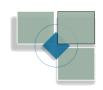


## **DeKalb County Department of Planning & Sustainability**

# 330 Ponce De Leon Avenue, Suite 500 Decatur, GA 30030

(404) 371-2155 / plandev@dekalbcountyga.gov



Michael Thurmond Chief Executive Officer

> Planning Commission Hearing Date: May 6, 2021 Board of Commissioners Hearing Date: May 27, 2021

### **STAFF ANALYSIS**

|  | <u></u>   |  |
|--|---|--|
| Case No.:  | Z-21-1244531  | Agenda #: D8   |
| Location/<br>Address:  | The east side of Northern Avenue, and the northern terminus of Creekview Drive, approximately 140 feet south of Indian Creek Way, at 671, 657, 635, 655, 649, 641, 631, and 623 Northern Avenue, Clarkston, Georgia.      | Commission District: 4 Super District: 6                       |
| Parcel ID:   | 18-045-08-001, 18-045-08-003, 18-045-08-004, 18-045-08-005, 18-045-08-006, 18-045-08-007, 18-045-08-008, 18-045-08-095  |  |
| Request:   | To rezone properties from R-75 (Residential-Me<br>Density Residential-2) District to RSM (Small Lot<br>construct townhomes and single-family detached   | Residential Mix) District to                                   |
| Property Owner:  | Fugees Land Holdings, LLC, Anjali Grandhige, &  | Hemanth Grandhige  |
| Applicant/Agent:   | Inline Communities LLC c/o Battle Law   |  |
| Acreage:   | 22  |  |
| Existing Land Use:   | Vacant land and Single-Family homes   |  |
| Surrounding Properties:  | A single-family detached subdivision (Cloudland family apartments (Navarro Apartments) to the and townhomes to the east (Ridgeland Creek Tohomes and multi-family apartments (Tuscany Visouthwest across Northern Avenue. | north; a stream buffer/floodplain ownhomes); and single-family |
| Adjacent Zoning:   | North: MR-2 South: R-75 East: MR-2 West: R  | 2-75 & MR-2  |
| Comprehensive Plan:  | SUB (Suburban) Consistent X   | Inconsistent   |
| Proposed Density: 5.54 u Proposed Units/Square Ft. comprising single-family at | •   | Square Feet: Vacant Land and                                   |

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**Existing Lot Coverage: NA** 

family detached homes. **Proposed Lot Coverage:** NA

#### Staff Recommendation: APPROVAL WITH CONDITIONS (REVISED 5/11/21)

#### **ZONING HISTORY**

In 2010, the Board of Commissioners approved to modify the conditions of the R-75 and MR-2 zoning (Case CZ-10-16332) and also approved a Special Land Use Permit (SLUP) (Case Number SLUP 10 16333) to allow a private, 120-student capacity middle school, soccer fields, a faculty residence, and a community garden on the subject properties.

#### **PROJECT ANALYSIS**

The subject property comprises 22 acres on the east side of Northern Avenue, approximately 140 feet south of Indian Creek Way, at 671, 657, 635, 655, 649, 641, 631, and 623 Northern Avenue, Clarkston. The site contains vacant land and single-family structures. The site slopes steeply downward from the north to the south. The site currently has an abundance of mature trees and vegetation. There is a stream and floodplain area on the eastern portion of the site that appears to be 120 feet wide.

Since the March 25<sup>th</sup> Board of Commissioners meeting, the applicant's latest site plan contains the following revisions:

- Decreased number of residential units from 147 to 122,
- Decreased density from 6.8 units per acre to 5.54 units per acre,
- Increased open space from 32% to 42%,
- The number of townhomes has decreased from 102 to 68 units, and
- The number of single-family detached units (including urban single-family detached with three feet between buildings and conventional single-family detached with ten feet between buildings) has increased from 45 to 54 units
- Increased buffer along the south property line from 20 feet to 40 feet (includes a 20 foot transitional buffer and 20 foot open space area).

The RSM district allows a base density of four (4) units per acre, with density up to eight (8) units per acre if certain community enhancements or provisions are provided. To achieve a density of 5.54 units per acre, the applicant is requesting a 50% density bonus based on the inclusion of enhanced open space (i.e. 4 DU/AC x 50% = 2 DU/AC; 4+2=6 DU/AC (maximum)). To get the density bonus, at least 20% open space must be enhanced open space such as dog parks, pocket parks, pool amenities, etc.). The site plan indicates that the applicant is providing more open space than required (i.e. 20% required (4.43 acres); 42% provided (9.33 acres)), and that 20% of the open space is enhanced open space consisting of a greenway trail along the northern and eastern portions of the site; pocket parks along the frontage of Northern Avenue, within courtyard areas of townhomes along east portion of site, and within the dog park; and a swimming pool/amenity area along the northwest portion of site plan. Additionally, the revised plans show a 10-foot wide multi-use trail proposed along the frontage of Northern Avenue which also extends off-site to Sandy Woods Lane to the south and Indian Creek Way to the north to provide area residents a safer walking experience along Northern Avenue.

Based on the submitted information, the revised plan's *conceptual* layout of single-family detached lots along the south and west perimeter of the site abutting single-family detached homes and providing townhomes along the north and east portions of the site abutting multi-family apartments and townhomes appears to be an appropriate transition of land uses. The proposed density of 5.54 units per acre is an appropriate transition between the density of the single-family detached homes to the south at two units per acre and the density of the multi-family apartments and townhomes to the north and east at 19 units per acre and 10 units per acre, respectively. The proposed three-story building heights of the proposed townhomes are consistent with the two and three story building heights of the multi-family apartments to the north and

the single-family attached townhomes to the east. The proposed two-story building heights of the single-family detached lots along the southern portion of the site, along with the 40-foot wide buffer (comprising a 20-foot planted buffer and a 20 foot wide open space area) should provide additional compatibility with the adjacent single-family detached subdivision to the south. Those proposed single-family detached lots also comply with the perimeter compatibility requirements of the *Zoning Ordinance* since those 64-foot-wide lots are 80% as wide as the abutting single-family lots to the south.

There are two access points for the project; a full access entrance/exit at the northern portion of the site which also provides for a 50 foot long left turn lane, and a limited right turn in/right turn out access at the southern portion of the site. Internal access within the project is via 26-foot-wide private streets. The DeKalb County Transportation Department has provided comments to address any potential traffic impacts (see attached), including a requirement that all access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents. The applicant will need to obtain a sewer capacity letter from the Department of Watershed Management to verify if sewer capacity is available.

Since the last Board of Commissioners meeting, the revised site plan and additional information have been provided to address Staff's previous concerns regarding compliance with RSM zoning standards and providing compatibility with surrounding properties. The minimum lot widths/lot areas were confirmed. The revised site plan contains greater specificity regarding enhanced open space. Moreover, the open space has been distributed throughout the proposed development more than the previous proposal. Regarding building height along the southern property line, Staff proposes a recommended condition of approval that the maximum building height be two-stories; consistent with the one and twostory single-family homes to the south on Sandy Woods Lane. While the revised plan does not indicate which trees are being saved, Staff is recommending as a condition of zoning approval that all existing trees that fall within designated open space areas and are not proposed for trails, buildings, structures, or parking lots be preserved to provide additional screening as well as assist with minimizing potential stormwater and flooding impacts. Additionally, Staff is recommending that all existing trees within proposed transitional buffer areas be preserved and supplemented with additional vegetation for provide an appropriate visual screen as approved by the County Arborist. At the May 6<sup>th</sup> Planning Commission meeting, the applicant provided revised site plans with reduced number of units from 124 to 122 and also increased the buffer along the southern property line from 20 feet to 40 feet (20-foot planted buffer and additional 20 foot open space area). Staff has also modified and added additional recommended zoning conditions submitted by the applicant at the May 6<sup>th</sup> Planning Commission meeting.

Surrounding uses include a single-family detached subdivision (Cloudland Subdivision) to the south; multi-family apartments (Navarro Apartments) to the north; a stream buffer, floodplain, and townhomes to the east (Ridgeland Creek Drive townhomes), and single-family homes and multi-family apartments (Tuscany Village Apartments) to the west and southwest across Northern Avenue.

<u>Supplemental Requirements:</u> There are no supplemental regulations in the zoning ordinance for single-family detached or attached homes.

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### **Compliance with District Standards:**

| STANDARD    | RSM REQUIREMENT                                 | EXISTING/PROPOSED   | COMPLIANCE   |
|-------------|---|---|--|
| MAX DENSITY | 4-8 units per acre                              | 5.54 units per acre with enhanced open space density bonus. | Yes. 50% Enhanced open space density bonus consists of pocket parks, a dog park and swimming pool amenity area, and a greenway trail. (50% Density Bonus calculated as follows: 4 units per acre base density multiplied by 50% density bonus = 2 units per acre. Four units per acre plus two units per acre = six units per acre, which accommodates the applicant's desired density of 5.59 units per acre) |
| LOT WIDTH   | 50 feet per single-family detache lot (minimum) | d 64 feet   | Yes  |
|             | 25 feet per single-family attached lot          | None required per Section 4.2.24.C                          | Yes  |
|             | 25 feet per urban single-family lo              | t 38 feet   | Yes  |

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| LOT AREA          | 5,000 s.f. (minimum for s.f. detached lot)                               | 6,400 s.f.  | Yes  |
|-------------------|--|---|--|
|                   | 1,350 s.f. (minimum for urban single-family detached)                    | 3,420 s.f.  | Yes  |
|                   | 1000 s.f. (minimum for s.f. attached lot)                                | Minimum building size containing townhome units is 2,700 s.f. | Yes  |
| MAX. LOT COVERAGE | 50% for single-family detached   | Information not provided                                      | Undetermined                                     |
|                   | 70% for single-family attached & urban single-family                     | Information not provided                                      | Undetermined                                     |
| FRONT SETBACK     | <u>Townhomes</u> :   |   |  |
|                   | 20 ft min local streets  | 10 ft   | No. Non-compliance will necessitate variances.   |
|                   | <u>Urban Single-Family</u> :<br>10 ft with alley access                  | 10 ft   | Undetermined. Non-                               |
|                   | 20 ft from local streets   | 15 ft   | compliance will<br>necessitate variances.<br>Yes |
|                   | Single-Family Conventional:  |   |  |
|                   | 20 ft from local streets   | 20 ft   | Yes  |
| REAR SETBACK      | Townhomes: 15 ft w no alley access 10 ft with alley access               | 15 ft<br>10 ft  | Yes<br>Yes                                       |
|                   | Urban Single-Family: 20 ft w no alley access 10 ft w alley access        | 20 ft<br>10 ft  | Yes<br>Yes                                       |
|                   | Single-Family Conventional: 20 ft w no alley access 10 ft w alley access | 20 ft<br>NA   | Yes<br>Yes                                       |

| SIDE SETBACK      | 3 ft from p/l with min. 10 feet<br>between buildings for single-<br>family detached conventional      | 3 feet from p/l with min. 10 feet between buildings   | Yes  |
|-------------------|---|---|--|
|                   | 0 ft from p/l with 3 feet between<br>buildings for urban single-family<br>detached lots—Interior Lots | Oft from p/I with 3 feet<br>between buildings for urban<br>single-family detached—<br>Interior Lots | Yes  |
|                   | 0 feet for townhomes  | 0 feet for townhomes  | Yes  |
|                   | Side corner lot on public street—same as front setback which is 20 ft                                 |   | Yes  |
| MAX. BLDG. HEIGHT | 35 feet for single-family detached conventional lots  | Information not provided  | Undetermined. Non-<br>compliance shall<br>necessitate variances. |
|                   | 45 feet or three stories, whichever is less for single-family attached and urban single-family        | 3 stories of 45 feet  | Yes  |
| MIN UNIT SIZE     | 1,200 s.f. for single-family detached or attached   | 1,200 s.f.  | Yes  |
|                   | 1,100 s.f. for urban single-<br>family detached   | 1,100 s.f.  | Yes  |
| MIN OPEN SPACE    | 20%   | 42%   | Yes  |

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| TRANSITIONAL BUFFER | 20 ft wide buffer along<br>northwest corner of plan<br>which abuts R-75 zoned<br>property.  | 20 ft buffer—a dog park and pocket park are proposed adjacent to buffer which provide additional screening.  | Yes  |
|---------------------|---|--|------|
|                     | None required along south p/l since single-family lots abut adjacent single-family subdivision  | None required. However, the proposed plan is providing a 20-ft wide transitional buffer along the southern property line which abuts an adjacent singlefamily subdivision. | Yes  |
| PARKING             | Min of 244 spaces  106 spaces for single-family detached (2 spaces per dwelling unit).  125 spaces for single-family attached (1.5 spaces per dwelling unit plus .25 spaces per unit for guest parking)  13 spaces for pool amenity (1 space per 10 homes)  Max of 468 spaces  212 spaces for single-family detached (4 spaces per dwelling unit)  231 Three (3) spaces per dwelling unit, plus one-quarter (0.25) space per dwelling unit to accommodate guest parking  25 spaces for pool amenity (1 space per 5 homes) |  | Yes. |

| SIDEWALKS AND<br>STREETSCAPING           | 6-ft. sidewalk along Northern<br>Avenue, 10-ft. landscape strip,<br>street trees 50 ft. on center  | 10 ft multi-use path,<br>landscape strips and<br>street trees not shown on<br>plan.  | Yes, for sidewalks. Undetermined for landscape strip and street trees. Non- compliance will necessitate variances. |
|--|--|--|--|
|  | 5-ft sidewalk and 5-ft landscape<br>strip along private drives with<br>street trees 50 ft on center.   | Sidewalk shown, but not<br>5-ft landscape strip  | Yes, to sidewalk, No to<br>Landscape strip. Non-<br>compliance will<br>necessitate variances.                      |
| STREETLIGHTS<br>AND PEDESTRIAN<br>LIGHTS | Streetlights shall be installed along public right of way within the landscape strip spaced at a maximum distance of 80 ft on center. Pedestrian lights shall be installed along public right of way at a maximum distance of 40 ft on center. | Information not provided   | Undetermined. Non-compliance will necessitate a variance.  |
| INTERNAL<br>SIDEWALKS                    | Pedestrian access shall be provided from all parking areas directly to a public sidewalk.  | Internal sidewalks<br>shown on plan connect<br>all units and open space<br>to public multi-use path<br>on Northern Avenue. | Yes  |

#### LAND USE AND ZONING ANALYSIS

Section 27-7.3.5 of the Zoning Ordinance, "Standards and factors governing review of proposed amendments to the official zoning map" states that the following standards and factors shall govern the review of all proposed amendments to the zoning maps.

