	3	Ō	2	0	0	0	0	0	0	286	10	0	6	208	0	0	515
7:30 PM 7:45 PM	3 0	0 0	6 2	0	0	0	0 0	0	0	276 257	13 12	0	13 11	193 169	0	1 0	505 451
7.15 FP																	
TOTAL VOLUMES :	NL 43	NT 0	NR 94	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 8439	ER 291	EU 2	WL 213	WT 5958	WR 0	WU 2	TOTAL 15042
APPROACH %'s:	31.39%	0.00%	68.61%	0.00%					0.00%	96.64%	3.33%	0.02%	3.45%	96.52%	0.00%	0.03%	
PEAK HR : PEAK HR VOL :	8	02:30 PM - 0	03:30 PM 21	0	0	0	0	0	0	1503	60	0	41	1133	0	0	TOTAL 2766
PEAK HR FACTOR :	0.667	0.000	0.656	0.000	0.000	0.000	0.000	0.000	0.000	0.959	0.938	0.000	0.732	0.944	0.000	0.000	0.959
		0.7	25							0.96	60			0.94	17		0.339

Chick-fil-A W Dwy & N Druid Hills Rd

Peak Hour Turning Movement Count



${\tt National\ Data\ \&\ Surveying\ Services} \\ Intersection\ Turning\ Movement\ Count$

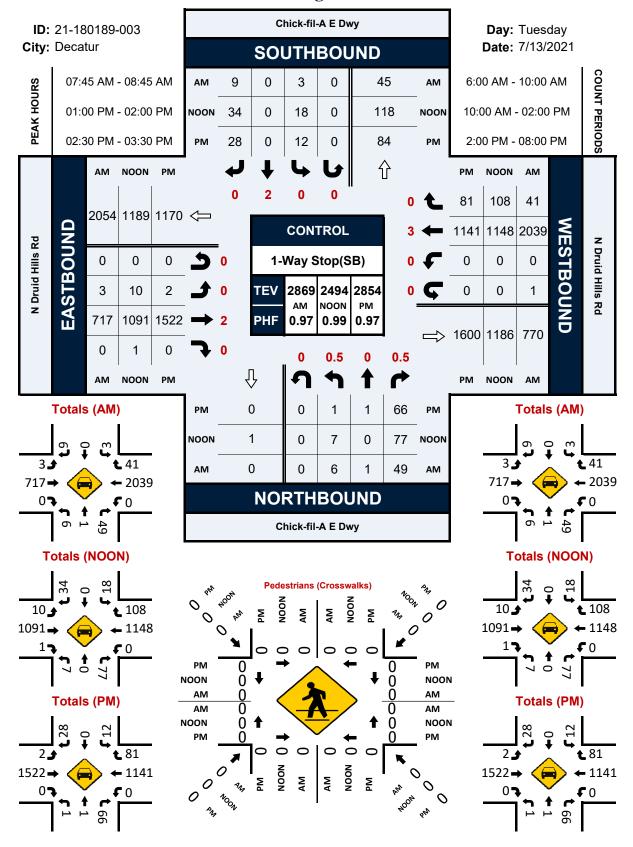
Location: Chick-fil-A E Dwy & N Druid Hills Rd City: Decatur Control: 1-Way Stop(SB)

Project ID: 21-180189-003 Date: 7/13/2021

Control	1-Way Stop	(30)						Data -	Totals					Date: /	/13/2021		
NS/EW Streets:		Chick-fil-A	E Dwy			Chick-fil-A		Dala -	TOLAIS	N Druid H	ills Rd			N Druid H	lills Rd		
		NORTHE	BOUND			SOUTH	BOUND			EASTBO	DUND			WESTB	OUND		
AM	0.5 NL	0 NT	0.5 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	3 WT	0 WR	0 WU	TOTAL
6:00 AM 6:15 AM	1 2	0	5 9	0	0	0	0	0	0	48 76	0	0	0	199 274	1 0	0	254 362
6:30 AM	2	Ō	8	0	0	0	2	0	0	85	0	0	0	334	3	0	434
6:45 AM 7:00 AM	4	0	9	0	0	0	2	0	1 2	111 110	0	0	0	339 376	6 8	0 1	470 512
7:15 AM 7:30 AM	4	0	13 12	0	0	0	0 3	0	0 2	126 163	0	0	0	420 470	6 8	0 1	569 662
7:45 AM	ī	Ō	14	0	2	0	5	0	1	176	0	Ō	0	526	9	Ō	734
8:00 AM 8:15 AM	3 1	1 0	10 9	0	1 0	Ö	3 0	0 0	0 1	181 179	0 0	0	0 0	529 504	12 10	0 1	740 705
8:30 AM 8:45 AM	1 2	0	16 13	0	0 0	0	1 2	0	1	181 188	0	0	0	480 491	10 15	0	690 712
9:00 AM 9:15 AM	2 2	0	18 15	0	2 2	0	6	0	0	196 185	0	0	0	367 396	16 16	0	607 621
9:30 AM	3	0	10	0	2	0	5 3	1	2	170 208	0	0	0	381 353	8	0	582
9:45 AM			19		2				•		0				21		611
TOTAL VOLUMES :	NL 34	NT 3	NR 189	NU 0	SL 12	ST 0	SR 37	SU 1	EL 15	ET 2383	ER 0	EU O	WL 0	WT 6439	WR 149	WU 3	TOTAL 9265
APPROACH %'s: PEAK HR:	15.04%	1.33% 07:45 AM -	83.63% 08:45 AM	0.00%	24.00%	0.00%	74.00%	2.00%	0.63%	99.37%	0.00%	0.00%	0.00%	97.69%	2.26%	0.05%	TOTAL
PEAK HR VOL : PEAK HR FACTOR :	6 0.500	1 0.250	49 0.766	0 0.000	3 0.375	0 0.000	9 0.450	0 0.000	3 0.750	717 0.990	0 0.000	0 0.000	0 0.000	2039 0.964	41 0.854	1 0.250	2869
PEAR HR FACTOR :	0.500	0.230		0.000	0.373	0.000		0.000	0.750	0.98		0.000	0.000	0.904		0.230	0.969
NOON	0.5	NORTHE		0	•	SOUTH			•	EASTBO		_	•	WESTB		0	
NOON	0.5 NL	0 NT	0.5 NR	NU	0 SL	2 ST	0 SR	0 SU	0 EL	2 ET	0 ER	<mark>0</mark> EU	0 WL	3 WT	0 WR	WU	TOTAL
10:00 AM 10:15 AM	1 2	0 0	17 13	0	5 3	0 0	2 5	0 0	0 1	174 193	1 0	0	0 0	271 262	21 19	0	492 498
10:30 AM 10:45 AM	4	1 0	16 13	0	7 6	0	8	0	2	228 200	0	0	0	278 263	17 13	0	561 502
11:00 AM 11:15 AM	2	0	8 22	0	3 1	0	7 5	0	1 3	214 236	0	0 2	0	239 274	26 17	0	500 561
11:30 AM	2	Ŏ	21	0	5	Ö	6	0	1	261	0	0	0	279	18	0	593
11:45 AM 12:00 PM	0 4	0	21 13	0	7	0	10 2	0	2	239 233	0	0	0	261 268	21 32	0	557 561
12:15 PM 12:30 PM	2	0	28 15	0	4 6	0	7 7	1 0	3 1	262 264	0	0	0	274 251	28 21	1 0	610 567
12:45 PM	2	0	15	0	3	0	15 10	0	3	289	0	0	0	264 277	21	0	613
1:00 PM 1:15 PM	2	Ō	18 21	0	4	0	9	0	2	279 274	0	0	0	292	25 22	0	616 626
1:30 PM 1:45 PM	2 1	0 0	20 18	0 0	6 6	0 0	10 5	0 0	2	260 278	0 1	0 0	0 0	304 275	28 33	0 0	632 620
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES : APPROACH %'s :	33 10.51%	2 0.64%	279 88.85%	0 0.00%	69 37.91%	0 0.00%	111 60.99%	2 1.10%	30 0.77%	3884 99.13%	2 0.05%	2 0.05%	0 0.00%	4332 92.27%	362 7.71%	1 0.02%	9109
PEAK HR : PEAK HR VOL :	7	01:00 PM -	02:00 PM 77	0	18	0	34	0	10	1091	1	0	0	1148	108	0	TOTAL 2494
PEAK HR FACTOR :	0.875	0.000	0.917 13	0.000	0.750	0.000	0.850 13	0.000	0.833	0.978	0.250 7	0.000	0.000	0.944 0.94	0.818 6	0.000	0.987
		NORTHE	BOUND			SOUTH	BOUND			EASTBO	DUND			WESTB	OUND		
PM	0.5 NL	0 NT	0.5 NR	0 NU	0 SL	2 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	3 WT	0 WR	0 WU	TOTAL
2:00 PM	0	0	20	0	4	0	8	0	1	320	0	0	0	231	19	1	604
2:15 PM 2:30 PM	0	1 0	12 12	0	6 4	0 0	10 8	0 0	2 1	338 399	0 0	0 0	0	275 294	18 13	0	662 731
2:45 PM 3:00 PM	0	1 0	14 20	0	2	0	7 6	0	0	360 370	0	0	0	272 281	19 28	0	678 707
3:15 PM 3:30 PM	1	0	20 19	0	2 6	0	7 10	0	0 2	393 360	0	0	0	294 255	21 14	0	738 666
3:45 PM 4:00 PM	0	0	16	0	1 7	0	4	0	5	367	0	0	0	250 281	31	0	674 745
4:15 PM	1	Ō	18 10	0	7	Ö	2	0	1	399 377	0	Ō	0	288	25 28	0	714
4:30 PM 4:45 PM	0	0 0	12 9	0	7 6	0 0	7 9	0	1	366 390	0 0	0	0 0	251 261	28 16	0	672 692
5:00 PM 5:15 PM	1 0	0 0	12 15	0 0	6 8	0	6 5	0 0	3 1	393 346	0	0	0 0	269 273	24 29	0 0	714 677
5:30 PM 5:45 PM	0 1	0	15 14	0	1 8	0	14 4	0	1 0	374 403	0	0	0	252 251	25 21	0	682 702
6:00 PM	1	0	9	0	6	0	7	0	1	366	0	0	0	240	25	0	655
6:15 PM 6:30 PM	0 1	0 0	11 17	0	6 4	0 0	9 6	0 0	1	356 385	0	0	0	226 245	23 34	0 0	632 693
6:45 PM 7:00 PM	2	0	13 16	0	<u>8</u> 4	0	<u>4</u> 0	0	0	324 287	0	0	0	233 191	27 18	0	610 518
7:15 PM 7:30 PM	2	1	20 16	0	8	0	6	0	1 2	293 281	0	0	0	217 190	24 14	0	572 513
7:45 PM	2	0	17	0	7	0	7	0	0	258	0	0	0	173	13	0	477
TOTAL VOLUMES : APPROACH %'s :	NL 16 4.26%	NT 3 0.80%	NR 357 94.95%	NU 0 0.00%	SL 126 43.90%	ST 0 0.00%	SR 161 56.10%	SU 0 0.00%	EL 29 0.34%	ET 8505 99.66%	ER 0 0.00%	EU 0 0.00%	WL 0 0.00%	WT 5993 91.76%	WR 537 8.22%	WU 1 0.02%	TOTAL 15728
PEAK HR : PEAK HR VOL : PEAK HR FACTOR :		1 0.250 0.81	03:30 PM 66 0.825	0 0.000	12 0.750	0 0.000 0.83	28 0.875	0 0.000	2 0.500	1522	0 0.000	0 0.000	0 0.000	1141 0.970 0.97	81 0.723	0.000	TOTAL 2854 0.967