#### A. Whether the zoning proposal is in conformity with the policy and intent of the comprehensive plan:

Based on the information and site plan, it appears that the proposed rezoning request is consistent with the following policies and strategies of the Suburban Character Area:

- 1. Protect stable neighborhoods from incompatible development (Suburban Policy #1).
- 2. Promote strong connectivity and continuity between existing and new development (Suburban Policy #10).

3. Create neighborhood focal points through the use of existing pocket parks and square for community activities (Suburban Policy #16).

The submitted plan's conceptual layout of single-family detached lots along the south and west perimeter of the site abutting single-family detached homes and providing townhomes along the north and east portions of the site abutting multi-family apartments and townhomes appears to be an appropriate transition of land uses. The proposed density of 5.54 units per acre is an appropriate transition between the density of the single-family detached homes to the south at two units per acre and the density of the multi-family apartments and townhomes to the north and east at 19 units per acre and 10 units per acre, respectively. Additionally, the plan provides more open space than is required by the *Zoning Ordinance* (42% provided, 20% required), and is also providing a 40-foot transitional buffer (20 foot planted buffer and 20 foot open space area) between the proposed single-family detached lots and the abutting single-family neighborhood to the south to further enhance compatibility with surrounding uses.

# B. Whether the zoning proposal will permit a use that is suitable in view of the use and development of adjacent and nearby properties:

Based on the information from the applicant, it appears that the zoning proposal with the conditions proposed by Staff will permit a use that is suitable. The submitted plan's *conceptual* layout of single-family detached lots along the south and west perimeter of the site abutting single-family detached homes and providing townhomes along the north and east portions of the site abutting multi-family apartments and townhomes appears to be an appropriate transition of land uses. The proposed density of 5.54 units per acre is an appropriate transition between the density of the single-family detached homes to the south at two units per acre and the density of the multi-family apartments and townhomes to the north and east at 19 units per acre and 10 units per acre, respectively. Additionally, the plan appears to be providing more open space than is required by the *Zoning Ordinance* (42% provided, 20% required), and is also providing a 40-foot transitional buffer between the proposed single-family detached lots and the abutting single-family neighborhood to the south to further enhance compatibility with surrounding uses.

# C. Whether the property to be affected by the zoning proposal has a reasonable economic use as currently zoned:

It appears that the property may have a reasonable economic use as currently zoned R-75 and MR-2 which allows single-family attached and detached residential development. However, bringing the properties under one zoning district could be more beneficial from marketing and development viewpoints. Split zoned projects may encounter more complexities due to differing zoning district development standards, permitted uses, and/or procedural requirements.

#### D. Whether the zoning proposal will adversely affect the existing use or usability of adjacent or nearby property:

There will be additional traffic along Northern Avenue from the proposed development. However, the traffic impact study concludes that "future traffic operations analysis results show that all the study intersections will continue to operate at satisfactory levels of service in both the AM and PM peak hours. The impact of site generated traffic on traffic operations on study intersections is insignificant. No improvements are recommended to lane geometry and traffic controls at any study intersection" (*Traffic Impact Study for Residential Development on Northern Avenue Dekalb County, Georgia,* pg. 20). The DeKalb County Transportation Department has reviewed the applicant's traffic impact study and has provided comments to address any potential traffic impacts

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(see attached), including a requirement that all access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents. Planning Department Staff has incorporated these comments into their recommended zoning conditions of approval, including a condition requiring written confirmation of approval from the Transportation Department prior to the issuance of any building permits. Therefore, it does not appear that the zoning proposal with conditions recommended by Planning Department Staff will adversely affect the existing usability of adjacent or nearby property.

E. Whether there are other existing or changing conditions affecting the use and development of the property, which give supporting grounds for either approval or disapproval of the zoning proposal:

The submitted plan's conceptual layout of single-family detached lots along the south and west perimeter of the site abutting single-family detached homes and providing townhomes along the north and east portions of the site abutting multi-family apartments and townhomes appears to be an appropriate transition of land uses.

See additional information in Criterion G regarding school impacts.

F. Whether the zoning proposal will adversely affect historic buildings, sites, districts, or archaeological resources:

Based on the submitted information, no historic buildings, sites, districts, or archaeological resources are located on the subject property or in the surrounding area.

G. Whether the zoning proposal will result in a use which will or could cause an excessive or burdensome use of existing streets, transportation facilities, utilities, or schools:

There has been no indication from reviewing departments and agencies that the proposal could cause excessive use of utilities. However, regarding school impacts, enrollment at Clarkston High School is already above capacity and "students from [the] new development may cause additional strain." The new replacement Indian Creek Elementary School will be opening in Fall 2021, which should provide additional capacity for elementary school students (see attached School comments). The DeKalb County Transportation Department has provided comments to address any potential traffic impacts (see attached), including a requirement that all access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents. The applicant will need to obtain a sewer capacity letter from the Department of Watershed Management to verify if sewer capacity is available.

H. Whether the zoning proposal adversely impacts the environment or surrounding natural resources:

The proposed development is not expected to have unusual impacts on the natural environment. If approved, many of the environmental impacts will be addressed during the land development permit review stage and beyond.

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#### Planning and Sustainability Department Recommendation: APPROVAL WITH CONDITIONS (REVISED 5/11/21)

Taken as a whole, the revised proposal contains a mixture of housing options that are designed to blend with existing development patterns, more than the minimum degree of open space along with opportunities for active and passive recreation for the community, and streetscape improvements. The applicant's traffic impact study did not produce findings indicating significant impact on the existing road network or the need for significant network improvements. However, the development's potential student yield may present issues for high school infrastructure in the surrounding community. Assuming that issue can be mitigated, and the development is constructed in compliance with applicable land development, building codes, and other county, state, and federal regulations, overall, the proposed project is consistent with the goals of Comprehensive Plan and Zoning Ordinance. The Applicant also submitted a list of additional zoning conditions at the May 6<sup>th</sup> Planning Commission meeting (see attached); and Staff has incorporated some of those conditions into the recommended conditions of approval (deletions from original recommended conditions are in strikethrough, additions are in bold). The Planning and Sustainability Department recommends that the rezoning application be "Approved" with the following conditions:

- 1. Up to a maximum of 124-122 residential units comprising single-family detached homes and single-family attached townhomes. Up to a maximum of 71 68 single-family attached townhomes and a maximum of 38 54 urban single-family detached homes (including conventional and urban single-family detached dwellings).
- 2. General compliance with the locations of single-family detached traditional homes, urban single-family detached homes, and single-family attached townhomes shown on the site plan entitled "Northern Avenue at Indian Creek Way" and dated 4/15/21-presented at the May 6, 2021 Planning Commission meeting. Only single-family detached traditional lots (minimum 10 feet between buildings) shall be provided along the southern property line and shall be at least 6,400 square feet in lot area and at least 64 feet wide.
- 3. A minimum of 39% open space shall be provided. Enhanced open space shall comprise at least 20% of the total site acreage. Location and size of the proposed open space and pocket parks shall be generally consistent with the conceptual site plan entitled, "Northern Avenue at Indian Creek Way" and dated 4/15/21 presented at the May 6, 2021 Planning Commission meeting.— Productive urban landscaping shall be incorporated into the design of and implementation of the pocket parks, transitional buffers and trails when proper light and soil conditions permit. This productive urban landscaping shall consist of fruit bearing trees and shrubs and other plants that support pollinating insects. The developer shall create a Homeowners' Association which shall be responsible for maintaining the open space and landscaping.
- 4. In exchange for enhanced open space (as referenced in Condition 3), the development shall be entitled to a maximum density of 5.5 dwelling units per acre. A schedule for the installation of enhanced open space elements must be submitted by the developer and approved by the Director of Planning and Sustainability (or his designee) prior to final plat approval.
- 5. Existing trees **located** within designated open space areas and are not proposed for trails, buildings, **structures**, or parking lots shall be preserved.
- 6. **Healthy** existing trees within the 20-foot transitional planted buffer along the south property line shall be preserved and supplemented with new trees to form an effective visual screen, as approved by the County Arborist prior to the issuance of any certificates of occupancy. Existing trees within the 20-foot transitional

- buffer along the northwest corner of the property shall be preserved and supplemented with new trees to form an effective visual screen as approved by the County Arborist prior to issuance of any certificates of occupancy.
- 7. Maximum building height of two stories for single-family (conventional) detached lots along the south property line, dwellings facing Northern Avenue on the west side of the property, and for units 116-124 on the northwest corner adjacent to tax parcel 18 045 08 002. Maximum building height of and three stories for single-family attached townhomes and urban single-family detached homes not adjacent to Northern Avenue or Dial Heights.
- 8. The development shall have no vehicular access to Creekview Drive.
- 9. Written confirmation of approval from the DeKalb County Transportation Department is required prior to the issuance of any building permits. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 35 feet from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents.
- 10. The conventional single-family detached lots shall include distinctly different front façade designs within each phase of the development. "Distinctly different" shall mean that each front façade must differ from adjacent buildings' front façades in at least four (4) of the following six (6) ways: a. The use of different primary exterior materials; b. Variation in the width or height of the front façade by four (4) feet or more; c. Variation of the type, placement or size of windows and doors on the front façades; d. Variations in rooflines, including the use of dormers and changes in the orientation of rooflines; e. Variation in the location and proportion of front porches; and f. Variation in the location or proportion of garages and garage doors. No single-family detached residence shall be of the same front façade design as any other single-family detached residence along the same block face within eight (8) lots of the subject residence. Mirror images of the same configuration are not permitted on the same block face. No single front façade design may be used for more than twenty-five (25) percent of the total units of any single phase of a conventional single-family detached residence subdivision.
- 11. Any single-family detached residence with a front façade width of forty (40) feet or more shall incorporate wall offsets in the form of projections or recesses in the front façade plane. Wall offsets shall have a minimum depth or projection of two (2) feet so that no single wall plane exceeds twenty-five (25) feet in width.
- 12. Street-facing garage façades of single-family detached conventional units shall not comprise more than forty-five (45) percent of the total width of the conventional single family detached residence's front façade. Street-facing garages shall be at least two (2) feet behind the primary front façade plane of a single-family detached residence.
- 13. During construction, the Developer shall post a contact phone number that nearby residents can call to discuss development and construction issues. The developer shall use Best Management Practices (BMPs) for run-off and sedimentation control in compliance with Section 14. 38 (Soil Erosion and Sediment Control) subject to approval of the Land Development Division of the Planning and Sustainability Department.

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- 14. The approval of this rezoning application by the Board of Commissioners has no bearing on the requirements for other regulatory approvals under the authority of the Historic Preservation Commission, the Zoning Board of Appeals, or other entity whose decision should be based on the merits of the application under review by such entity.
- 15. Subject to Department of Transportation and County Arborist approval, the developer shall provide a 10-foot wide multi-use path (to include bike lanes) along the frontage of the Northern Avenue right-of-way to extend from Sandy Woods Lane to Indian Creek Way.
- 16. Blasting shall be permitted only with the written approval of the County under the requirements of Section 14-324.C of the DeKalb County Code of Ordinances. The developer shall notify residents of adjacent properties and properties within the Dial Heights subdivision, by way of mailbox flyers, 24 hours in advance of any blasting.
- 17. The applicant, developer, or HOA shall coordinate with the DeKalb County School District and provide an annual development progress report through the year in which the final building permit for the last residential unit is issued.
- 18. A tree survey shall be performed in the proposed open spaces areas by a qualified expert and all proposed tree removal shall be confirmed by the County Arborist. Additionally, the applicant shall procure the services of a qualified engineer to assess all wetlands and floodplains within or adjacent to the subject properties and document those findings. Findings shall be confirmed by County Wetland/Floodplain experts and presented to the adjacent neighborhoods prior to any approval of site plans or land development permits.
- 19. Subject to the approval of the DeKalb County Transportation Department, traffic calming measures shall be installed on Northern Avenue prior to the issuance of any building permits, including, but not restricted to:
  - a. Inclusion of a landscape strip or other traffic-calming streetscape in the right-of-way in front of the proposed development; and
  - b. Installation of signage regarding speed limits and dangerous curves and hills.
- 20. There shall be a minimum of two retaining ponds (i.e. stormwater detention facilities) located on the property.
- 21. Applicant agrees to perform maintenance on fallen trees and debris within property boundaries within the Indian Creek stream banks, and to the extent given permission by landowners, Applicant shall work cooperatively with the landowners to remove fallen trees and debris which are creating the blockage of the flow of the stream up to Dial Drive. These activities shall be completed prior to the issuance of any land disturbance permits.
- 22. Development-related trucks, vehicles, and equipment shall not park on Northern Avenue.
- 23. Appropriate measures must be taken to mitigate dust and debris from construction and truck traffic.
- 24. Property must be secured by temporary fencing when no active work is underway.
- 25. All housing units built in the development shall be "solar ready" and have electric service panels with sufficient capacity to accommodate electric vehicle charging within the garages and solar roof panels.

#### **Attachments:**

- 1. Public Works Department Comments
  - a. Land Development Division
  - b. Traffic Engineering Division
- 2. Watershed Management Department Comments
- 3. Board of Health Comments
- 4. Board of Education Comments
- 5. Application
- 6. Site Plan
- 7. Zoning Map
- 8. Aerial Photograph
- 9. Photographs

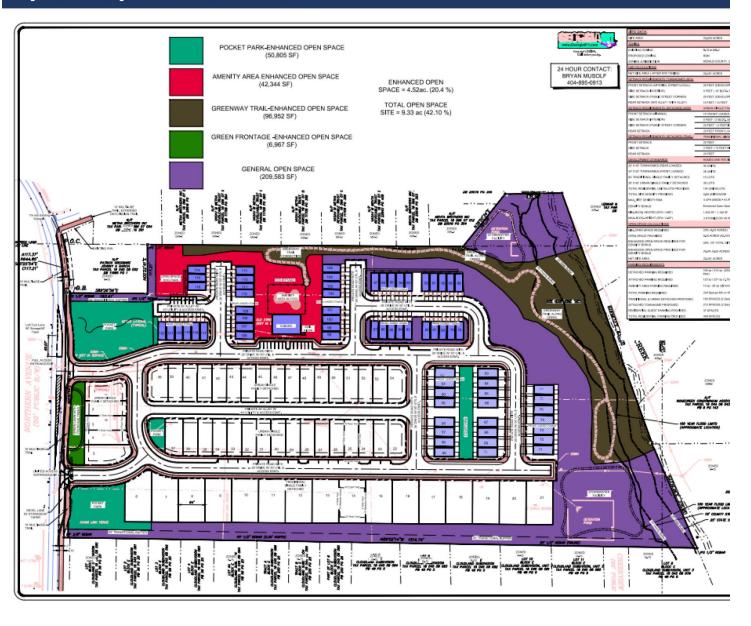
Current Site Plan - 54 SFD + 68 THs (5.5 homes/acre)
-2 Homes since Community Council. -43 Homes from original plan.



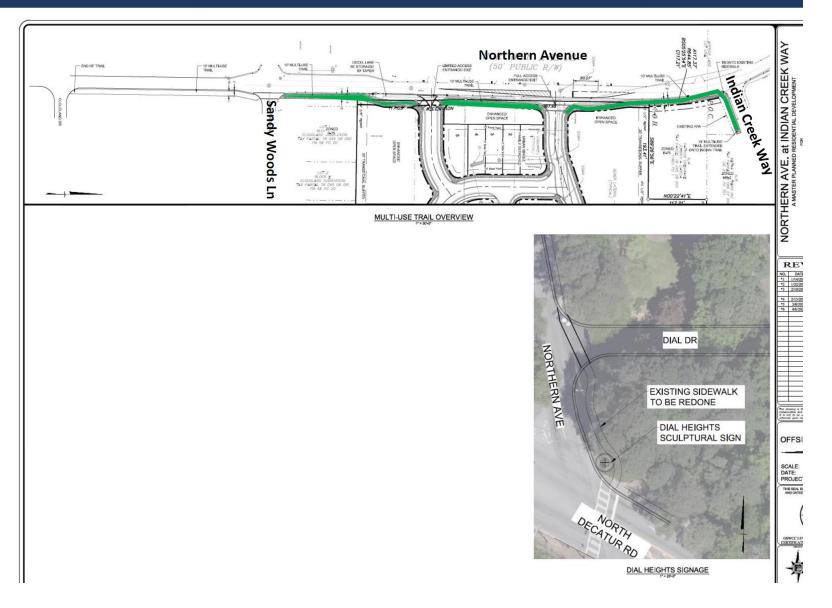
Lots adjacent to Sandy Woods Ln. will have a 20' Rear Yard Setback + 20' Open Space Area + 20' Transitional Buffer (owned by HOA).



# Open Space Exhibit



Trail will be extended south to Sandy Woods Lan north to wrap around Indian Creek Way.



# PROPOSED ADDITIONAL ZONING CONDITIONS PROPOSED BY APPLICANT AT May 6 Planning Commission Meeting

# **Proposed Additional Conditions**

- Maximum building height of 35 feet for single-family (conventional) detached lots along the south property line and dwellings facing Northern Avenue on the west side of property and for units 116-124 on the northwest corner adjacent to tax parcel 18 045 08 002 that is a single-family home. Maximum building height of three stories for single-family attached townhomes and urban single family detached homes not adjacent to Northern Avenue or Dial Heights.
- Property should be walked and documented by the County Arborist and Wetland/Floodplain expert. Documented findings should be presented to the adjacent neighborhoods prior to any approval of site plans.
- Subject to Department of Transportation and County Arborist approval, the developer shall provide a 10-foot wide multi-use path (to include bike lanes) along the frontage of the Northern Avenue rightof-way to extend from Sandy Woods Lane to Indian Creek Way.
- 4. Subject to the approval of the DeKalb County Transportation Department, traffic calming measures shall be installed on Northern, including, but not restricted to:
  - Inclusion of a landscape strip of land, or other traffic-calming streetscape, in the right-of- way in front of the proposed development; and
  - Installation of signage regarding speed limits and dangerous curves and hills.
- 5. The development shall have no vehicular access to Creekview Drive.
- 6. There shall be a minimum of two retaining ponds located on the property.
- 7. Applicant agrees to perform maintenance on fallen trees and debris within property boundaries within the Indian Creek stream banks, and to the extent given permission by landowners, Applicant shall work cooperatively with the landowners to remove fallen trees and debris which are creating the blockage of the flow of the stream up to Dial Drive.
- 8. Development-related trucks, vehicles and equipment shall not be parked on Northern Avenue.
- 9. Appropriate measures must be taken to mitigate dust and debris from construction and truck traffic.
- Appropriate measures must be employed to prevent runoff and impact on adjacent properties and lands during construction.
- 11. Property must be secured by temporary fencing when no active work is underway.



# DEKALB COUNTY GOVERNMENT PLANNING DEPARTMENT DISTRIBUTION FORM

The following areas below may warrant comments from the Development Division. Please respond accordingly as the issues relate to the proposed request and the site plan enclosed as it relates to Chapter 14. You may address applicable disciplines.

#### **DEVELOPMENT ANALYSIS:**

#### Transportation/Access/Row

Consult the Georgia DOT as well as the DeKalb County Transportation Department prior to land development permit. Verify widths from the centerline of the roadways to the property line for possible right-of-way dedication. Improvements within the right-of-way may be required as a condition for land development application review approval. Safe vehicular circulation is required. Paved off-street parking is required.

#### • Storm Water Management

Compliance with the Georgia Stormwater Management Manual, DeKalb County Code of Ordinances 14-40 for Stormwater Management and 14-42 for Storm Water Quality Control, to include Runoff Reduction Volume where applicable is required as a condition of land development permit approval. Use Volume Three of the G.S.M.M. for best maintenance practices. Use the NOAA Atlas 14 Point Precipitation Data set specific to the site. Recommend Low Impact Development features/ Green Infrastructure be included in the proposed site design to protect as much as practicable the statewaters and special flood hazard areas.

#### • Flood Hazard Area/Wetlands

The presence of FEMA Flood Hazard Area was indicated in the County G.I.S. mapping records for the site; and should be noted in the plans at the time of any land development permit application. Encroachment of flood hazard areas require compliance with Article IV of Chapter 14 and FEMA floodplain regulations.

#### • Landscaping/Tree Preservation

Landscaping and tree preservation plans for any building, or parking lot must comply with DeKalb County Code of Ordinances 14-39 as well as Chapter 27 Article 5 and are subject to approval from the County Arborist.

#### • Tributary Buffer

State water buffer was reflected in the G.I.S. records for the site. Typical state waters buffer have a 75' undisturbed stream buffer and land development within the undisturbed creek buffer is prohibited without a variance per DeKalb County Code of Ordinances 14-44.1.

#### Fire Safety

<u>Plans for land development permit must comply with Chapter 12 DeKalb County Code for fire protection and prevention.</u>



## DEKALB COUNTY GOVERNMENT PLANNING DEPARTMENT DISTRIBUTION FORM

**NOTE:** PLEASE RETURN ALL COMMENTS VIA EMAIL OR FAX TO EXPEDITE THE PROCESS TO **MICHELLE M ALEXANDER** <u>mmalexander@dekalbcountyga.gov</u> OR **JOHN REID** <u>IREID@DEKALBCOUNTYGA.GOV</u>

#### COMMENTS FORM: PUBLIC WORKS WATER AND SEWER

| Case No.:Z-21-1244531  |
|--|
| Parcel I.D. #: <u>18-045-08-001, 18-045-08-003, 18-045-08-004, 18-045-08-005, 18-045-08-006, 18-045-08-007, 18-045-08-008, 18-045-08-095</u>   |
| Address: 671, 657, 635, 655, 649, 641, 631, and 623 Northern Avenue  |
| Clarkston, Georgia   |
| WATER:   |
| Size of existing water main: _6" AC Water Main (adequate/inadequate)   |
| Distance from property to nearest main: Adjacent to Property   |
| Size of line required, if inadequate:N/A   |
| SEWER:   |
| Outfall Servicing Project: Indian Creek Basin  |
| Is sewer adjacent to property: Yes (X) No ( ) If no, distance to nearest line:   |
| Water Treatment Facility: Snapfinger WTF () adequate () inadequate   |
| Sewage Capacity; * (MGPD) Current Flow: 21.77 (MGPD)   |
| COMMENTS:  |
| * Please note that the sewer capacity has not been reviewed or approved for this project. A Sewer Capacity Request (SCR) must be completed and submitted for review. This can be a lengthy process and should be addressed early in the process. |
| Lapacity Restricted bred   |
| 5.1  |
|  |
|  |
|  |
|  |
| Signature:   |



# DEKALB COUNTY GOVERNMENT PLANNING DEPARTMENT DISTRIBUTION FORM



NOTE: PLEASE RETURN ALL COMMENTS VIA EMAIL OR FAX TO EXPEDITE THE PROCESS TO MICHELLE ALEXANDER MMALEXANDER@DEKALBCOUNTYGA.GOV OR JOHN REID JREID@DEKALBCOUNTYGA.GOV

# COMMENTS FORM: PUBLIC WORKS TRAFFIC ENGINEERING

| Case No.: Z-2 -                             | 124453   Parcel I.D. #: /   | 8-045-0,8-00/   |  |
|---|---|---|--|
| Address: 67                                 |   |   |  |
| NORTHE                                      | ESTON, GABLUZI  |   |  |
| Clark                                       | region d. 3001  |   |  |
|   | 104, (A 300)  |   |  |
|   | Adjacent l  | Roadway (s):  |  |
|   |   |   |  |
|   | (classification)  | (classification)  |  |
|   | <b>(</b>  | (,  |  |
|   | Capacity (TPD)  | Capacity (TPD)  |  |
|   | Latest Count (TPD)Hourly Capacity (VPH)   | Latest Count (TPD) Hourly Capacity (VPH)  |  |
|   | Peak Hour, Volume (VPH)   | Peak Hour, Volume (VPH)   | <del></del>                            |
|   | Existing number of traffic lanes  | Existing number of traffic lanes  |  |
|   | Existing right of way width Proposed number of traffic lanes  | Existing right of way width   |  |
|   | Proposed number of traffic lanes<br>Proposed right of way width   | Proposed number of traffic lanes Proposed right of way width                        |  |
|   |   |   |  |
| Please provide additi                       | onal information relating to the following st   | atement.  |  |
| generate an average of factor. Based on the | conducted by the Institute of Traffic Engine<br>of fifteen (15) vehicle trip end (VTE) per 1, (<br>above formula, thesquare foot plac<br>peak hour vehicle trip ends.                   | 000 square feet of floor area, with an eight (                                      | 8%) percent peak hou                   |
| peak hour factor. Ba<br>a maximum ofu       | nce, on the other hand, would generate ten (<br>ised on the above referenced formula, the _<br>nits per acres, and the given fact that the pr<br>peak hour vehicle trip end would be ge | (Single Family Residential) District des<br>oject site is approximatelyacres in lar | signation which allow<br>(d area,daily |
| COMMENTS:                                   |   |   |  |
| REVIEWE                                     | A NoThing four  | d That would  | (ISRLYDT                               |
| TRATIC                                      | Clow.   |   |  |
|   |   |   | 17                                     |
| -   |   |   |  |
|   |   |   |  |
|   |   |   |  |
|   |   |   |  |

THE DAVID WILLOW

#### N1. No Comment

N2 & N3. Coordinate and provide the required right of way for the GDOT Managed Lanes I-285 East Project prior to permitting. GDOT PM: Tim Matthews at <a href="mailto:TMatthews@dot.ga.gov">TMatthews@dot.ga.gov</a>. Rockbridge Road is classified as a minor arterial. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 40 from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required: 10- foot landscape strip, 6-foot sidewalk, bike lanes, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for 35 mph and presented (signed and sealed by a professional engineer) with the land development permit documents.