Chick-fil-A E Dwy & N Druid Hills Rd

Peak Hour Turning Movement Count



Appendix B: Synchro Analysis

Intersection: 1: Birch Rd & North Druid Hills

Directions Served T T R LT T L R Maximum Queue (ft) 193 144 50 213 231 83 20 Average Queue (ft) 99 51 5 58 67 20 2 95th Queue (ft) 173 116 29 144 167 57 10 Link Distance (ft) 804 804 1083 1083 311 311
Average Queue (ft) 99 51 5 58 67 20 2 95th Queue (ft) 173 116 29 144 167 57 10 Link Distance (ft) 804 804 1083 1083 311 311
95th Queue (ft) 173 116 29 144 167 57 10 Link Distance (ft) 804 804 1083 1083 311 311
Link Distance (ft) 804 804 1083 1083 311 311
Unatropia DII Timo (0/)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft) 85
Storage Blk Time (%) 2
Queuing Penalty (veh) 1

Existing AM SimTraffic Report Page 1

	→	•	•	←	1	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414	ሻ	7
Traffic Volume (veh/h)	737	32	12	2030	25	3
Future Volume (veh/h)	737	32	12	2030	25	3
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	U	1.00	1.00	U	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No	1.00	1.00	No	No	1.00
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
	801	35	13	2207	27	3
Adj Flow Rate, veh/h						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2580	1151	31	2513	350	312
Arrive On Green	0.73	0.73	1.00	1.00	0.20	0.20
Sat Flow, veh/h	3647	1585	9	3547	1781	1585
Grp Volume(v), veh/h	801	35	1191	1029	27	3
Grp Sat Flow(s), veh/h/ln	1777	1585	1854	1617	1781	1585
Q Serve(g_s), s	12.0	0.9	0.0	0.0	1.9	0.2
Cycle Q Clear(g_c), s	12.0	0.9	0.0	0.0	1.9	0.2
Prop In Lane	12.0	1.00	0.01	0.0	1.00	1.00
	2500			1171		
Lane Grp Cap(c), veh/h	2580	1151	1370	1174	350	312
V/C Ratio(X)	0.31	0.03	0.87	0.88	0.08	0.01
Avail Cap(c_a), veh/h	2580	1151	1370	1174	350	312
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.30	0.30	1.00	1.00
Uniform Delay (d), s/veh	7.3	5.8	0.0	0.0	49.1	48.5
Incr Delay (d2), s/veh	0.3	0.0	2.5	3.1	0.4	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.3	0.3	1.0	1.0	0.9	0.1
Unsig. Movement Delay, s/veh		0.0	1.0	1.0	0.0	0.1
	7.6	5.8	2.5	3.1	49.6	48.5
LnGrp Delay(d),s/veh						
LnGrp LOS	A	A	A	Α	D	D
Approach Vol, veh/h	836			2220	30	
Approach Delay, s/veh	7.5			2.8	49.5	
Approach LOS	Α			Α	D	
Timer - Assigned Phs		2		4		6
·						
Phs Duration (G+Y+Rc), s		115.0		35.0		115.0
Change Period (Y+Rc), s		* 6.1		5.5		* 6.1
Max Green Setting (Gmax), s		* 1.1E2		29.5		* 1.1E2
Max Q Clear Time (g_c+l1), s		2.0		3.9		14.0
Green Ext Time (p_c), s		48.2		0.0		6.4
Intersection Summary						
HCM 6th Ctrl Delay			4.5			
HCM 6th LOS			4.5 A			
			^			
Notes						

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Existing AM 08/10/2021 Synchro 10 Report Page 1

Intersection: 1: Birch Rd & North Druid Hills

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	T	L	R
Maximum Queue (ft)	239	209	45	140	124	80	26
Average Queue (ft)	134	84	7	31	18	19	6
95th Queue (ft)	220	179	29	95	70	57	22
Link Distance (ft)	804	804		1075	1075	536	536
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		4	0				
Queuing Penalty (veh)		1	0				

Existing MD SimTraffic Report
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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	Z Z	,,,,,	41∱	ሻ	7
Traffic Volume (veh/h)	884	31	18	993	30	14
Future Volume (veh/h)	884	31	18	993	30	14
Initial Q (Qb), veh	0	0	0	993	0	0
Ped-Bike Adj(A_pbT)	U	1.00	1.00	U	1.00	1.00
, , , , , , , , , , , , , , , , , , ,	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No	4070	4070	No	No	4070
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	961	34	20	1079	33	15
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2533	1130	49	2407	374	333
Arrive On Green	0.71	0.71	1.00	1.00	0.21	0.21
Sat Flow, veh/h	3647	1585	33	3462	1781	1585
Grp Volume(v), veh/h	961	34	578	521	33	15
Grp Sat Flow(s), veh/h/ln	1777	1585	1793	1617	1781	1585
Q Serve(g_s), s	16.0	0.9	0.0	0.0	2.2	1.1
Cycle Q Clear(g_c), s	16.0	0.9	0.0	0.0	2.2	1.1
Prop In Lane	10.0	1.00	0.03	0.0	1.00	1.00
	0522			1150		
Lane Grp Cap(c), veh/h	2533	1130	1303	1152	374	333
V/C Ratio(X)	0.38	0.03	0.44	0.45	0.09	0.05
Avail Cap(c_a), veh/h	2533	1130	1303	1152	374	333
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.84	0.84	1.00	1.00
Uniform Delay (d), s/veh	8.5	6.3	0.0	0.0	47.7	47.3
Incr Delay (d2), s/veh	0.4	0.0	0.9	1.1	0.5	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.8	0.3	0.3	0.3	1.1	0.5
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	8.9	6.4	0.9	1.1	48.2	47.5
LnGrp LOS	Α	Α	Α	Α	40.2 D	47.3 D
	995			1099	48	D D
Approach Vol, veh/h						
Approach Delay, s/veh	8.8			1.0	48.0	
Approach LOS	Α			Α	D	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		113.0		37.0		113.0
Change Period (Y+Rc), s		* 6.1		5.5		* 6.1
Max Green Setting (Gmax), s		* 1.1E2		31.5		* 1.1E2
Max Q Clear Time (g_c+l1), s		2.0		4.2		18.0
Green Ext Time (p_c), s		9.0		0.1		8.3
U = 7:		9.0		0.1		0.3
Intersection Summary						
HCM 6th Ctrl Delay			5.7			
HCM 6th LOS			Α			
Notes						

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Existing MD 08/10/2021 Synchro 10 Report Page 1

Intersection: 1: Birch Rd & North Druid Hills

Directions Served T T R LT T L R Maximum Queue (ft) 4633 4613 125 217 235 89 40 Average Queue (ft) 2134 2145 69 52 52 35 7 95th Queue (ft) 4539 4532 168 160 166 77 27 Link Distance (ft) 4950 4950 1027 1027 498 498 Upstream Blk Time (%) 3 3 3 3 3 3 498	Movement	EB	EB	EB	WB	WB	NB	NB
Average Queue (ft) 2134 2145 69 52 52 35 7 95th Queue (ft) 4539 4532 168 160 166 77 27 Link Distance (ft) 4950 4950 1027 1027 498 498 Upstream Blk Time (%) 3 3 Queuing Penalty (veh) 0 0 Storage Bay Dist (ft) 85 Storage Blk Time (%) 46 0	Directions Served	T	Т	R	LT	Т	L	R
95th Queue (ft) 4539 4532 168 160 166 77 27 Link Distance (ft) 4950 4950 1027 1027 498 498 Upstream Blk Time (%) 3 3 Queuing Penalty (veh) 0 0 Storage Bay Dist (ft) 85 Storage Blk Time (%) 46 0	Maximum Queue (ft)	4633	4613	125	217	235	89	40
Link Distance (ft) 4950 4950 1027 1027 498 498 Upstream Blk Time (%) 3 3 Queuing Penalty (veh) 0 0 Storage Bay Dist (ft) 85 Storage Blk Time (%) 46 0	Average Queue (ft)	2134	2145	69	52	52	35	7
Upstream Blk Time (%) 3 3 Queuing Penalty (veh) 0 0 Storage Bay Dist (ft) 85 Storage Blk Time (%) 46 0	95th Queue (ft)	4539	4532	168	160	166	77	27
Queuing Penalty (veh)00Storage Bay Dist (ft)85Storage Blk Time (%)460	Link Distance (ft)	4950	4950		1027	1027	498	498
Storage Bay Dist (ft) 85 Storage Blk Time (%) 46 0	Upstream Blk Time (%)	3	3					
Storage Blk Time (%) 46 0	Queuing Penalty (veh)	0	0					
	Storage Bay Dist (ft)			85				
Ouguing Panalty (yeh) 36 0	Storage Blk Time (%)		46	0				
queding renaity (ven)	Queuing Penalty (veh)		36	0				