N4 & N5. Covington Hwy is a state route. Review and approval by GDOT District 7 (Justin Hatch at <a href="Juhatch@dot.ga.gov">Juhatch@dot.ga.gov</a>) required prior to issuance land development permit. Covington Hwy is classified as a major arterial. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 50 from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required at a minimum (GDOT may have additional requirements): 10- foot landscape strip, 6-foot sidewalk, bike lanes or multiuse path, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents.

N6 & N7. Pine Mountain Road is classified as a local residential. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 27.5 feet from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required: 6- foot landscape strip, 5-foot sidewalk, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents.

Please note that we received complaints about truck traffic on this street and it is posted no trucks. Consideration should be given to how to handle truck access and traffic. Limit all truck access to SR 124 Turner Hill Road. No truck access on Pine Mountain Rd.

#### N8. No Comment

N9. This development requires a traffic study (337 units) be presented to identify required improvements prior to zoning. I recommend deferral until a traffic study is submitted so that we can incorporate the result of the traffic study into the zoning conditions. Traffic study should address requirements for left turning lanes and right turn lane on North Druid Hills at the Mont Moriah Road and the need for a potential traffic signal. Please confirm the existing right of way on Mount Moriah Road. The county records show a 60 foot right of way and it appears that the development is encroaching on the right of way. The study should also address the lanes needed to accommodate the traffic exiting Mount Moriah Rd at the intersection. Direct pedestrian access is to be provided from the public sidewalks to the proposed development. North Druid Hills Road is classified as a major arterial. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 50 from centerline or such that all public infrastructure

(sidewalks/streetlights) are within right of way, whichever greater. Required at a minimum: 10- foot landscape strip, 6-foot sidewalk, bike lanes or multiuse path, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents. Mount Moriah Road is classified as a local road. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 27.5 feet from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required at a minimum: 6- foot landscape strip, 5-foot sidewalk, streetlights. Mount Moriah Road must be brought up to minimum county standards to include at least 22 feet of pavement along entire property frontage. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents.

N10 & N11. Pine Mountain Road is classified as a local. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 27.5 feet from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required: 6- foot landscape strip, 5-foot sidewalk, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents.

N12. Requesting a traffic study be completed prior to zoning to determine the impacts of the development on the intersection of Rockbridge Road at Mountain Park Trail and the proposed driveway on Rockbridge Road. Only one access point of Mountain Park Trail. The access point on Mountain Park Trail must be shifted to the rear property line away from Rockbridge Road. Please note the minimum driveway/street separation required in Section 14-200 (6). Remove acceleration lane from Rockbridge Road frontage. Provide direct pedestrian access from public right of way to the proposed destinations. Rockbridge Road is classified as a minor arterial. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 40 from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required: 10- foot landscape strip, 6-foot sidewalk, bike lanes, streetlights. Mountain Park Trail is classified as a local. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 27.5 feet from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required: 6- foot landscape strip, 5-foot sidewalk, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for 35 mph and presented (signed and sealed by a professional engineer) with the land development permit documents.

N13. Northern Ave is classified as a collector road. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 35 from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required: 10- foot landscape strip, 6-foot sidewalk, bike lanes or multiuse path, streetlights. All access points must meet minimum intersection and stopping sight

distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents.

#### N14. No comment.

N15, N16 and N17. Panola Road is classified as a major arterial. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 50 feet from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required at a minimum: 10- foot landscape strip, 6-foot sidewalk, bike lanes or multiuse path, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents. Young Road is classified as a collector road. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 35 feet from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required: 10- foot landscape strip, 6-foot sidewalk, bike lanes, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for 35 mph and presented (signed and sealed by a professional engineer) with the land development permit documents. Please note the minimum driveway/street separation required in Section 14-200 (6). Applies to driveways on the opposite side of the road also. Access point on Young Road needs to be relocated away from the traffic signal. The developer is required to upgrade the pedestrian features of the traffic signal at Panola Road at Young Road, as needed, as identified by the Transportation Division of Public Works. A pedestrian connection must be provided from the public sidewalk to the building entrances.

N18. Clairmont Road is a state route. Review and approval by GDOT District 7 required prior to issuance land development permit. Clairmont Road is classified as a major arterial. Only one access point allowed on Clairmont Road located away from the intersection with N Williamsburg Dr. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 50 from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required at a minimum (GDOT may have additional requirements): 10- foot landscape strip, 6-foot sidewalk, bike lanes or multiuse path, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents. N. Williamsburg Drive is classified as a local road. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 27.5 feet from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required at a minimum: 6- foot landscape strip, 5-foot sidewalk, streetlights. Only one access point allowed on N Williamsburg Road located away from the intersection on Clairmont Road. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents.

#### N19. No Comment

N20. Clifton Springs Road is classified as a minor arterial. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 40 feet from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required at a minimum: 10- foot landscape strip, 6-foot sidewalk, bike lanes or multiuse path, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents. If interior roads are to public. They will need to meet the requirements for a local road. Please note the infrastructure requirements in Chapter 5 of the Zoning Code and Chapter 14-190 of the Land Development Code. A right of way dedication of 27.5 feet from centerline or such that all public infrastructure (sidewalks/streetlights) are within right of way, whichever greater. Required: 6- foot landscape strip, 5-foot sidewalk, streetlights. All access points must meet minimum intersection and stopping sight distance requirements per AASHTO Greenbook for the posted speed limit and presented (signed and sealed by a professional engineer) with the land development permit documents.

#### DEKALB COUNTY

## Board of Health

#### 02/15/2021

To: Mr. John Reid, Senior Planner

From: Ryan Cira, Environmental Health Manager Cc: Alan Gaines, Technical Services Manager

Re: Rezone Application Review

#### General Comments:

DeKalb County Health Regulations prohibit use of on-site sewage disposal systems for:

- multiple dwellings
- · food service establishments
- · hotels and motels
- commercial laundries
- funeral homes
- schools
- nursing care facilities
- personal care homes with more than six (6) clients
- child or adult day care facilities with more than six (6) clients
- residential facilities containing food service establishments

If proposal will use on-site sewage disposal, please contact the Land Use Section (404) 508-7900.

Any proposal, which will alter wastewater flow to an on-site sewage disposal system, must be reviewed by this office prior to construction.

This office must approve any proposed food service operation or swimming pool prior to starting construction.

Public health recommends the inclusion of sidewalks to continue a preexisting sidewalk network or begin a new sidewalk network. Sidewalks can provide safe and convenient pedestrian access to a community-oriented facility and access to adjacent facilities and neighborhoods.

For a public transportation route, there shall be a 5ft. sidewalk with a buffer between the sidewalk and the road. There shall be enough space next to sidewalk for bus shelter's concrete pad installation. Recommendation: Provide trash can with liner at each bus stop with bench and monitor for proper removal of waste.

Since DeKalb County is classified as a Zone 1 radon county, this office recommends the use of radon resistant construction.

## Board of Health

| N.1 | TA-21-1244539      | 2021-2108 |
|-----|--------------------|-----------|
|     | County-Wide (All I | District) |
|     | 36                 |           |

### N.2 LP-21-12439332021-2109/18-011-06-001,18-011-06-004,18-011-06-005,18-011-06-006,18-

011-06-007

District 04 Super District 06

3581 Rockbridge Road, Stone Mountain, GA 30083

3605 Rockbridge Road, Stone Mountain, GA 30083

3611 Rockbridge Road, Stone Mountain, GA 30083

3599 Rockbridge Road, Stone Mountain, GA 30083

3593 Rockbridge Road, Stone Mountain, GA 30083

3581 Rockbridge Road, Stone Mountain, GA 30083

- Please review general comments
- Septic system installed on September 23, 1960 for property 3605
- Septic system installed on September 23, 1960 for property 3611

Total acres 4.8

## $N.3 \qquad \textbf{Z-21-1243934} \quad \textbf{2021-2110} \, / \, 18\text{-}011\text{-}06\text{-}001, \, 18\text{-}011\text{-}06\text{-}004, \, 18\text{-}011\text{-}06\text{-}005, \, 18\text{-}011\text{-}06\text{-}006, \, 18\text{-}011\text{-}06\text{-}006, \, 18\text{-}011\text{-}06\text{-}006, \, 18\text{-}011\text{-}06\text{-}008, \, 18\text{-}011\text{-}08\text{-}011\text{-$

011-06-007

District 04 Super District 06

3581 Rockbridge Road, Stone Mountain, GA 30083

3605 Rockbridge Road, Stone Mountain, GA 30083

3611 Rockbridge Road, Stone Mountain, GA 30083

3599 Rockbridge Road, Stone Mountain, GA 30083

3593 Rockbridge Road, Stone Mountain, GA 30083

3581 Rockbridge Road, Stone Mountain, GA 30083

- Please review general comments
- Septic system installed on September 23, 1960 for property 3605
- Septic system installed on September 23, 1960 for property 3611

Total acres 4.8

#### N.4 LP-21-1244555 2021-2111 / 15-162-04-008 Dis

District 05 Super District 07

5011 Covington Highway, Decatur, GA 30035

- Please review general comments

Total acres 0.61

#### N.5 **Z-21-1244408 202102112 / 15-162-04-008**

District 05 Super District 07

5011 Covington Highway, Decatur, GA 30045

- Please review general comments

Total acres 0.61

#### N.6 LP-21-1244580 2021-2113 / 16-168-01-008

District 05 Super District 07

2346 Pine Mountain Street, Lithonia, GA 30058

- Please review general comments

Total acres 1.2

#### **DeKalb County Board of Health**

445 Winn Way – Box 987 Decatur, GA 30031

404.294.3700 • www.dekalbhealth.net

# Board of Health

N.7 **Z-21-1244581 2021-2114 / 16-168-01-008** District 05 Super District 07

2346 Pine Mountain Street, Lithonia, GA 30058

- Please review general comments

Total acres 1.2

N.8 **TA-21-1244599 2021-2115** 

District 02 Super District 06

North Druid Hills Briarcliff Node, Atlanta, GA 30329

- Please review general comments

Total acres (not stated)

N.9 **Z-21-1244535 2021-2116 / 18-152-01-005, 18-152-01-006, 18-152-01-054** 

District 02 Super District 06

2490 North Druid Hills Road, Atlanta, GA 30329

- Please review general comments
- Several surrounding properties with septic system installed Total acres 5.6
- N.10 **LP-21-1244541 2021-2117 / 16-167-08-010** District 05 Super District 07 2328 Pine Mountain Street, Lithonia, GA 30058
  - Please review general comments
  - Several surrounding properties with septic system installed Total acres 0.79
- N.11 **Z-21-1244542 2021-2118 / 16-167-08-010** District 05 Super District 07 2328 Pine Mountain Street, Lithonia, GA 30058
  - Please review general comments
  - Several surrounding properties with septic system installed

Total acres 0.79

# **DeKalb County School District Development Review Comments**

 Submitted to:
 DeKalb County
 Case #:
 Z-21-1244531

 Parcel #:
 18-045-08

001/003/004/005/006/007/008/095

**Analysis Date:** 

2/8/2021

Name of Development: Northern Ave. at Indian Creek Way

**Location:** 623,631,641,649,655,635,657,671 Northern Avenue

**Description:** Proposed attached and detached homes off Northern Avenue

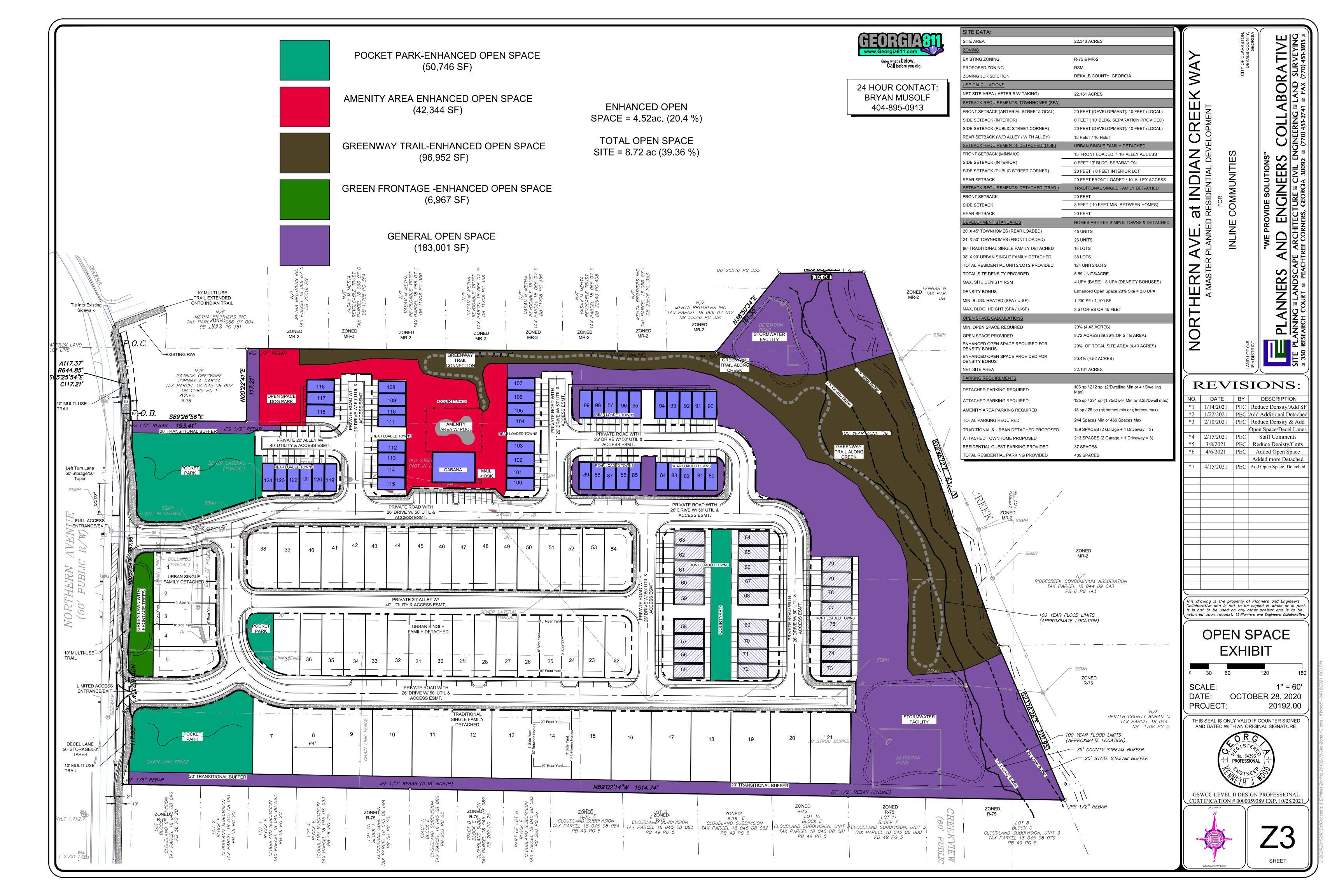
Impact of Development: When fully constructed, this development would be expected to generate 43 students: 6 at Indian

Creek Elementary School, 8 at Freedom Middle School, 9 at Clarkston High School, 17 at other DCSD schools, and 3 at private school. Enrollment at Clarkston HS is already above capacity and students from new development may cause additional strain. The new Replacement Indian Creek

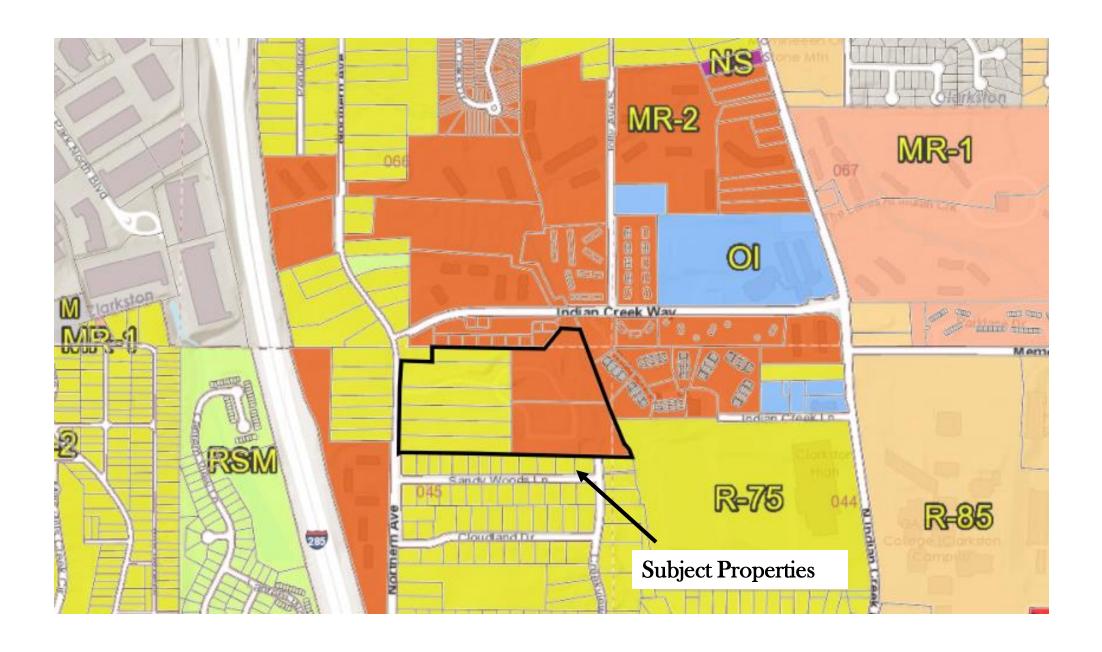
ES will be opening Fall 2021, providing additional capacity for elementary students.

| Current Condition of Schools  | Indian Creek<br>Elementary<br>School | Freedom<br>Middle<br>School | Clarkston<br>High School | Other DCSD Schools | Private<br>Schools | Total |
|-------------------------------|--------------------------------------|-----------------------------|--------------------------|--------------------|--------------------|-------|
| Capacity                      | 1,200                                | 1,251                       | 1,190                    |                    |                    |       |
| Portables                     | 0                                    | 0                           | 16                       |                    |                    |       |
| Enrollment (Fcast. Oct. 2021) | 849                                  | 1,116                       | 1,513                    |                    |                    |       |
| Seats Available               | 351                                  | 135                         | -323                     |                    |                    |       |
| Utilization (%)               | 70.8%                                | 89.2%                       | 127.1%                   |                    |                    |       |
| New students from development | 6                                    | 8                           | 9                        | 17                 | 3                  | 43    |
| New Enrollment                | 855                                  | 1,124                       | 1,522                    | ]                  |                    |       |
| New Seats Available           | 345                                  | 127                         | -332                     |                    |                    |       |
| New Utilization               | 71.3%                                | 89.8%                       | 127.9%                   |                    |                    |       |

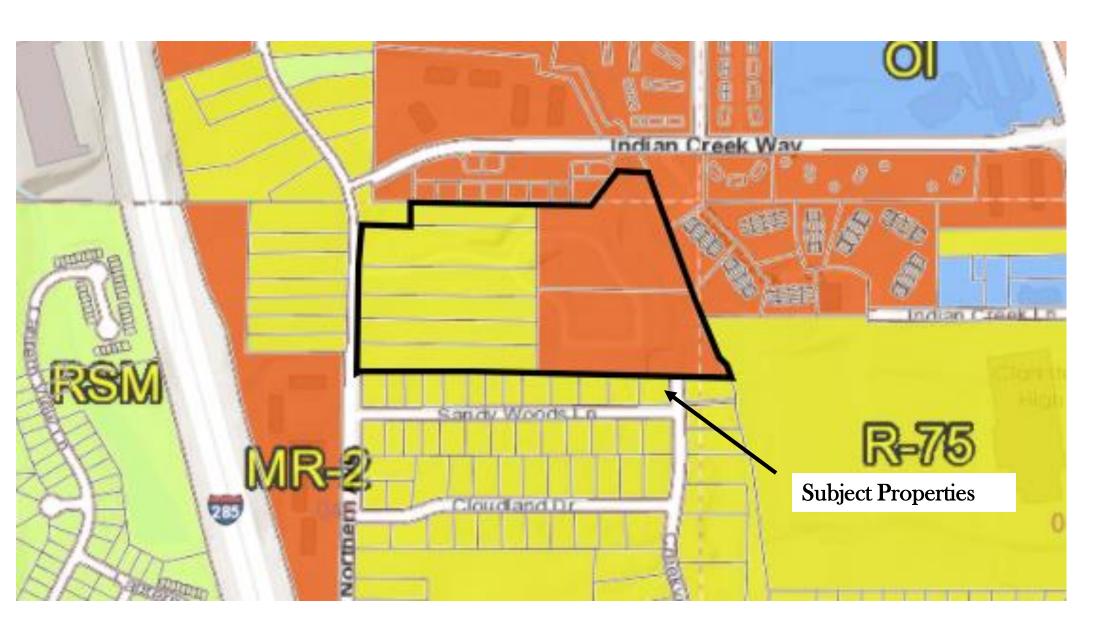
|                       |           | Attend      | Attend other |         |        |
|-----------------------|-----------|-------------|--------------|---------|--------|
|                       |           | Home        | DCSD         | Private |        |
| Yield Rates           |           | School      | School       | School  | Total  |
| Elementary            |           | 0.0388      | 0.0782       | 0.0176  | 0.0449 |
| Middle                |           | 0.0562      | 0.0147       | 0.0000  | 0.0236 |
| High                  |           | 0.0567      | 0.0180       | 0.0000  | 0.0249 |
| Total                 |           | 0.0506      | 0.0370       | 0.0059  | 0.0311 |
| Student Calculations  |           |             |              |         |        |
| Proposed Units        | 1         | 51          |              |         |        |
| Unit Type             | Α         | \PT         |              |         |        |
| Cluster               | Clarkston | High School |              |         |        |
|                       |           | Attend      | Attend other |         |        |
|                       |           | Home        | DCSD         | Private |        |
| Units x Yield         |           | School      | School       | School  | Total  |
| Elementary            |           | 5.87        | 11.80        | 2.66    | 20.33  |
| Middle                |           | 8.48        | 2.22         | 0.00    | 10.70  |
| High                  |           | 8.56        | 2.72         | 0.00    | 11.28  |
| Total                 |           | 22.91       | 16.74        | 2.66    | 42.31  |
|                       |           | Attend      | Attend other |         |        |
|                       |           | Home        | DCSD         | Private |        |
| Anticipated Stud      | ents      | School      | School       | School  | Total  |
| Indian Creek Elementa | ry School | 6           | 12           | 3       | 21     |
| Freedom Middle S      | chool     | 8           | 2            | 0       | 10     |
| Clarkston High So     | hool      | 9           | 3            | 0       | 12     |
| Total                 | ·         | 23          | 17           | 3       | 43     |