Existing PM SimTraffic Report
Page 1

	-	•	•	•	~	/
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7	1,00	41∱	ኘ	7
Traffic Volume (veh/h)	2040	79	12	1166	43	10
Future Volume (veh/h)	2040	79	12	1166	43	10
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	U	1.00	1.00	U	1.00	1.00
, , , , , , , , , , , , , , , , , , ,	1.00		1.00	1.00	1.00	1.00
Parking Bus, Adj		1.00	1.00	1.00		1.00
Work Zone On Approach	No	4477	4 477	No	No	4477
Adj Sat Flow, veh/h/ln	1477	1477	1477	1477	1477	1477
Adj Flow Rate, veh/h	2103	86	13	1267	47	11
Peak Hour Factor	0.97	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2127	949	22	1594	244	217
Arrive On Green	0.76	0.76	1.00	1.00	0.17	0.17
Sat Flow, veh/h	2879	1251	1	2169	1406	1251
Grp Volume(v), veh/h	2103	86	651	629	47	11
Grp Sat Flow(s), veh/h/ln	1403	1251	827	1277	1406	1251
Q Serve(g_s), s	123.0	3.0	5.9	0.0	4.9	1.2
Cycle Q Clear(g_c), s	123.0	3.0	128.9	0.0	4.9	1.2
Prop In Lane	120.0	1.00	0.02	0.0	1.00	1.00
Lane Grp Cap(c), veh/h	2127	949	648	968	244	217
1 1 7	0.99	0.09	1.00	0.65	0.19	0.05
V/C Ratio(X)						
Avail Cap(c_a), veh/h	2127	949	648	968	244	217
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.75	0.75	1.00	1.00
Uniform Delay (d), s/veh	19.8	5.3	41.8	0.0	60.1	58.6
Incr Delay (d2), s/veh	17.0	0.2	31.5	2.6	1.8	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	39.1	0.8	26.5	0.7	1.9	0.4
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	36.8	5.5	73.2	2.6	61.8	59.0
LnGrp LOS	D	A	F	Α	E	E
Approach Vol, veh/h	2189	, · ·		1280	58	
	35.6			38.5	61.3	
Approach LOS						
Approach LOS	D			D	Е	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		135.0		35.0		135.0
Change Period (Y+Rc), s		* 6.1		5.5		* 6.1
Max Green Setting (Gmax), s		* 1.3E2		29.5		* 1.3E2
Max Q Clear Time (g_c+l1), s		130.9		6.9		125.0
Green Ext Time (p_c), s		0.0		0.9		3.5
		0.0		0.1		0.0
Intersection Summary						
HCM 6th Ctrl Delay			37.1			
HCM 6th LOS			D			
Notes						

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Existing PM 08/10/2021 Synchro 10 Report

Intersection: 1: Birch Rd & North Druid Hills

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	Т	L	R
Maximum Queue (ft)	214	192	94	175	211	105	49
Average Queue (ft)	109	61	13	102	104	39	11
95th Queue (ft)	186	147	49	183	200	84	37
Link Distance (ft)	804	804		156	156	122	122
Upstream Blk Time (%)				5	6	1	
Queuing Penalty (veh)				52	59	0	
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		2					
Queuing Penalty (veh)		1					

Intersection: 2: DW1 & North Druid Hills

Movement	WB	WB	NB
Directions Served	T	T	R
Maximum Queue (ft)	240	213	42
Average Queue (ft)	29	27	18
95th Queue (ft)	134	123	38
Link Distance (ft)	868	868	123
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 3: Birch Rd & DW2

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	54	4	35
Average Queue (ft)	27	0	4
95th Queue (ft)	50	3	21
Link Distance (ft)	130	282	122
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Zone Summary

Zone wide Queuing Penalty: 112

Existing AM SimTraffic Report Page 1

	-	•	•	←	~	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7	,,,,,	41∱	7	7
Traffic Volume (veh/h)	746	60	49	2030	51	29
Future Volume (veh/h)	746	60	49	2030	51	29
Initial Q (Qb), veh	0	0	0	2030	0	0
Ped-Bike Adj(A_pbT)	U	1.00	1.00	U	1.00	1.00
	1.00		1.00	1.00	1.00	1.00
Parking Bus, Adj		1.00	1.00	1.00		1.00
Work Zone On Approach	No	4070	4070	No	No	4070
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	811	65	53	2207	55	32
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2580	1151	69	2401	350	312
Arrive On Green	0.73	0.73	1.00	1.00	0.20	0.20
Sat Flow, veh/h	3647	1585	61	3393	1781	1585
Grp Volume(v), veh/h	811	65	1213	1047	55	32
Grp Sat Flow(s), veh/h/ln	1777	1585	1751	1617	1781	1585
Q Serve(g_s), s	12.2	1.8	0.0	0.0	3.8	2.5
Cycle Q Clear(g_c), s	12.2	1.8	0.0	0.0	3.8	2.5
Prop In Lane	12.2	1.00	0.04	0.0	1.00	1.00
	2500			1171		
Lane Grp Cap(c), veh/h	2580	1151	1297	1174	350	312
V/C Ratio(X)	0.31	0.06	0.94	0.89	0.16	0.10
Avail Cap(c_a), veh/h	2580	1151	1297	1174	350	312
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	7.3	5.9	0.0	0.0	49.9	49.4
Incr Delay (d2), s/veh	0.3	0.1	13.6	10.4	1.0	0.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.4	0.6	4.9	3.4	1.8	1.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	7.6	6.0	13.6	10.4	50.9	50.1
LnGrp LOS	Α	A	В	В	D	D
Approach Vol, veh/h	876	, , , , , , , , , , , , , , , , , , ,		2260	87	
Approach Delay, s/veh	7.5			12.2	50.6	
Approach LOS	Α			В	D	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		115.0		35.0		115.0
Change Period (Y+Rc), s		* 6.1		5.5		* 6.1
Max Green Setting (Gmax), s		* 1.1E2		29.5		* 1.1E2
Max Q Clear Time (g_c+l1), s		2.0		5.8		14.2
Green Ext Time (p_c), s		52.7		0.2		6.6
u = 7:		52.7		U.Z		0.0
Intersection Summary						
HCM 6th Ctrl Delay			11.9			
HCM 6th LOS			В			
Notes						

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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Intersection						
Int Delay, s/veh	0.1					
		EDD	MOL	MET	ND	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		^		- 7
Traffic Vol, veh/h	766	9	0	2079	0	35
Future Vol, veh/h	766	9	0	2079	0	35
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	833	10	0	2260	0	38
NA - ' /NA'			1.1.0		P	
	ajor1		//ajor2		Minor1	4.1-
Conflicting Flow All	0	0	-	-	-	417
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	-	-	0	-	0	585
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	-	-	-	-	585
Mov Cap-2 Maneuver	_	_	_	_	_	-
Stage 1	_	_	_	_	_	_
Stage 2	_	_	_	_	_	_
Olago Z						
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		11.6	
HCM LOS					В	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBT	
	-	585	LDI	LDIX	VVDI	
Capacity (veh/h)					-	
HCM Cantral Dalay (a)		0.065	-	-	-	
		0.11	-	-	-	
HCM Long LOS						
HCM Lane LOS HCM 95th %tile Q(veh)		B 0.2	-	-	-	

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Intersection						
Int Delay, s/veh	4.7					
		WED	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	- W		↑ }		^-	ની
Traffic Vol, veh/h	0	53	28	9	65	44
Future Vol, veh/h	0	53	28	9	65	44
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	58	30	10	71	48
NA - 1 (NA1	M		1.1.1		4.:-2	
	Minor1		Major1		Major2	
Conflicting Flow All	225	20	0	0	40	0
Stage 1	35	-	-	-	-	-
Stage 2	190	-	-	-	-	-
Critical Hdwy	6.63	6.93	-	-	4.13	-
Critical Hdwy Stg 1	5.83	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.519	3.319	-	-	2.219	-
Pot Cap-1 Maneuver	753	1053	-	-	1569	-
Stage 1	983	-	-	-	-	-
Stage 2	842	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	718	1053	-	-	1569	-
Mov Cap-2 Maneuver	718	-	_	_	_	_
Stage 1	983	_	_	_	-	_
Stage 2	803	<u>-</u>	_	_	<u>-</u>	_
Olago Z	300					
Approach	WB		NB		SB	
HCM Control Delay, s	8.6		0		4.4	
HCM LOS	Α					
Minar Lana/Majar Mym	.4	NDT	NDDV	MDI 51	CDI	CDT
Minor Lane/Major Mvn	IL	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-			1569	-
HCM Lane V/C Ratio		-		0.055		-
HCM Control Delay (s)		-	-	8.6	7.4	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh)	-	-	0.2	0.1	-

Synchro 10 Report Page 3 Build AM 08/10/2021

Movement EBT EBR WBL WBT NBL NBR Lane Configurations ↑↑ ↑
Lane Configurations ↑↑ ↑
Traffic Volume (veh/h) 746 60 49 2030 51 29 Future Volume (veh/h) 746 60 49 2030 51 29 Initial Q (Qb), veh 0 0 0 0 0 0 0 Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Parking Bus, Adj 1.00
Future Volume (veh/h) 746 60 49 2030 51 29 Initial Q (Qb), veh 0 0 0 0 0 0 0 Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Parking Bus, Adj 1.00 1
Initial Q (Qb), veh 0 0 0 0 0 0 Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 1.00 Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 1.00 Work Zone On Approach No No No No No No Adj Sat Flow, veh/h/In 1870
Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 1.00 Work Zone On Approach No No No No No Adj Sat Flow, veh/h/In 1870 1870 1870 1870 1870 Adj Flow Rate, veh/h 811 65 53 2207 55 32 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 Percent Heavy Veh, % 2 2 2 2 2 2 2 Cap, veh/h 3117 1390 78 2902 81 72 Arrive On Green 0.88 0.88 1.00 1.00 0.05 0.05 Sat Flow, veh/h 3647 1585 61 3393 1781 1585 Grp Volume(v), veh/h 811 65 1212 1048 55 32 Grp Sat Flow(s),veh/h/ln 1777 1585<
Parking Bus, Adj 1.00
Work Zone On Approach No No No Adj Sat Flow, veh/h/ln 1870 1870 1870 1870 1870 Adj Flow Rate, veh/h 811 65 53 2207 55 32 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 Percent Heavy Veh, % 2 2 2 2 2 2 2 Cap, veh/h 3117 1390 78 2902 81 72 Arrive On Green 0.88 0.88 1.00 1.00 0.05 0.05 Sat Flow, veh/h 3647 1585 61 3393 1781 1585 Grp Volume(v), veh/h 811 65 1212 1048 55 32 Grp Sat Flow(s),veh/h/In 1777 1585 1752 1617 1781 1585 Q Serve(g_s), s 5.4 0.8 0.0 0.0 4.6 3.0 Cycle Q Clear(g_c), s 5.4 0.8 0.0
Adj Sat Flow, veh/h/ln 1870 1820 187
Adj Flow Rate, veh/h 811 65 53 2207 55 32 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 Percent Heavy Veh, % 2<
Peak Hour Factor 0.92 0.05 0.05 0.05 0.05 0.05 0.05 0.05 0.05 0.05 0.05 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01
Percent Heavy Veh, % 2
Cap, veh/h 3117 1390 78 2902 81 72 Arrive On Green 0.88 0.88 1.00 1.00 0.05 0.05 Sat Flow, veh/h 3647 1585 61 3393 1781 1585 Grp Volume(v), veh/h 811 65 1212 1048 55 32 Grp Sat Flow(s), veh/h/ln 1777 1585 1752 1617 1781 1585 Q Serve(g_s), s 5.4 0.8 0.0 0.0 4.6 3.0 Cycle Q Clear(g_c), s 5.4 0.8 0.0 0.0 4.6 3.0
Arrive On Green 0.88 0.88 1.00 1.00 0.05 0.05 Sat Flow, veh/h 3647 1585 61 3393 1781 1585 Grp Volume(v), veh/h 811 65 1212 1048 55 32 Grp Sat Flow(s), veh/h/ln 1777 1585 1752 1617 1781 1585 Q Serve(g_s), s 5.4 0.8 0.0 0.0 4.6 3.0 Cycle Q Clear(g_c), s 5.4 0.8 0.0 0.0 4.6 3.0
Sat Flow, veh/h 3647 1585 61 3393 1781 1585 Grp Volume(v), veh/h 811 65 1212 1048 55 32 Grp Sat Flow(s),veh/h/ln 1777 1585 1752 1617 1781 1585 Q Serve(g_s), s 5.4 0.8 0.0 0.0 4.6 3.0 Cycle Q Clear(g_c), s 5.4 0.8 0.0 0.0 4.6 3.0
Grp Volume(v), veh/h 811 65 1212 1048 55 32 Grp Sat Flow(s), veh/h/ln 1777 1585 1752 1617 1781 1585 Q Serve(g_s), s 5.4 0.8 0.0 0.0 4.6 3.0 Cycle Q Clear(g_c), s 5.4 0.8 0.0 0.0 4.6 3.0
Grp Sat Flow(s),veh/h/ln 1777 1585 1752 1617 1781 1585 Q Serve(g_s), s 5.4 0.8 0.0 0.0 4.6 3.0 Cycle Q Clear(g_c), s 5.4 0.8 0.0 0.0 4.6 3.0
Q Serve(g_s), s 5.4 0.8 0.0 0.0 4.6 3.0 Cycle Q Clear(g_c), s 5.4 0.8 0.0 0.0 4.6 3.0
Cycle Q Clear(g_c), s 5.4 0.8 0.0 0.0 4.6 3.0
7 (0- //
Prop In Lane 1.00 0.04 1.00 1.00
Lane Grp Cap(c), veh/h 3117 1390 1562 1418 81 72
V/C Ratio(X) 0.26 0.05 0.78 0.74 0.68 0.44
Avail Cap(c_a), veh/h 3117 1390 1562 1418 350 312
HCM Platoon Ratio 1.00 1.00 2.00 2.00 1.00 1.00
Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 1.00
Uniform Delay (d), s/veh 1.5 1.2 0.0 0.0 70.5 69.7
Incr Delay (d2), s/veh 0.2 0.1 3.9 3.5 9.6 4.2
%ile BackOfQ(50%),veh/ln 1.0 0.1 1.7 1.4 2.3 1.3
Unsig. Movement Delay, s/veh
LnGrp Delay(d),s/veh 1.7 1.2 3.9 3.5 80.1 74.0
LnGrp LOS A A A F E
Approach Vol, veh/h 876 2260 87
Approach Delay, s/veh 1.6 3.7 77.8
Approach LOS A A E
Timer - Assigned Phs 2 4 6
Phs Duration (G+Y+Rc), s 137.7 12.3 137.7
Change Period (Y+Rc), s * 6.1 5.5 * 6.1
Max Green Setting (Gmax), s * 1.1E2 29.5 * 1.1E2
Max Q Clear Time (g_c+l1), s 2.0 6.6 7.4
Green Ext Time (p_c), s 52.7 0.2 6.6
Intersection Summary
HCM 6th Ctrl Delay 5.1
HCM 6th LOS A
Notes

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

BUILD AM ALT1 08/10/2021 Synchro 10 Report Page 1

Intersection						
Int Delay, s/veh	0.1					
		EDD	\A/D1	MOT	ND	NIDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	- 7		^		- 7
Traffic Vol, veh/h	766	9	0	2079	0	35
Future Vol, veh/h	766	9	0	2079	0	35
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	833	10	0	2260	0	38
		_		_		
	ajor1		//ajor2	N	/linor1	
Conflicting Flow All	0	0	-	-	-	417
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	_	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	_	-	-	-
Follow-up Hdwy	-	-	_	-	_	3.32
Pot Cap-1 Maneuver	-	-	0	-	0	585
Stage 1	_	_	0	_	0	-
Stage 2	_	_	0	_	0	_
Platoon blocked, %	_	_	•	_	•	
Mov Cap-1 Maneuver	_	_	_	_	_	585
Mov Cap-1 Maneuver		_	_	_	_	-
Stage 1	_	-	_	_	<u>-</u>	-
	-	-	-	_	-	-
Stage 2	-	-	-	-	-	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		11.6	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBT	
Capacity (veh/h)		585	-	-	-	
HCM Lane V/C Ratio		0.065	-	-	-	
HCM Control Delay (s)		11.6	-	-	-	
HCM Lane LOS		В	-	-	-	
HCM 95th %tile Q(veh)		0.2	-	-	-	

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Intersection						
Int Delay, s/veh	4.7					
•		WED	NOT	NDD	051	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		∱ }			4
Traffic Vol, veh/h	0	53	28	9	65	44
Future Vol, veh/h	0	53	28	9	65	44
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	58	30	10	71	48
	Minor1		/lajor1		Major2	
Conflicting Flow All	225	20	0	0	40	0
Stage 1	35	-	-	-	-	-
Stage 2	190	-	-	-	-	-
Critical Hdwy	6.63	6.93	-	-	4.13	-
Critical Hdwy Stg 1	5.83	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	_	-	-	_
Follow-up Hdwy	3.519	3.319	_	_	2.219	-
Pot Cap-1 Maneuver	753	1053	_	_	1569	_
Stage 1	983	-	_	_	-	_
Stage 2	842	_	_	_	_	_
Platoon blocked, %	J-12		_			_
Mov Cap-1 Maneuver	718	1053			1569	_
Mov Cap-1 Maneuver	718	1000		_	1009	
•	983	-	-	-	-	-
Stage 1				-	-	_
Stage 2	803	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.6		0		4.4	
HCM LOS	A					
	,,					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-		1053	1569	-
HCM Lane V/C Ratio		-	-	0.055	0.045	-
HCM Control Delay (s)		-	-		7.4	0
HCM Lane LOS		_	_	Α	Α	A
HCM 95th %tile Q(veh	1	_	_		0.1	_

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Intersection: 1: Birch Rd & North Druid Hills

Directions Served T T R LT T L R
Maximum Queue (ft) 138 127 53 162 153 118 31
Average Queue (ft) 43 21 8 51 45 61 4
95th Queue (ft) 105 72 31 132 134 115 20
Link Distance (ft) 804 804 156 156 122 122
Upstream Blk Time (%) 0 0 2
Queuing Penalty (veh) 5 0 1
Storage Bay Dist (ft) 85
Storage Blk Time (%) 0
Queuing Penalty (veh) 0

Build AM Optimize

SimTraffic Report

Page 1

	→	•	•	←	4	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414	ሻ	7
Traffic Volume (veh/h)	746	60	49	2030	51	29
Future Volume (veh/h)	746	60	49	2030	51	29
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	U	1.00	1.00	U	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No	1.00	1.00	No	No	1.00
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
				2207		32
Adj Flow Rate, veh/h	811	65	53		55	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3117	1390	78	2902	81	72
Arrive On Green	0.88	0.88	1.00	1.00	0.05	0.05
Sat Flow, veh/h	3647	1585	61	3393	1781	1585
Grp Volume(v), veh/h	811	65	1212	1048	55	32
Grp Sat Flow(s),veh/h/ln	1777	1585	1752	1617	1781	1585
Q Serve(g_s), s	5.4	0.8	0.0	0.0	4.6	3.0
Cycle Q Clear(g_c), s	5.4	0.8	0.0	0.0	4.6	3.0
Prop In Lane	5.4	1.00	0.04	0.0	1.00	1.00
	2117			1/10	81	72
Lane Grp Cap(c), veh/h	3117	1390	1562	1418		
V/C Ratio(X)	0.26	0.05	0.78	0.74	0.68	0.44
Avail Cap(c_a), veh/h	3117	1390	1562	1418	232	206
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	1.5	1.2	0.0	0.0	70.5	69.7
Incr Delay (d2), s/veh	0.2	0.1	3.9	3.5	9.6	4.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	0.1	1.7	1.4	2.3	1.3
Unsig. Movement Delay, s/veh		V.1				1.0
LnGrp Delay(d),s/veh	1.7	1.2	3.9	3.5	80.1	74.0
LnGrp LOS	Α	1.2 A		3.5 A	F	74.0 E
-		Α	A			
Approach Vol, veh/h	876			2260	87	
Approach Delay, s/veh	1.6			3.7	77.8	
Approach LOS	Α			Α	Е	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		137.7		12.3		137.7
Change Period (Y+Rc), s		* 6.1		5.5		* 6.1
Max Green Setting (Gmax), s		* 1.2E2		19.5		* 1.2E2
				6.6		
Max Q Clear Time (g_c+l1), s		2.0				7.4
Green Ext Time (p_c), s		54.6		0.1		6.6
Intersection Summary						
HCM 6th Ctrl Delay			5.1			
HCM 6th LOS			Α			
Notes						

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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Page 1