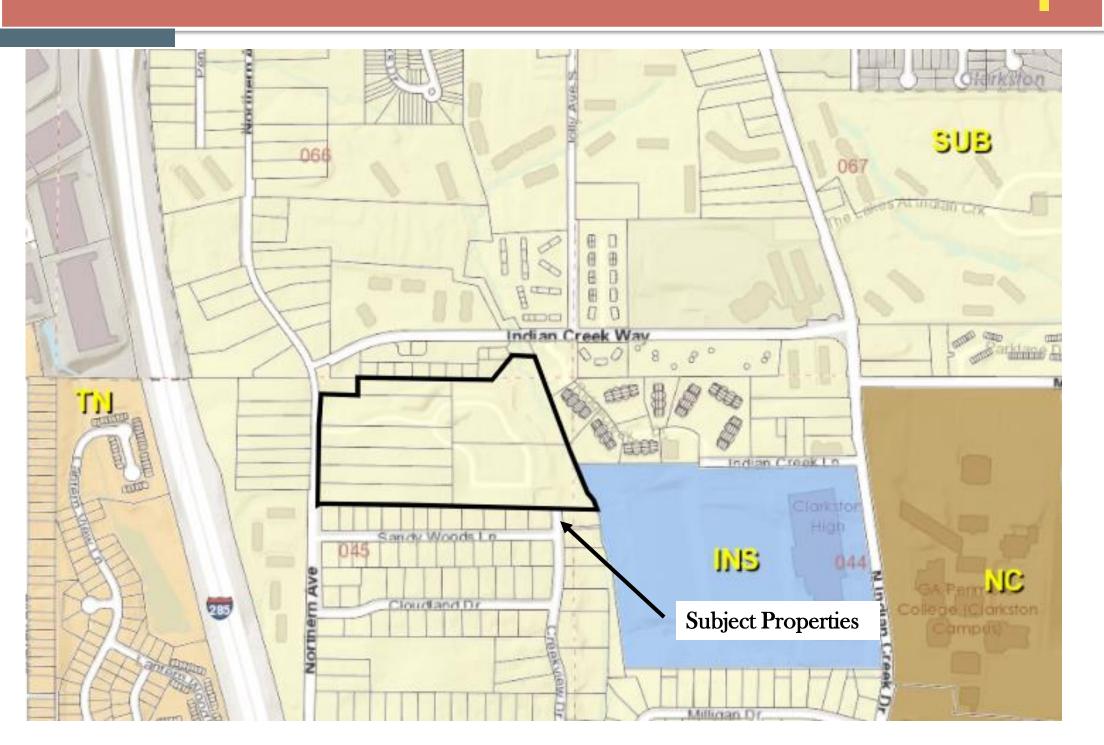


D8 Z 21 1244531 ZONING MAP



D8 Z 21 1244531 ZONING MAP

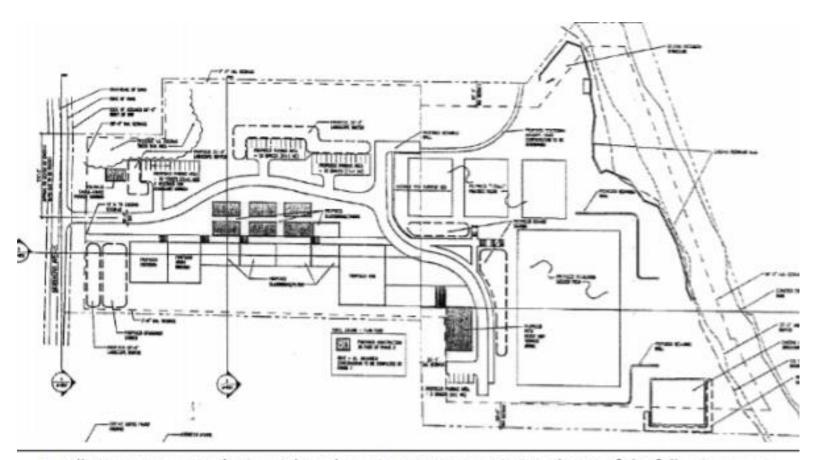




**D8 Z 21 1244531** Aerial



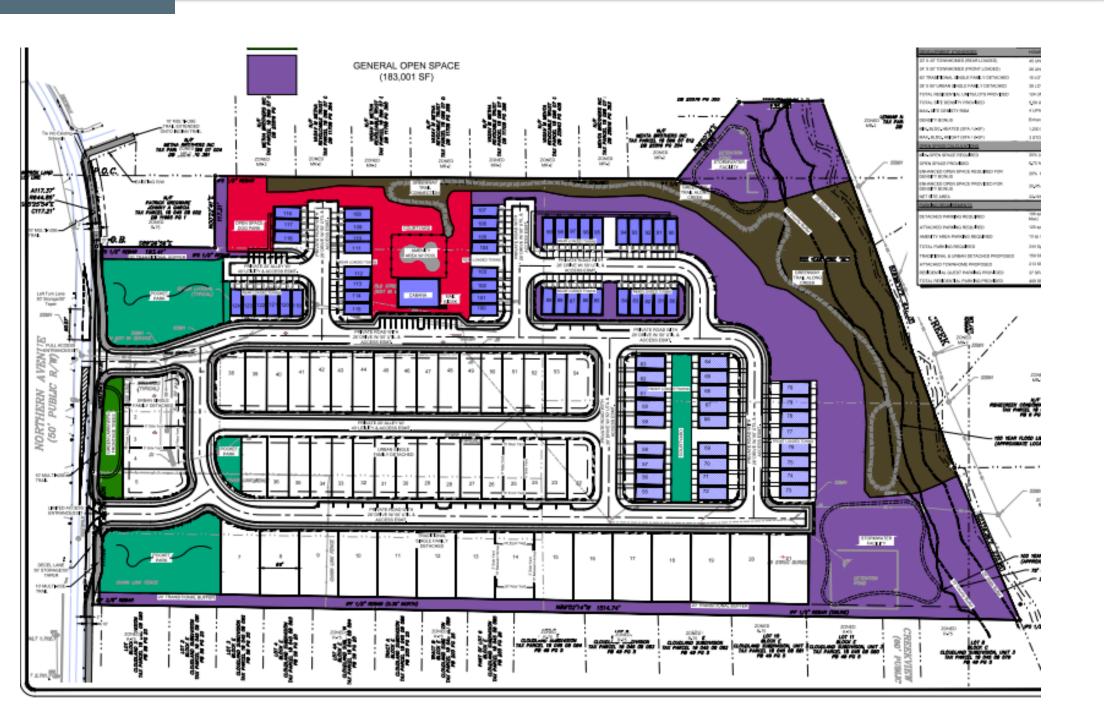
#### **CURRENT ZONING IS APPROVED FOR PRIVATE SCHOOL FOR 120 STUDENTS**

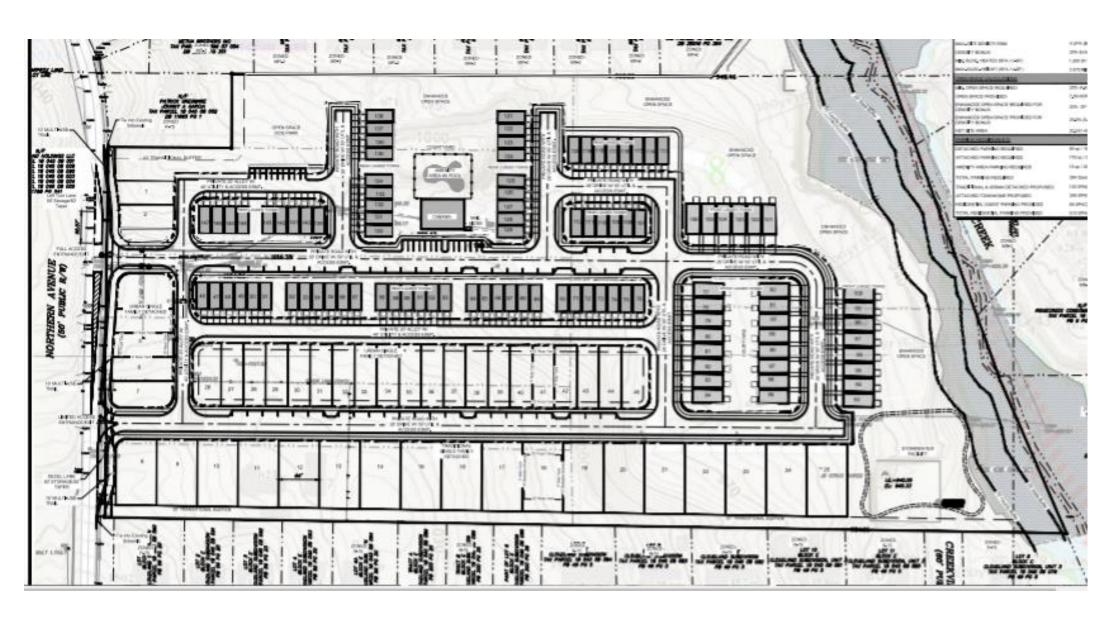


DeKalb County approved a Special Land Use Permit in June 2010, inclusive of the following uses:

- Private middle school w/ a max capacity of 120 students
- Two soccer fields
- Faculty residence
- · Community garden

#### **NEW REVISED SITE PLAN**





#### **Adjacent Land Uses** Townhomes Single-Family Detached Multi-family Multi-family Indian Creek Elementary Single-Family School Detached Multi-family Townhomes Clarkston Multifamily Georgia State University Perimeter Single-Family Detached Single-Family Detached Townhomes



#### **CONCEPTUAL ELEVATIONS**





















#### APPLICATION TO AMEND OFFICIAL ZONING MAP OF DEKALB COUNTY, GEORGIA

|                       |  | Z/CZ No  |
|-----------------------|--|--|
|                       |  | Filing Fee:  |
| Date Received         | I: Applica   | ation No.:   |
| Applicant:            | Inline Communities LLC c/o Battle Law PC                         | E-Mail: mlb@battlelawpc.com  |
| Applicant Maili<br>On | ng Address:<br>e West Court Square , Suite 750, Decatur GA 30030 |  |
| Applicant Phor        | ne: (404) 601-7616   | Fax: (404) 745-0045  |
| ******                | ***********  | *************  |
|                       | efer to attachment   | E-Mail:  |
| (If r                 | more than one owner, attach as Exhibit "A")                      |  |
| Owner's Mailin        | ng Address:  |  |
| Owner(s) Phor         | ne:  | Fax:   |
| Address/Locat         | ion of Subject Property:671, 657, 635, 655, 64                   | 49, 641, 631, 623 Northern Ave Clarkston GA 30021                      |
| District(s): 18       | Land Lot(s):045 Bl   | ock: <u>08</u> Parcel(s: <u>095, 001, 008, 003, 005, 006, 007, 004</u> |
| Acreage: 22.07        | Commission   | District(s): District 4, Super District 6                              |
| Present Zoning        | g Category: R-75 & MR-2 Prop                                     | posed Zoning Category: RSM   |
| Present Land I        | Jse Category: Suburban   | ******   |
|                       | PLEASE READ THE FOLLOW   | ING BEFORE SIGNING   |

### attachments and filing fees identified on the attachments. An application, which lacks any of the required attachments, shall be determined as incomplete and shall not be accepted.

This form must be completed in its entirety before the Planning Department accepts it. It must include the

**Disclosure of Campaign Contributions** 

# TRAFFIC IMPACT STUDY FOR RESIDENTIAL DEVELOPMENT ON NORTHERN AVENUE DEKALB COUNTY, GEORGIA



#### Prepared for:

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#### Prepared By:



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> January 14, 2021 A & R Project # 20-147

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#### 1.0 INTRODUCTION

The purpose of this study is to determine the traffic impact that will result from the proposed residential development located in the southeast corner of the intersection of Northern Avenue and Indian Creek Way in DeKalb County, Georgia. The traffic analysis evaluates the current operations compared to the future conditions with the traffic generated by the development. The proposed development will consist of:

- 26 Single-family detached housing units and
- 139 Multifamily Low-Rise Housing units



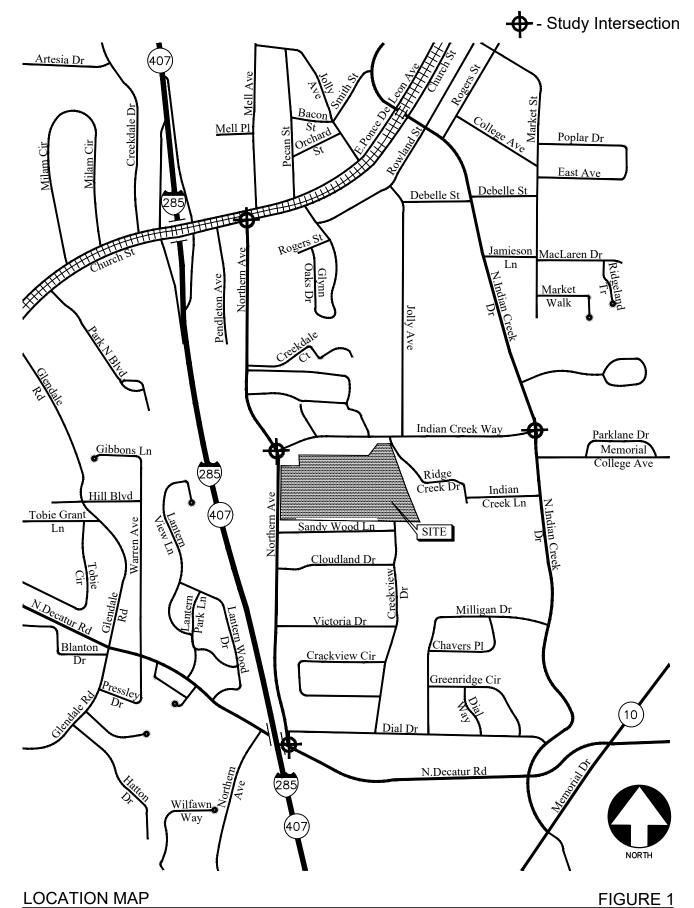
The development proposes access at the following locations:

- Site Driveway 1: Full-access (northern) driveway on Northern Avenue
- Site Driveway 2: Full-access (southern) driveway on Northern Avenue

The AM and PM peak hours have been analyzed in this study. In addition to the site driveways, this study includes the evaluation of traffic operations at the intersections of:

- Church Street at Northern Avenue
- Northern Avenue at Indian Creek Way
- N. Decatur Road at Northern Avenue
- N. Indian Creek Drive at Indian Creek Way

Recommendations to improve traffic operations have been identified as appropriate and are discussed in detail in the following sections of the report. The location of the development and the surrounding roadway network is shown in Figure 1.