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	Т	L	R
Maximum Queue (ft)	245	210	120	168	194	109	64
Average Queue (ft)	135	92	19	98	66	45	28
95th Queue (ft)	221	180	64	176	173	93	57
Link Distance (ft)	804	804		157	157	123	123
Upstream Blk Time (%)				2	1	1	
Queuing Penalty (veh)				11	4	0	
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		5	0				
Queuing Penalty (veh)		3	0				

Intersection: 2: DW1 & North Druid Hills

Movement	WB	WB	NB
Directions Served	T	T	R
Maximum Queue (ft)	150	119	56
Average Queue (ft)	19	10	24
95th Queue (ft)	88	63	47
Link Distance (ft)	860	860	68
Upstream Blk Time (%)			0
Queuing Penalty (veh)			0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 3: Birch Rd & DW2

D' (' O		
Directions Served	LR	LT
Maximum Queue (ft)	68	60
Average Queue (ft)	30	10
95th Queue (ft)	54	38
Link Distance (ft)	123	123
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Zone Summary

Zone wide Queuing Penalty: 18

Existing MD SimTraffic Report Page 1

	-	•	•	←	1	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414		7
Traffic Volume (veh/h)	898	74	90	993	57	54
Future Volume (veh/h)	898	74	90	993	57	54
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	976	80	98	1079	62	59
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2559	1141	173	1902	380	338
Arrive On Green	0.72	0.72	1.00	1.00	0.21	0.21
Sat Flow, veh/h	3647	1585	201	2727	1781	1585
Grp Volume(v), veh/h	976	80	534	643	62	59
Grp Sat Flow(s), veh/h/ln	1777	1585	1225	1617	1781	1585
Q Serve(g_s), s	15.9	2.2	11.4	0.0	4.3	4.6
Cycle Q Clear(g_c), s	15.9	2.2	27.3	0.0	4.3	4.6
Prop In Lane	. 5.0	1.00	0.18	3.0	1.00	1.00
Lane Grp Cap(c), veh/h	2559	1141	911	1164	380	338
V/C Ratio(X)	0.38	0.07	0.59	0.55	0.16	0.17
Avail Cap(c_a), veh/h	2559	1141	911	1164	380	338
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	8.1	6.2	0.8	0.0	48.1	48.2
Incr Delay (d2), s/veh	0.4	0.1	2.8	1.9	0.9	1.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.7	0.7	0.7	0.6	2.0	1.9
Unsig. Movement Delay, s/veh		J .,	J .,	3.0		- 1.0
LnGrp Delay(d),s/veh	8.5	6.3	3.5	1.9	49.0	49.3
LnGrp LOS	A	A	A	A	D	43.0 D
Approach Vol, veh/h	1056	, , , , , , , , , , , , , , , , , , ,	,,	1177	121	
Approach Delay, s/veh	8.4			2.6	49.2	
Approach LOS	Α			2.0 A	43.2 D	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		113.0		37.0		113.0
Change Period (Y+Rc), s		5.0		5.0		5.0
Max Green Setting (Gmax), s		108.0		32.0		108.0
Max Q Clear Time (g_c+l1), s		29.3		6.6		17.9
Green Ext Time (p_c), s		11.7		0.3		8.7
Intersection Summary						
HCM 6th Ctrl Delay			7.6			
HCM 6th LOS			A			
HOW OUT LOO			\wedge			

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Intersection Int Delay, s/veh O.4 Movement EBT EBR WBL WBT NBL NBR NBK Traffic Vol, veh/h 938 14 O 1083 O 67 Future Vol, veh/h 938 14 O 1083 O 67 Future Vol, veh/h 938 14 O 1083 O 67 Conflicting Peds, #/hr O O O O O O O O O
Movement EBT EBR WBL WBT NBL NBR Lane Configurations ↑↑ ० 0
Lane Configurations ↑↑
Traffic Vol, veh/h 938 14 0 1083 0 67 Future Vol, veh/h 938 14 0 1083 0 67 Conflicting Peds, #/hr 0 0 0 0 0 0 0 Sign Control Free Free Free Free Free Free Stop Stop RT Channelized - None - -
Future Vol, veh/h Conflicting Peds, #/hr O O O O O O O O O O O O O O O O O O O
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Stop Stop Stop RT Channelized - None - O 0 - - 0 0 - - 0 0 - - 0 0 - - 2 2 2 2 2 2 2 2 2 2 2<
Sign Control Free Free Free Free Free Stop Stop RT Channelized - None - None - None - None Storage Length - 0 - 0 0 - 0 Veh in Median Storage, # 0 0 0 0 0 0 Grade, % 0
RT Channelized - None - None - None Storage Length - 0 0 0 0 0 - 0 0 Veh in Median Storage, # 0 0 0 0 - 0 - 0 0 0 - 0 0 Grade, % 0 0 0 0 - 0 0 - 0 0 0 - 0 - 0 0 0 0 0 0 0 Peak Hour Factor 92 92 92 92 92 92 92 92 92 92 92 92 92 92 Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
Storage Length - 0 - - 0 Veh in Median Storage, # 0 - - 0 0 Grade, % 0 - - 0 0 - Peak Hour Factor 92
Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 92 92 92 92 92 92 Heavy Vehicles, % 2 3 <td< td=""></td<>
Grade, % 0 - - 0 0 - Peak Hour Factor 92 93 93 93 93 92 93 93 93 93 93 93
Peak Hour Factor 92 93 93 93 93 93 93 93 92 93 93 93 93 92 92 92 92 92 92 92 92 92 92 92 92 92 93 93 93 93 93
Meavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
Mvmt Flow 1020 15 0 1177 0 73 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 - - 510 Stage 1 - - - - - - 510 Stage 2 -
Mvmt Flow 1020 15 0 1177 0 73 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 - - 510 Stage 1 - - - - - - 510 Stage 2 -
Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 - - 510 Stage 1 - - - - - - - 510 Stage 2 -
Conflicting Flow All 0 0 - - 510 Stage 1 -
Conflicting Flow All 0 0 - - 510 Stage 1 -
Stage 1 - </td
Stage 2 - - - - - Critical Hdwy Stg 1 - - - - - - Critical Hdwy Stg 2 -
Critical Hdwy - - - 6.94 Critical Hdwy Stg 1 - - - - - Critical Hdwy Stg 2 - - - - - Follow-up Hdwy - - - - 0 509 Stage 1 - - 0 - 0 - 0 Stage 2 - - 0 - 0 -
Critical Hdwy Stg 1
Critical Hdwy Stg 2
Critical Hdwy Stg 2
Follow-up Hdwy 3.32 Pot Cap-1 Maneuver 0 - 0 509 Stage 1 0 - 0 - 0 - 509 Stage 2 0 - 0 - 0 - 509 Platoon blocked, % 509 Mov Cap-1 Maneuver 509 Mov Cap-2 Maneuver 509 Stage 1
Pot Cap-1 Maneuver - - 0 - 0 509 Stage 1 - - 0 - 0 - Stage 2 - - 0 - 0 - Platoon blocked, % - - - - - - Mov Cap-1 Maneuver - - - - - - - 509 Mov Cap-2 Maneuver -
Stage 1 - - 0 - 0 - Stage 2 - - 0 - 0 - Platoon blocked, % - - - - - Mov Cap-1 Maneuver - - - - - 509 Mov Cap-2 Maneuver -
Stage 2 - - 0 - 0 - Platoon blocked, % - - - - - Mov Cap-1 Maneuver - - - - - 509 Mov Cap-2 Maneuver -
Platoon blocked, % - - - Mov Cap-1 Maneuver - - - - 509 Mov Cap-2 Maneuver - </td
Mov Cap-1 Maneuver - - - 509 Mov Cap-2 Maneuver - - - - - - Stage 1 - - - - - - - Stage 2 - - - - - - - - Approach EB WB NB HCM Control Delay, s 0 0 13.3
Mov Cap-2 Maneuver -
Stage 1 - </td
Stage 2 - </td
Approach EB WB NB HCM Control Delay, s 0 0 13.3
HCM Control Delay, s 0 0 13.3
HCM Control Delay, s 0 0 13.3
HCM Control Delay, s 0 0 13.3
Maria Maria Maria Albi 4 EDT EDD MEDT
Minor Lane/Major Mvmt NBLn1 EBT EBR WBT
Capacity (veh/h) 509
HCM Lane V/C Ratio 0.143
HCM Control Delay (s) 13.3
HCM Lane LOS B
HCM 95th %tile Q(veh) 0.5

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<u> </u>					
	WBR	NBT	NBR	SBL	SBT
N/F		∱ }			ની
0	67	44			49
0	67	44	14	114	49
0	0	0	0	0	0
Stop	Stop	Free	Free	Free	Free
-	None	-	None	-	None
0	-	-	-	-	-
e, # 0	-	0	-	-	0
0	_	0	-	_	0
92	92	92	92	92	92
					2
					53
- 3	10		- 10	, L 1	- 00
		//ajor1			
357	32	0	0	63	0
56	-	-	-	-	-
301	-	-	-	-	-
6.63	6.93	-	-	4.13	-
5.83	-	-	-	_	-
	-	_	_	_	_
	3.319	-	-	2.219	-
		-	_		-
	-	_	_	-	_
	_	_	_	_	_
100		_	_		_
576	1035			1530	_
					_
		-	-	<u>-</u>	-
			-	=	_
000	-	-	-	-	-
MD		NB		SB	
WB					
		0		5.3	
8.7				5.3	
				5.3	
8.7 A		0			
8.7	NBT	0	VBLn1	SBL	SBT
8.7 A	NBT -	0	1035	SBL 1539	SBT_
8.7 A	NBT -	0 NBRV	1035 0.07	SBL 1539 0.081	-
8.7 A	-	0 NBRV	1035	SBL 1539	-
8.7 A	-	0 NBRV -	1035 0.07	SBL 1539 0.081	-
	0 0 Stop 0 92 2 0 Minor1 357 56 301 6.63 5.83 5.43 3.519 628 960 750 576 960 688	WBL WBR 0 67 0 67 0 0 67 0 0 0 Stop Stop - None 0 9, # 0 92 92 2 2 2 0 73 Minor1 N 357 32 56 301 6.63 6.93 5.83 5.43 3.519 3.319 628 1035 960 750 576 1035 576 960 688	WBL WBR NBT 0 67 44 0 67 44 0 0 0 0 Stop Stop Free - None 0 0 - 0 92 92 92 2 2 2 0 73 48 Minor1 Major1 357 32 0 56 301 6.63 6.93 - 5.83 5.43 3.519 3.319 - 628 1035 - 960 750 576 1035 - 960 576 960	WBL WBR NBT NBR 0 67 44 14 0 67 44 14 0 0 0 0 Stop Stop Free Free - None - None 0 - - - 0 - 0 - 92 92 92 92 2 2 2 2 2 2 2 2 357 32 0 0 56 - - - 301 - - - 5.83 - - - 5.43 - - - 3.519 3.319 - - 628 1035 - - 750 - - - 576 - - - 960 - - <td< td=""><td>WBL WBR NBT NBR SBL Y 15 14 14 114 0 67 44 14 114 0 0 0 0 0 Stop Stop Free Free Free - None - None - 0 - - - - 9, # 0 - 0 - - 92 92 92 92 92 2 2 2 2 2 2 2 2 2 2 2 2 2 2 357 32 0 0 63 56 - - - - - 301 -</td></td<>	WBL WBR NBT NBR SBL Y 15 14 14 114 0 67 44 14 114 0 0 0 0 0 Stop Stop Free Free Free - None - None - 0 - - - - 9, # 0 - 0 - - 92 92 92 92 92 2 2 2 2 2 2 2 2 2 2 2 2 2 2 357 32 0 0 63 56 - - - - - 301 -