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#### 2.0 EXISTING FACILITIES / CONDITIONS

#### 2.1 Roadway Facilities

The following is a brief description of each of the roadway facilities located in proximity to the site:

#### 2.1.1 Church Street

Church Street is an east-west, two-lane, undivided roadway with a posted speed limit of 35 mph in the vicinity of the site.

#### 2.1.2 Northern Avenue

Northern Avenue is a north-south, two-lane, undivided roadway with a posted speed limit of 35 mph in the vicinity of the site.

#### 2.1.3 Indian Creek Way

Indian Creek Way is an east-west, two-lane, undivided roadway with a posted speed limit of 35 mph.

#### 2.1.4 N. Indian Creek Drive

N. Indian Creek Drive is a north-south, three-lane roadway with a two-way left-turn lane and posted speed limit of 35 mph in the vicinity of the site. GDOT traffic counts (Station ID 089-3754) indicate that the daily traffic volume on N. Indian Creek Drive in 2019 was 17,400 vehicles per day north of Indian Creek Way. GDOT classifies N. Indian Creek Drive as an Urban Minor Collector roadway.

#### 2.1.5 N. Decatur Road

N. Decatur Road is an east-west, four-lane, undivided roadway with a posted speed limit of 40 mph in the vicinity of the site. GDOT traffic counts (Station ID 089-3729) indicate that the daily traffic volume on N. Decatur Road in 2019 was 15,200 vehicles per day between Northern Avenue and N. Indian Creek Drive. GDOT classifies N. Decatur Road as an Urban Minor Arterial roadway.

#### 3.0 STUDY METHODOLOGY

In this study, the methodology used for evaluating traffic operations at each of the subject intersections is based on the criteria set forth in the Transportation Research Board's <u>Highway Capacity Manual</u>, 6<sup>th</sup> edition (HCM 6). Synchro software, which utilizes the HCM 6 methodology, was used for the analysis. The following is a description of the methodology employed for the analysis of unsignalized and signalized intersections.

#### 3.1 Unsignalized Intersections

For unsignalized intersections at which the side street or minor street is controlled by a stop sign, the criteria for evaluating traffic operations are the level-of-service (LOS) for the turning movements at the intersection and the level-of-service for the overall intersection. Level-of-service is based on the average controlled delay incurred at the intersection. Controlled delay for unsignalized intersections includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. Several factors affect the controlled delay for unsignalized intersections, such as the availability and distribution of gaps in the conflicting traffic stream, critical gaps, and follow-up time for a vehicle in the queue.

Level-of-service is assigned a letter designation from "A" through "F". Level-of-service "A" indicates excellent operations with little delay to motorists, while level-of-service "F" exists when there are insufficient gaps of acceptable size to allow vehicles on the side street to cross safely, resulting in extremely long total delays and long queues. The level-of-service criteria for two-way stop-controlled and all-way stop-controlled (unsignalized) intersections are given in Table 1.

| Table 1 — Level-of-service Criteria for Unsignalized Intersections |               |  |  |  |
|--|---------------|--|--|--|
| Level-of-service Average Delay (sec)                               |               |  |  |  |
| Α  | ≤ 10          |  |  |  |
| <b>B</b> > 10 and ≤ 15   |               |  |  |  |
| С  | > 15 and ≤ 25 |  |  |  |
| D  | > 25 and ≤ 35 |  |  |  |
| E  | > 35 and ≤ 50 |  |  |  |
| F > 50   |               |  |  |  |

Source: Highway Capacity Manual

#### 3.2 Signalized Intersections

For signalized intersections, it is necessary to evaluate both capacity and level-of-service in order to evaluate the overall operation of the intersection. The capacity analysis of an intersection is performed by comparing the volume of traffic using the various lane groups at the intersection to the capacity of those lane groups. This results in a volume/capacity (v/c) ratio for each lane group. A v/c ratio greater than 1.0 indicates that the volume of traffic has exceeded the capacity available, resulting in a temporary excess of demand. Although the capacity of the entire intersection is not defined, a composite v/c ratio for the sum of the critical lane groups within the intersection is computed. This composite v/c ratio is an indication of the overall intersection sufficiency.

Level-of-service for a signalized intersection is defined in terms of average controlled delay per vehicle, which is composed of initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. The level-of-service criteria for signalized intersections, based on average controlled delay, are shown in Table 2. Level-of-service "A" indicates operations with very low controlled delay, while level-of-service "F" describes operations with extremely high average controlled delay. Level-of-service "E" is typically considered to be the limit of acceptable delay, and level-of-service "F" is considered unacceptable by most drivers.

| Table 2 – Level-of-service Criteria for Signalized Intersections |  |  |  |  |  |
|--|--|--|--|--|--|
| Level-of-service   | <b>f-service</b> Average Control Delay (sec) |  |  |  |  |
| Α  | ≤ 10   |  |  |  |  |
| <b>B</b> > 10 and ≤ 20   |  |  |  |  |  |
| С  | > 20 and ≤ 35                                |  |  |  |  |
| D  | > 35 and ≤ 55                                |  |  |  |  |
| <b>E</b> > 55 and ≤ 80   |  |  |  |  |  |
| F  | > 80   |  |  |  |  |

Source: Highway Capacity Manual

#### 4.0 Existing 2021 Traffic Analysis

#### 4.1 Existing Traffic Volumes

Existing traffic counts were obtained at the following study intersections:

- Church Street at Northern Avenue
- Northern Avenue at Indian Creek Way
- N. Decatur Road at Northern Avenue

Turning movement counts were collected on Thursday, January 7, 2021. All turning movement counts were recorded during the AM and PM peak hours between 7:00 AM to 9:00 AM and 4:00 PM to 6:00 PM, respectively. The four consecutive 15-minute interval volumes that summed to produce the highest volume at the intersections were then determined. These volumes make up the peak hour traffic volumes for the intersections counted and are shown in Figure 2.

We had evaluated the intersection of N. Indian Creek Drive and Indian Creek Way in 2018 for 2020 build-out year for the expansion of Indian Creek Elementary School from 950 students to 1,200 students. Since schools were closed at the time of collection of traffic counts now, we have used the projected Build 2020 traffic volumes at the intersection of N. Indian Creek Drive and Indian Creek Way, which included the 2018 traffic counts grown to 2020 and the projected school generated traffic after its expansion to 1,200 students. The 2020 build volumes from that project were grown for one year at a 1% growth rate to obtain the existing 2021 volumes. These 2021 volumes are also shown in Figure 2.

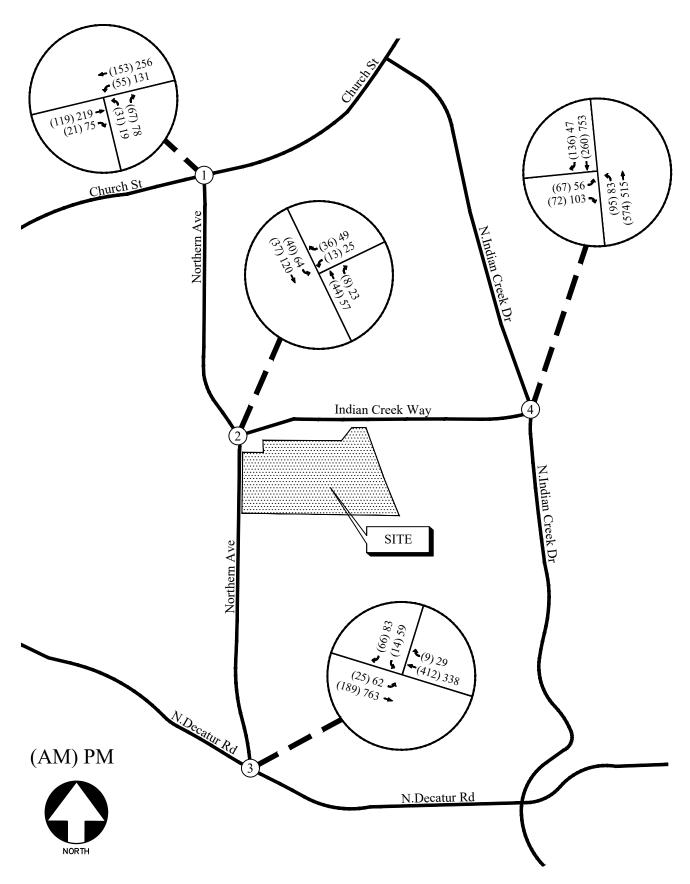
For the recently collected counts, since traffic patterns are irregular due to the COVID-19 pandemic, adjustment factors for the AM and PM peak hours were determined by comparing current traffic volumes to historic traffic volumes at a location that had historical GDOT counts available. GDOT had traffic counts available from 2009 at Station ID 089-3729 on N. Decatur Road. GDOT recorded counts from 2009 were increased by the annual growth rate of 1% for 12 years to project 2021 counts and compared to the new counts collected. The comparison of the projected 2021 GDOT counts and the new counts revealed that historic traffic volumes are higher by 70% in the AM peak hour and higher by 15% in the PM peak hour. Therefore, new turning movement counts were increased by 70% in the AM peak hour and 15% in the PM peak hour at all study intersections except the intersection of N. Indian Creek Drive and Indian Creek Way (please see above paragraph explaining the methodology for this intersection). No other adjustments were made to the 2021 volumes at the N. Indian Creek Drive at Indian Creek Way intersection The adjusted existing peak hour volumes are shown in Figure 3 and were used in the existing traffic operations analysis.

#### **4.2 Existing Traffic Operations**

Existing traffic operations were analyzed at the study intersections in accordance with the HCM methodology. The results of the analyses are shown in Table 3. The existing traffic control and lane geometry for the intersections are shown in Figure 4.

|              | Table 3 — Existing Intersection Operations   |                                   |   |  |  |  |
|--------------|--|-----------------------------------|---|--|--|--|
| Intersection |  | Traffic Control                   | LOS (Delay)                                       |  |  |  |
|              | intersection   | Traffic Control                   | AM Peak Hour                                      | PM Peak Hour                               |  |  |
| 1            | Church Street @ Northern Avenue -Westbound Left -Northbound Approach                                   | Stop Controlled on<br>NB Approach | A (8.0)<br>B (12.4)                               | A (8.5)<br>B (12.3)                        |  |  |
| 2            | Northern Avenue @ Indian Creek Way -Westbound Approach -Southbound Left                                | Stop Controlled on<br>WB Approach | A (9.8)<br>A (7.6)                                | B (10.4)<br>A (7.6)                        |  |  |
| 3            | N. Decatur Road St @ Northern Avenue -Eastbound Approach -Westbound Approach -Southbound Approach      | Signalized                        | A (2.8)<br>A (1.1)<br>A (1.5)<br>E (64.8)         | A (5.1)<br>A (2.0)<br>A (1.6)<br>E (68.1)  |  |  |
| 4            | N. Indian Creek Drive @ Indian Creek Way -Eastbound Approach -Northbound Approach -Southbound Approach | Signalized                        | <b>B (10.8)</b><br>E (62.0)<br>A (4.4)<br>A (3.6) | B (11.5)<br>E (61.3)<br>A (5.2)<br>A (6.2) |  |  |

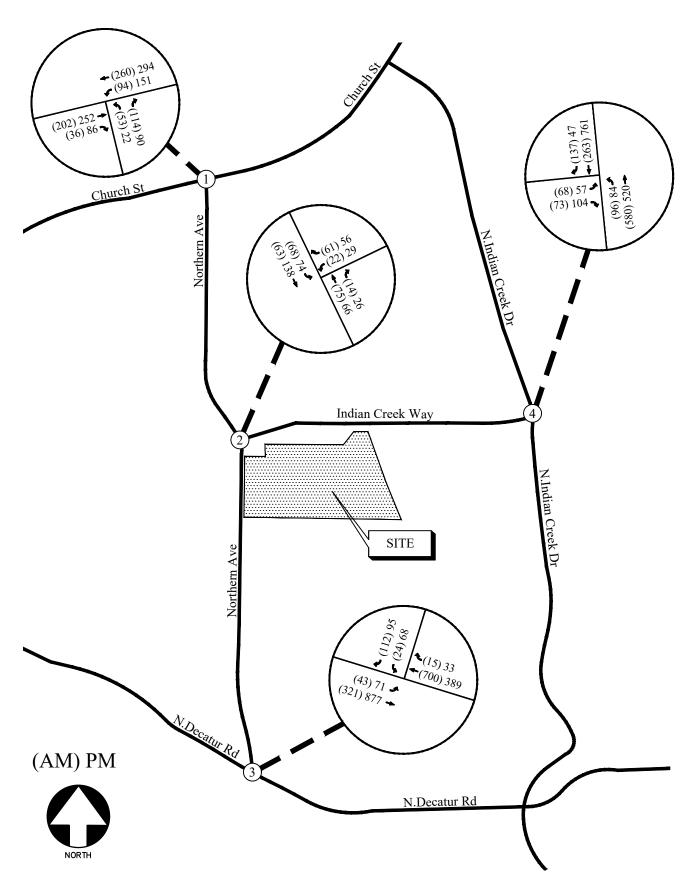
The results of existing traffic operations analysis indicate that all the study intersections are operating at satisfactory levels of service in both the AM and PM peak hours.