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	→	•	•	•	4	/
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414	ሻ	7
Traffic Volume (veh/h)	898	74	90	993	57	54
Future Volume (veh/h)	898	74	90	993	57	54
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	976	80	98	1079	62	59
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3133	1397	222	2397	92	82
Arrive On Green	0.88	0.88	1.00	1.00	0.05	0.05
Sat Flow, veh/h	3647	1585	219	2805	1781	1585
Grp Volume(v), veh/h	976	80	534	643	62	59
Grp Sat Flow(s), veh/h/ln	1777	1585	1322	1617	1781	1585
Q Serve(g_s), s	6.7	0.9	0.0	0.0	5.1	5.5
Cycle Q Clear(g_c), s	6.7	0.9	0.0	0.0	5.1	5.5
Prop In Lane	J .,	1.00	0.18	3.0	1.00	1.00
Lane Grp Cap(c), veh/h	3133	1397	1194	1425	92	82
V/C Ratio(X)	0.31	0.06	0.45	0.45	0.67	0.72
Avail Cap(c_a), veh/h	3133	1397	1194	1425	380	338
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	1.5	1.1	0.0	0.0	69.9	70.0
Incr Delay (d2), s/veh	0.3	0.1	1.2	1.0	8.2	11.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.2	0.2	0.4	0.4	2.6	2.5
Unsig. Movement Delay, s/veh		J.L	J. 1	J. 1	2.0	2.0
LnGrp Delay(d),s/veh	1.7	1.2	1.2	1.0	78.1	81.2
LnGrp LOS	A	Α	Α	Α	70.1 E	F
Approach Vol, veh/h	1056	/\	/\	1177	121	
Approach Delay, s/veh	1.7			1.1	79.6	
Approach LOS	Α			Α	7 5.0 E	
	A			A		
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		137.2		12.8		137.2
Change Period (Y+Rc), s		5.0		5.0		5.0
Max Green Setting (Gmax), s		108.0		32.0		108.0
Max Q Clear Time (g_c+l1), s		2.0		7.5		8.7
Green Ext Time (p_c), s		11.7		0.3		8.7
Intersection Summary						
HCM 6th Ctrl Delay			5.4			
HCM 6th LOS						
HOW OUT LOS			Α			

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Intersection						
Int Delay, s/veh	0.4					
		EDD	14/51	MET	ND	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		^		7
Traffic Vol, veh/h	938	14	0	1083	0	67
Future Vol, veh/h	938	14	0	1083	0	67
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
	1020	15	0	1177	0	73
	ajor1		//ajor2	N	/linor1	
Conflicting Flow All	0	0	-	-	-	510
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	_	-	_	-	_	-
Follow-up Hdwy	_	_	_	_	_	3.32
Pot Cap-1 Maneuver	_	_	0	_	0	509
Stage 1	_	_	0	<u>-</u>	0	-
Stage 1	<u>-</u>	-	0	_	0	-
Platoon blocked, %	-	<u>-</u>	U	_	U	-
	-	-				E00
Mov Cap-1 Maneuver	-	-	-	-	-	509
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
			0		13.3	
HCM Control Delay, s	0		U			
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBT	
Capacity (veh/h)		509				
HCM Lane V/C Ratio		0.143	_	-	_	
HCM Control Delay (s)		13.3	-	_	-	
HCM Lane LOS		13.3 B				
			-	-	-	
HCM 95th %tile Q(veh)		0.5	-	-	-	

BUILD MD ALT 1 08/10/2021 Synchro 10 Report Page 2

Intersection						
Int Delay, s/veh	5					
•						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		∱ ∱			सी
Traffic Vol, veh/h	0	67	44	14	114	49
Future Vol, veh/h	0	67	44	14	114	49
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	_	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	73	48	15	124	53
Major/Miner	Minari		Anic 1		Mais = 0	
	Minor1		Major1		Major2	
Conflicting Flow All	357	32	0	0	63	0
Stage 1	56	-	-	-	-	-
Stage 2	301	-	-	-	-	-
Critical Hdwy	6.63	6.93	-	-	4.13	-
Critical Hdwy Stg 1	5.83	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.519	3.319	-	-	2.219	-
Pot Cap-1 Maneuver	628	1035	-	-	1539	-
Stage 1	960	-	-	-	-	-
Stage 2	750	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	576	1035	-	_	1539	-
Mov Cap-2 Maneuver	576	-	_	_	-	_
Stage 1	960	-	-	-	_	-
Stage 2	688	_	_	_	_	_
Olago Z	300					
Approach	WB		NB		SB	
HCM Control Delay, s	8.7		0		5.3	
HCM LOS	Α					
Minor Long/Major My	. +	NBT	NDDV	VBLn1	SBL	SBT
Minor Lane/Major Mvn	ιι	INDI				
Capacity (veh/h)		-	-	1035	1539	-
HCM Lane V/C Ratio		-	-		0.081	-
HCM Control Delay (s)		-	-	8.7	7.5	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh				0.2	0.3	

BUILD MD ALT 1 08/10/2021 Synchro 10 Report Page 3

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	T	L	R
Maximum Queue (ft)	120	73	31	163	169	123	54
Average Queue (ft)	35	12	4	68	18	62	27
95th Queue (ft)	91	44	20	146	92	112	55
Link Distance (ft)	804	804		157	157	123	123
Upstream Blk Time (%)				1	0	1	
Queuing Penalty (veh)				3	1	1	
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		0					
Queuing Penalty (veh)		0					

Build MD Opt1 SimTraffic Report
Page 1

	-	\rightarrow	•	←	1	/
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414	ች	7
Traffic Volume (veh/h)	898	74	90	993	57	54
Future Volume (veh/h)	898	74	90	993	57	54
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	976	80	98	1079	62	59
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	3135	1398	222	2399	91	81
Arrive On Green	0.88	0.88	1.00	1.00	0.05	0.05
Sat Flow, veh/h	3647	1585	219	2805	1781	1585
Grp Volume(v), veh/h	976	80	534	643	62	59
Grp Sat Flow(s), veh/h/ln	1777	1585	1322	1617	1781	1585
Q Serve(g_s), s	6.7	0.9	0.0	0.0	5.1	5.5
Cycle Q Clear(g_c), s	6.7	0.9	0.0	0.0	5.1	5.5
Prop In Lane		1.00	0.18	3.0	1.00	1.00
Lane Grp Cap(c), veh/h	3135	1398	1195	1426	91	81
V/C Ratio(X)	0.31	0.06	0.45	0.45	0.68	0.73
Avail Cap(c_a), veh/h	3135	1398	1195	1426	238	211
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	1.4	1.1	0.0	0.0	69.9	70.1
Incr Delay (d2), s/veh	0.3	0.1	1.2	1.0	8.5	11.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.2	0.2	0.4	0.4	2.6	2.5
Unsig. Movement Delay, s/vel		J.L	J. 1	J. 1	2.0	2.0
LnGrp Delay(d),s/veh	1.7	1.2	1.2	1.0	78.5	81.8
LnGrp LOS	A	A	A	A	7 0.0 E	F
Approach Vol, veh/h	1056	, , , , , , , , , , , , , , , , , , ,	,,	1177	121	
Approach Delay, s/veh	1.7			1.1	80.1	
Approach LOS	Α			Α	F	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		137.3		12.7		137.3
Change Period (Y+Rc), s		5.0		5.0		5.0
Max Green Setting (Gmax), s		120.0		20.0		120.0
Max Q Clear Time (g_c+l1), s		2.0		7.5		8.7
Green Ext Time (p_c), s		11.7		0.2		8.7
Intersection Summary						
HCM 6th Ctrl Delay			5.4			
HCM 6th LOS			3. 4			
HOW OUT LOS			^			

Build MD Opt1 08/10/2021 Synchro 10 Report Page 1

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	T	L	R
Maximum Queue (ft)	4878	4868	125	177	227	130	119
Average Queue (ft)	2659	2669	74	100	106	48	48
95th Queue (ft)	5456	5446	170	207	246	100	96
Link Distance (ft)	4950	4950		158	158	126	126
Upstream Blk Time (%)	13	14		7	7	1	0
Queuing Penalty (veh)	0	0		43	43	0	0
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		44	0				
Queuing Penalty (veh)		56	0				

Intersection: 2: DW1 & North Druid Hills

Movement	EB	EB	WB	WB	NB
Directions Served	Т	Т	Т	Т	R
Maximum Queue (ft)	190	190	196	202	103
Average Queue (ft)	129	136	38	31	29
95th Queue (ft)	228	232	142	132	96
Link Distance (ft)	158	158	810	810	317
Upstream Blk Time (%)	6	8			
Queuing Penalty (veh)	39	56			
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 3: Birch Rd & DW2

WB	NB	SB
LR	T	LT
57	6	44
30	0	5
45	5	24
164	320	126
	LR 57 30 45	LR T 57 6 30 0 45 5

Zone Summary

Zone wide Queuing Penalty: 238

BuildPM SimTraffic Report Page 1

	→	•	•	•	1	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414	ሻ	7
Traffic Volume (veh/h)	2040	129	31	1166	58	54
Future Volume (veh/h)	2040	129	31	1166	58	54
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00	•	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No	1.00	1.00	No	No	1.00
Adj Sat Flow, veh/h/ln	1477	1477	1477	1477	1477	1477
Adj Flow Rate, veh/h	2103	140	34	1267	63	59
Peak Hour Factor	0.97	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
•						
Cap, veh/h	2127	949	24	1270	244	217
Arrive On Green	0.76	0.76	1.00	1.00	0.17	0.17
Sat Flow, veh/h	2879	1251	2	1742	1406	1251
Grp Volume(v), veh/h	2103	140	576	725	63	59
Grp Sat Flow(s),veh/h/ln	1403	1251	400	1277	1406	1251
Q Serve(g_s), s	123.0	5.2	5.9	0.0	6.6	7.0
Cycle Q Clear(g_c), s	123.0	5.2	128.9	0.0	6.6	7.0
Prop In Lane		1.00	0.06		1.00	1.00
Lane Grp Cap(c), veh/h	2127	949	326	968	244	217
V/C Ratio(X)	0.99	0.15	1.77	0.75	0.26	0.27
Avail Cap(c_a), veh/h	2127	949	326	968	244	217
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	19.8	5.6	68.5	0.0	60.8	60.9
Incr Delay (d2), s/veh	17.0	0.3	357.7	5.3	2.5	3.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	39.1	1.4	43.2	1.4	2.6	2.4
, , ,		1.4	43.2	1.4	2.0	2.4
Unsig. Movement Delay, s/vel		F 0	400.0	F 2	C2 2	C4 0
LnGrp Delay(d),s/veh	36.8	5.9	426.2	5.3	63.3	64.0
LnGrp LOS	D	A	F	Α	E	E
Approach Vol, veh/h	2243			1301	122	
Approach Delay, s/veh	34.9			191.7	63.7	
Approach LOS	С			F	Е	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		135.0		35.0		135.0
Change Period (Y+Rc), s		* 6.1		5.5		* 6.1
Max Green Setting (Gmax), s		* 1.3E2		29.5		* 1.3E2
Max Q Clear Time (g_c+l1), s		130.9		9.0		125.0
Green Ext Time (p_c), s		0.0		0.3		3.6
Intersection Summary						
HCM 6th Ctrl Delay			91.5			
HCM 6th LOS			F			
Notes						

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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Intersection						
Int Delay, s/veh	0.1					
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	- 7		^		7
	2073	1	0	1197	0	8
Future Vol, veh/h	2073	1	0	1197	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	0
Veh in Median Storage, 7	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
	2253	1	0	1301	0	9
		•			•	
		_		_		
	ajor1		//ajor2	N	/linor1	
Conflicting Flow All	0	0	-	-	-	1127
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	_	-	-	_	3.32
Pot Cap-1 Maneuver	_	-	0	-	0	199
Stage 1	_	_	0	_	0	-
Stage 2	_	_	0	_	0	_
Platoon blocked, %	_	_		_	•	
Mov Cap-1 Maneuver	_	_	_	_	_	199
Mov Cap-2 Maneuver	_	_	_	_	_	199
Stage 1	-	_	_			_
		_		_	_	-
Stage 2	-	-	-	-	-	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		23.9	
HCM LOS					С	
NA' 1 /NA - ' NA 1		UDL .4	CDT	EDD	MOT	
Minor Lane/Major Mvmt	ľ	NBLn1	EBT	EBR	WBT	
Capacity (veh/h)		199	-	-	-	
		0.044	-	-	-	
HCM Lane V/C Ratio						
HCM Control Delay (s)		23.9	-	-	-	
					-	

Build PM 08/10/2021 Synchro 10 Report Page 2

Intersection						
Int Delay, s/veh	3.9					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		↑ }			ની
Traffic Vol, veh/h	0	69	53	8	69	91
Future Vol, veh/h	0	69	53	8	69	91
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	-	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	75	58	9	75	99
Major/Minor	Minor1	, n	Aniar1		Major	
	Minor1		Major1		Major2	
Conflicting Flow All	312	34	0	0	67	0
Stage 1	63	-	-	-	-	-
Stage 2	249	-	-	-	-	-
Critical Hdwy	6.63	6.93	-	-	4.13	-
Critical Hdwy Stg 1	5.83	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy		3.319	-	-	2.219	-
Pot Cap-1 Maneuver	668	1032	-	-	1534	-
Stage 1	953	-	-	-	-	-
Stage 2	792	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	633	1032	-	-	1534	-
Mov Cap-2 Maneuver	633	_	-	_	_	-
Stage 1	953	_	-	_	-	-
Stage 2	751	<u>-</u>	_	_	<u>-</u>	_
Clayo Z	, 0 1					
Approach	WB		NB		SB	
HCM Control Delay, s	8.8		0		3.2	
HCM LOS	Α					
Minor Lane/Major Mvm	\	NBT	NRDV	VBLn1	SBL	SBT
	IL					
Capacity (veh/h)		-		1032		-
HCM Lane V/C Ratio		-		0.073		-
HCM Control Delay (s)		-	-	8.8	7.5	0
HCM Lane LOS		-	-	A	A	Α
HCM 95th %tile Q(veh)	-	-	0.2	0.2	-

Synchro 10 Report Page 3 Build PM 08/10/2021

Movement	EB	EB	EB	WB	WB	WB	NB	NB
Directions Served	Т	Т	R	L	Т	Т	L	R
Maximum Queue (ft)	4887	4895	125	80	100	94	114	114
Average Queue (ft)	2591	2605	73	31	18	27	54	48
95th Queue (ft)	5482	5483	168	73	61	74	104	101
Link Distance (ft)	4951	4951			158	158	120	120
Upstream Blk Time (%)	18	19			0		2	1
Queuing Penalty (veh)	0	0			0		1	1
Storage Bay Dist (ft)			85	250				
Storage Blk Time (%)		42	0		0			
Queuing Penalty (veh)		54	0		0			

Build PM Alternate 3 SimTraffic Report
Page 1

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7	ች	^	ች	7
Traffic Volume (veh/h)	2040	129	31	1166	58	54
Future Volume (veh/h)	2040	129	31	1166	58	54
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	Ū	1.00	1.00	•	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No	1.00	1.00	No	No	1.00
Adj Sat Flow, veh/h/ln	1477	1477	1477	1477	1477	1477
Adj Flow Rate, veh/h	2103	140	34	1267	63	59
Peak Hour Factor	0.97	0.92	0.92	0.92	0.92	0.92
	0.97			0.92	0.92	0.92
Percent Heavy Veh, %		2	2			
Cap, veh/h	2144	956	83	2292	161	144
Arrive On Green	0.76	0.76	0.05	1.00	0.11	0.11
Sat Flow, veh/h	2879	1251	1406	2879	1406	1251
Grp Volume(v), veh/h	2103	140	34	1267	63	59
Grp Sat Flow(s),veh/h/ln	1403	1251	1406	1403	1406	1251
Q Serve(g_s), s	120.0	5.1	8.0	0.0	7.1	7.4
Cycle Q Clear(g_c), s	120.0	5.1	0.8	0.0	7.1	7.4
Prop In Lane		1.00	1.00		1.00	1.00
Lane Grp Cap(c), veh/h	2144	956	83	2292	161	144
V/C Ratio(X)	0.98	0.15	0.41	0.55	0.39	0.41
Avail Cap(c_a), veh/h	2144	956	216	2292	161	144
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	18.9	5.3	52.2	0.0	69.7	69.9
Incr Delay (d2), s/veh	15.4	0.3	3.2	1.0	7.0	8.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	37.4	1.3	1.2	0.3	2.9	5.6
Unsig. Movement Delay, s/veh		- ^	·	4.0	70 -	70.4
LnGrp Delay(d),s/veh	34.3	5.6	55.4	1.0	76.7	78.4
LnGrp LOS	С	A	<u>E</u>	A	E	E
Approach Vol, veh/h	2243			1301	122	
Approach Delay, s/veh	32.5			2.4	77.5	
Approach LOS	С			Α	Е	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		145.0		25.0	9.0	136.0
, , ,		* 6.1			5.0	* 6.1
Change Period (Y+Rc), s				5.5		
Max Green Setting (Gmax), s		* 1.4E2		19.5	20.0	* 1.1E2
Max Q Clear Time (g_c+l1), s		2.0		9.4	2.8	122.0
Green Ext Time (p_c), s		12.8		0.2	0.0	0.0
Intersection Summary						
HCM 6th Ctrl Delay			23.3			
HCM 6th LOS			С			
Notes						

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Build PM Alternate 3 08/10/2021 Synchro 10 Report
Page 1

Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	Т	Т	R	LT	Т	L	R
Maximum Queue (ft)	4994	4995	125	182	234	127	135
Average Queue (ft)	2594	2606	69	101	108	55	56
95th Queue (ft)	5601	5596	164	212	248	114	121
Link Distance (ft)	4950	4950		158	158	126	126
Upstream Blk Time (%)	16	15		6	6	1	2
Queuing Penalty (veh)	0	0		35	33	1	1
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		42	0				
Queuing Penalty (veh)		54	0				

Intersection: 2: DW1 & North Druid Hills

Movement	EB	EB	EB	WB	WB	NB
Directions Served	T	T	R	Т	T	R
Maximum Queue (ft)	196	189	101	163	152	81
Average Queue (ft)	141	143	4	33	28	22
95th Queue (ft)	233	229	43	118	107	67
Link Distance (ft)	158	158	158	810	810	317
Upstream Blk Time (%)	14	16	0			
Queuing Penalty (veh)	99	113	2			
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 3: Birch Rd & DW2

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	77	12	36
Average Queue (ft)	30	0	5
95th Queue (ft)	56	6	25
Link Distance (ft)	164	320	126
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Zone Summary

Zone wide Queuing Penalty: 337

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	₹	1100	41∱	7	7
Traffic Volume (veh/h)	2040	129	31	1166	58	54
Future Volume (veh/h)	2040	129	31	1166	58	54
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	U	1.00	1.00	U	1.00	1.00
	1.00		1.00	1.00	1.00	1.00
Parking Bus, Adj		1.00	1.00	1.00		1.00
Work Zone On Approach	No	4 4 7 7	4 477	No	No	4 4 7 7
Adj Sat Flow, veh/h/ln	1477	1477	1477	1477	1477	1477
Adj Flow Rate, veh/h	2103	140	34	1267	63	59
Peak Hour Factor	0.97	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2446	1091	49	1825	84	75
Arrive On Green	0.87	0.87	1.00	1.00	0.06	0.06
Sat Flow, veh/h	2879	1251	30	2160	1406	1251
Grp Volume(v), veh/h	2103	140	591	710	63	59
Grp Sat Flow(s), veh/h/ln	1403	1251	847	1277	1406	1251
Q Serve(g_s), s	65.2	2.7	83.1	0.0	7.5	7.9
Cycle Q Clear(g_c), s	65.2	2.7	148.2	0.0	7.5	7.9
Prop In Lane	03.2	1.00	0.06	0.0	1.00	1.00
•	2446			1112		
Lane Grp Cap(c), veh/h	2446	1091	761	1113	84	75
V/C Ratio(X)	0.86	0.13	0.78	0.64	0.75	0.79
Avail Cap(c_a), veh/h	2446	1091	761	1113	244	217
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	5.6	1.6	13.1	0.0	78.7	78.9
Incr Delay (d2), s/veh	4.2	0.2	7.7	2.8	12.4	16.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	12.9	0.5	15.8	0.9	3.0	2.9
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	9.8	1.8	20.8	2.8	91.1	95.3
LnGrp LOS	A	A	C	Α	F	F
Approach Vol, veh/h	2243	,,		1301	122	
Approach Delay, s/veh	9.3			11.0	93.1	
Approach LOS	Α			В	F	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		154.3		15.7		154.3
Change Period (Y+Rc), s		* 6.1		5.5		* 6.1
Max Green Setting (Gmax), s		* 1.3E2		29.5		* 1.3E2
Max Q Clear Time (g_c+l1), s		150.2		9.9		67.2
Green Ext Time (p_c), s		0.0		0.3		34.5
Intersection Summary						
HCM 6th Ctrl Delay			12.7			
HCM 6th LOS			В			
Notes						

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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Interception						
Intersection	0.1					
Int Delay, s/veh						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		^		7
Traffic Vol, veh/h	2073	1	0	1197	0	8
Future Vol, veh/h	2073	1	0	1197	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
	2253	1	0	1301	0	9
MVIIICI ION		•	•	1001		J
Major/Minor Ma	ajor1	N	/lajor2	N	/linor1	
Conflicting Flow All	0	0	-	-	-	1127
Stage 1	-	-	-	-	-	
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	_	_	0	-	0	199
Stage 1	-	_	0	_	0	-
Stage 2	_	_	0	_	0	_
Platoon blocked, %	_	_	· ·	_	J	
Mov Cap-1 Maneuver	_	_		_	_	199
Mov Cap-1 Maneuver	_	-	_	_	-	133
		-	_	-	<u>-</u>	-
Stage 1		-		_	<u>-</u>	
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
Approach HCM Control Delay, s	EB 0		WB 0			
HCM Control Delay, s					23.9	
HCM Control Delay, s HCM LOS	0		0		23.9 C	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt	0	NBLn1		EBR	23.9	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	0	199	0	EBR -	23.9 C	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	0	199 0.044	0 EBT	EBR -	23.9 C	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	0	199	0 EBT	EBR - -	23.9 C	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	0	199 0.044	0 EBT -	EBR - -	23.9 C	

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Intersection						
Int Delay, s/veh	3.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		Λ₽			4
Traffic Vol, veh/h	0	69	53	8	69	91
Future Vol, veh/h	0	69	53	8	69	91
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	75	58	9	75	99
				•		
	Minor1		Major1		Major2	
Conflicting Flow All	312	34	0	0	67	0
Stage 1	63	-	-	-	-	-
Stage 2	249	-	-	-	-	-
Critical Hdwy	6.63	6.93	-	-	4.13	-
Critical Hdwy Stg 1	5.83	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.519	3.319	-	-	2.219	-
Pot Cap-1 Maneuver	668	1032	_	_	1534	_
Stage 1	953	-	_	_	-	_
Stage 2	792	-	-	_	-	-
Platoon blocked, %	.02		_	_		_
Mov Cap-1 Maneuver	633	1032	_	_	1534	_
Mov Cap-1 Maneuver	633	1002			100-	_
Stage 1	953	-	-	-	-	-
	751	-	-	-	-	-
Stage 2	751	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.8		0		3.2	
HCM LOS	A					
	, ,					
Minor Lane/Major Mvn	nt	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-			1534	-
HCM Lane V/C Ratio		-	-	0.073	0.049	-
HCM Control Delay (s)	-	-	8.8	7.5	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh	1)	-	-	0.2	0.2	-

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Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	T	T	R	LT	T	L	R
Maximum Queue (ft)	4986	4998	125	188	233	69	79
Average Queue (ft)	2741	2755	73	154	170	32	30
95th Queue (ft)	5527	5527	165	203	238	64	65
Link Distance (ft)	4950	4950		158	158	126	126
Upstream Blk Time (%)	19	19		13	15		0
Queuing Penalty (veh)	0	0		76	91		0
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		44	0				
Queuing Penalty (veh)		57	0				

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	ZDI€		41∱	ሻ	7
Traffic Volume (veh/h)	2040	129	31	1166	58	54
Future Volume (veh/h)	2040	129	31	1166	58	54
Initial Q (Qb), veh	0	0	0	0	0	0
	U	1.00		U	1.00	1.00
Ped-Bike Adj(A_pbT)	4.00		1.00	4.00		
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1477	1477	1477	1477	1477	1477
Adj Flow Rate, veh/h	2103	140	34	1267	63	59
Peak Hour Factor	0.97	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2220	990	53	1670	102	90
Arrive On Green	0.79	0.79	1.00	1.00	0.07	0.07
Sat Flow, veh/h	2879	1251	11	2177	1406	1251
Grp Volume(v), veh/h	2103	140	621	680	63	59
Grp Sat Flow(s), veh/h/ln	1403	1251	845	1277	1406	1251
, , , , , , , , , , , , , , , , , , , ,						
Q Serve(g_s), s	53.1	2.2	14.1	0.0	3.7	3.9
Cycle Q Clear(g_c), s	53.1	2.2	67.3	0.0	3.7	3.9
Prop In Lane		1.00	0.05		1.00	1.00
Lane Grp Cap(c), veh/h	2220	990	713	1010	102	90
V/C Ratio(X)	0.95	0.14	0.87	0.67	0.62	0.65
Avail Cap(c_a), veh/h	2220	990	713	1010	323	287
HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	7.4	2.1	13.6	0.0	38.3	38.4
Incr Delay (d2), s/veh	10.2	0.3	13.8	3.6	6.0	7.7
	0.0	0.0	0.0	0.0	0.0	0.0
Initial Q Delay(d3),s/veh						
%ile BackOfQ(50%),veh/ln	9.9	0.3	6.2	1.0	1.4	1.4
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	17.6	2.4	27.4	3.6	44.3	46.1
LnGrp LOS	В	Α	С	Α	D	D
Approach Vol, veh/h	2243			1301	122	
Approach Delay, s/veh	16.7			15.0	45.2	
Approach LOS	В			В	D	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		73.4		11.6		73.4
Change Period (Y+Rc), s		* 6.1		5.5		* 6.1
Max Green Setting (Gmax), s		* 54		19.5		* 54
Max Q Clear Time (g_c+l1), s		69.3		5.9		55.1
Green Ext Time (p_c), s		0.0		0.2		0.0
. ,		0.0		U.Z		0.0
Intersection Summary			4= 4			
HCM 6th Ctrl Delay			17.0			
HCM 6th LOS			В			
Notes						

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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Movement	EB	EB	EB	WB	WB	NB	NB
Directions Served	Т	Т	R	LT	Т	L	R
Maximum Queue (ft)	4985	5002	125	164	223	95	147
Average Queue (ft)	2916	2927	61	95	99	46	81
95th Queue (ft)	5718	5721	160	210	238	92	155
Link Distance (ft)	4950	4950		158	158	126	126
Upstream Blk Time (%)	24	25		5	4		23
Queuing Penalty (veh)	0	0		28	23		14
Storage Bay Dist (ft)			85				
Storage Blk Time (%)		45	0				
Queuing Penalty (veh)		58	0				

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		414	ሻ	7
Traffic Volume (veh/h)	2040	129	31	1166	58	54
Future Volume (veh/h)	2040	129	31	1166	58	54
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	U	1.00	1.00	U	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No	1.00	1.00	No	No	1.00
Adj Sat Flow, veh/h/ln	1477	1477	1477	1477	1477	1477
Adj Flow Rate, veh/h	2103	1477	34	1267	63	59
	0.97	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor						
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	2448	1092	49	1828	83	74
Arrive On Green	0.87	0.87	1.00	1.00	0.06	0.06
Sat Flow, veh/h	2879	1251	30	2162	1406	1251
Grp Volume(v), veh/h	2103	140	591	710	63	59
Grp Sat Flow(s),veh/h/ln	1403	1251	849	1277	1406	1251
Q Serve(g_s), s	64.9	2.7	83.4	0.0	7.5	7.9
Cycle Q Clear(g_c), s	64.9	2.7	148.3	0.0	7.5	7.9
Prop In Lane		1.00	0.06		1.00	1.00
Lane Grp Cap(c), veh/h	2448	1092	763	1114	83	74
V/C Ratio(X)	0.86	0.13	0.77	0.64	0.76	0.80
. ,	2448	1092	763	1114	161	144
Avail Cap(c_a), veh/h HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	5.5	1.6	12.9	0.0	78.7	78.9
Incr Delay (d2), s/veh	4.2	0.2	7.6	2.8	12.9	17.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	12.7	0.5	15.6	0.9	3.0	2.9
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	9.7	1.8	20.5	2.8	91.7	96.1
LnGrp LOS	Α	А	С	A	F	F
Approach Vol, veh/h	2243			1301	122	
Approach Delay, s/veh	9.2			10.8	93.8	
Approach LOS				В	95.0 F	
Approach LOS	Α			D	Г	
Timer - Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		154.4		15.6		154.4
Change Period (Y+Rc), s		* 6.1		5.5		* 6.1
Max Green Setting (Gmax), s		* 1.4E2		19.5		* 1.4E2
Max Q Clear Time (g_c+l1), s		150.3		9.9		66.9
Green Ext Time (p_c), s		0.0		0.2		37.2
		0.0		0.2		J1 .Z
Intersection Summary						
HCM 6th Ctrl Delay			12.6			
HCM 6th LOS			В			
Notes						

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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