**EXISTING WEEKDAY PEAK-HOUR VOLUMES** 

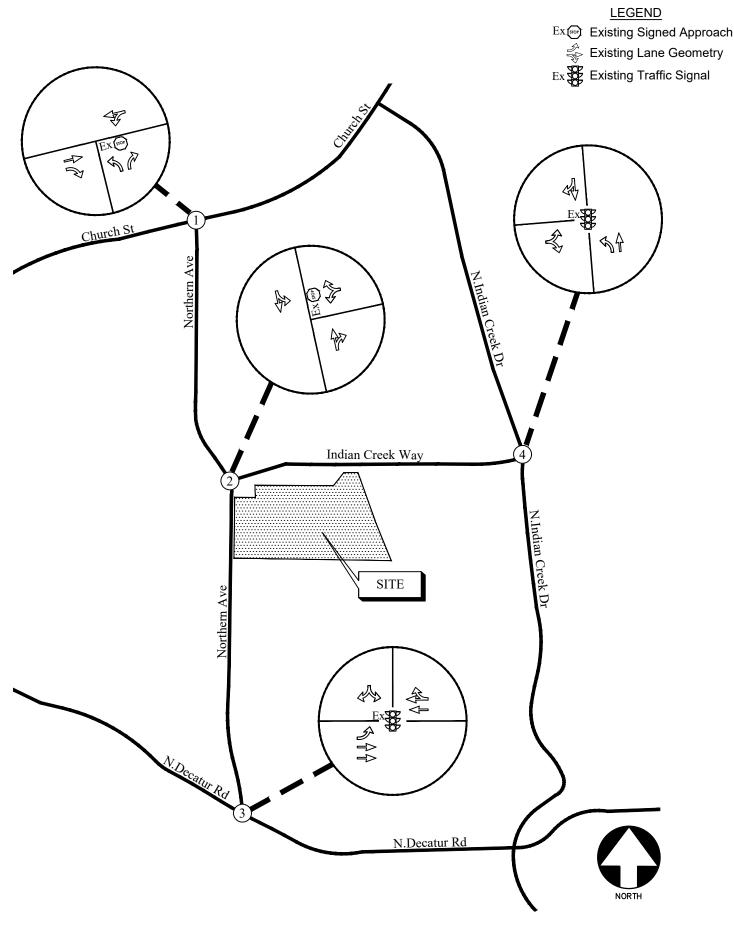
FIGURE 2

(DURING COVID-19)



ADJUSTED WEEKDAY PEAK-HOUR VOLUMES

FIGURE 3



EXISTING TRAFFIC CONTROL AND LANE GEOMETRY

#### 5.0 PROPOSED DEVELOPMENT

The proposed residential development will be located in the southeast corner of the intersection of Northern Avenue and Indian Creek Way in DeKalb County, Georgia. The development will consist of:

- 26 Single-family detached housing units and
- 139 Multifamily Low-Rise Housing units

The development proposes access at the following locations:

- Site Driveway 1: Full-access (northern) driveway on Northern Avenue
- Site Driveway 2: Full-access (southern) driveway on Northern Avenue

A site plan is shown in Figure 5.

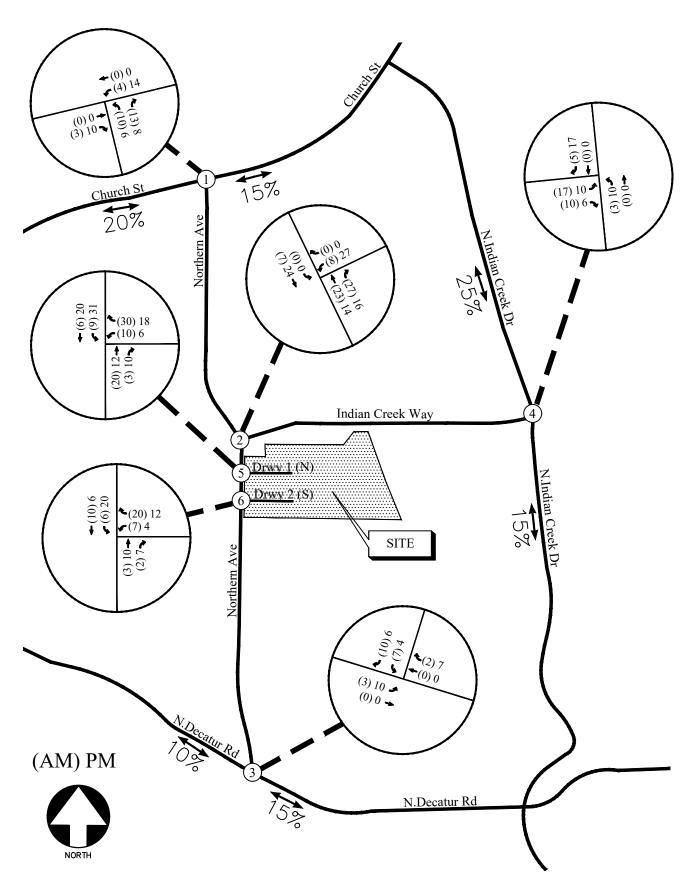
#### 5.1 Trip Generation

Trip generation estimates for the project were based on the rates and equations published in the 10th edition of the Institute of Transportation Engineers (ITE) Trip Generation report. This reference contains traffic volume count data collected at similar facilities nationwide. The trip generation was based on the following ITE Land Uses: 210 – Single-Family Detached Housing and 220 – Multifamily Housing (Low-Rise). The calculated total trip generation for the proposed development is shown in Table 4.

| Table 4 — Trip Generation                          |          |              |      |              |       |         |       |       |      |
|--|----------|--------------|------|--------------|-------|---------|-------|-------|------|
| Land Use   | Size     | AM Peak Hour |      | PM Peak Hour |       | 24-Hour |       |       |      |
|  |          | Enter        | Exit | Total        | Enter | Exit    | Total | Enter | Exit |
| ITE 210 – Single Family Detached Housing           | 26 units | 6            | 17   | 23           | 18    | 10      | 28    | 150   | 151  |
| ITE 220 – Multifamily Housing (Low-Rise) 139 units |          | 15           | 50   | 65           | 50    | 29      | 79    | 505   | 505  |
| Total Site Trips                                   |          | 21           | 67   | 88           | 68    | 39      | 107   | 655   | 656  |

#### **5.2** Trip Distribution

The trip distribution describes how traffic arrives and departs from the site. An overall trip distribution was developed for the site based on a review of the existing travel patterns in the area and the locations of major roadways and highways that will serve the development. The site-generated peak hour traffic volumes, shown in Table 4, were assigned to the study area intersections based on this distribution. The outer-leg distribution and AM and PM peak hour new traffic generated by the site are shown in Figure 6.



TRIP DISTRIBUTION AND SITE-GENERATED

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FIGURE 6

#### 6.0 FUTURE 2023 TRAFFIC ANALYSIS

The future 2023 traffic operations are analyzed for the "Build" and "No-Build" conditions.

#### 6.1 Future "No-Build" Conditions

The "No-Build" (or background) conditions provide an assessment of how traffic will operate in the study horizon year without the study site being developed as proposed, with projected increases in through traffic volumes due to normal annual growth. The Future "No-Build" volumes consist of the adjusted existing traffic volumes (Figure 3) plus increases for annual growth of through traffic.

#### 6.1.1 Annual Traffic Growth

To evaluate future traffic operations in this area, a projection of normal traffic growth was applied to the adjusted existing volumes. The Georgia Department of Transportation recorded average daily traffic volumes at several locations in the vicinity of the site. Reviewing the growth over the last three years revealed growth of approximately 1% in the area. This growth factor was applied to the adjusted existing traffic volumes (Figure 3) between collector and arterial roadways in order to estimate the future year traffic volumes prior to the addition of site-generated traffic. The resulting Future "No-Build" volumes on the roadway are shown in Figure 7.

#### 6.2 Future "Build" Conditions

The "Build" or development conditions include the estimated background traffic from the "No-Build" conditions plus the added traffic from the proposed development. In order to evaluate future traffic operations in this area, the additional traffic volumes from the site (Figure 6) were added to base traffic volumes (Figure 7) to calculate the future traffic volumes after the construction of the development. These total future "Build" traffic volumes are shown in Figure 8.

#### 6.3 Auxiliary Lane Analysis

Included below are analyses for left-turn lanes and deceleration lanes for all site driveways per GDOT standards. The analysis assumes that the average annual daily traffic (ADT) count on Northern Avenue is less than 6,000 vehicles per day based on the peak hour volumes on all three study intersections on Northern Avenue. The analysis is based on the trip distribution described in Section 5.2 and shown in Figure 6. The 24-hour two-way volume is 1,311 vehicles entering and exiting the site as shown in Trip Generation Table 4.

#### 6.3.1 Left Turn Lane Analysis

For two lane roadways with AADT's less than 6,000 vehicles and a posted speed limit of 35 mph, the daily site generated traffic left-turn movements threshold to warrant a left-turn lane is 300 left-turning vehicles a day. The projected left-turn volumes per day for each driveway are included below.

| TABLE 5 - GDOT REQUIREMENTS FOR LEFT TURN LANES |  |  |                                |                       |  |  |
|---|--|--|--------------------------------|-----------------------|--|--|
| Intersection                                    | Left-turn traffic<br>(% total<br>entering) | Left-turn Volume (veh/day)             | GDOT<br>Threshold<br>(veh/day) | Left-Turn<br>Warrants |  |  |
| Northern Avenue @<br>Site Driveway 1 (North)    | 45%  | 295<br>(total trips 1311) ÷ 2 × 0.45   | 300                            | Not Met               |  |  |
| Northern Avenue @<br>Site Driveway 2 (South)    | 30%  | 196<br>(total trips 1311) ÷ 2 × 0.30 = | 300                            | Not Met               |  |  |

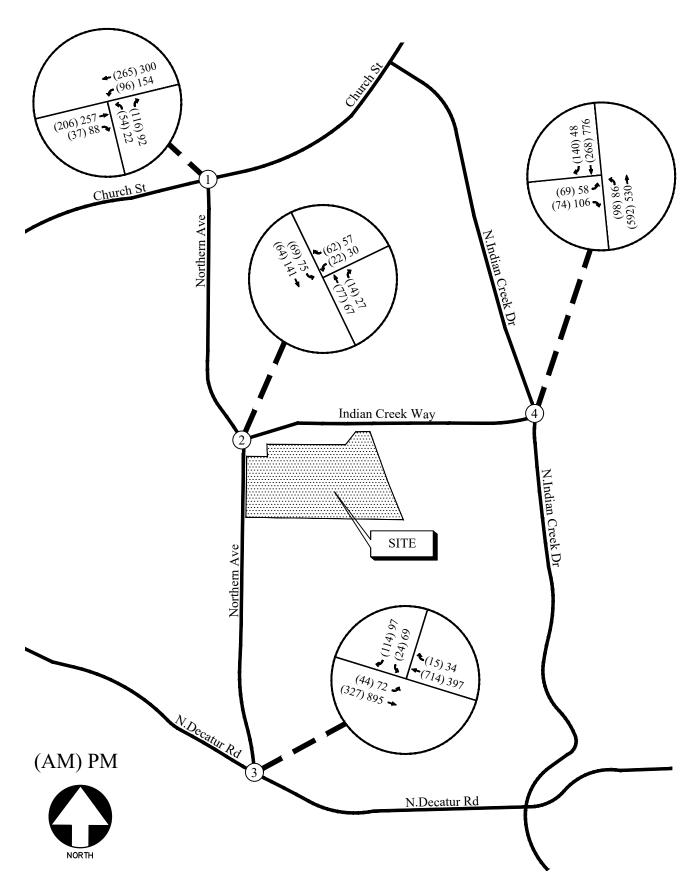
Since the projected number of left-turning vehicles is below the threshold of 300 left turning vehicles at both driveways, left-turn lanes are not warranted at both site driveways on Northern Avenue as per GDOT standards.

#### **6.3.2** Deceleration Turn Lane Analysis

For two lane roadways with AADT's less than 6,000 vehicles and a posted speed limit of 35 mph, the daily site generated right-turn volume threshold to warrant a deceleration lane is 200 right -turning vehicles a day. The projected right-turn volumes per day for each driveway are shown in Table 6.

| TABLE 6 - GDOT REQUIREMENTS FOR DECELERATION LANES |                                       |                                     |                                |                        |  |
|--|---------------------------------------|-------------------------------------|--------------------------------|------------------------|--|
| Intersection                                       | Right-turn traffic (% total entering) | Right-turn Volume<br>(veh/day)      | GDOT<br>Threshold<br>(veh/day) | Right-Turn<br>Warrants |  |
| Northern Avenue @<br>Site Driveway 1 (North)       | 15%                                   | 98<br>(total trips 1311) ÷ 2 × 0.15 | 200                            | Not Met                |  |
| Northern Avenue @<br>Site Driveway 2 (South)       | 10%                                   | 66<br>(total 1311 trips) ÷ 2 × 0.16 | 200                            | Not Met                |  |

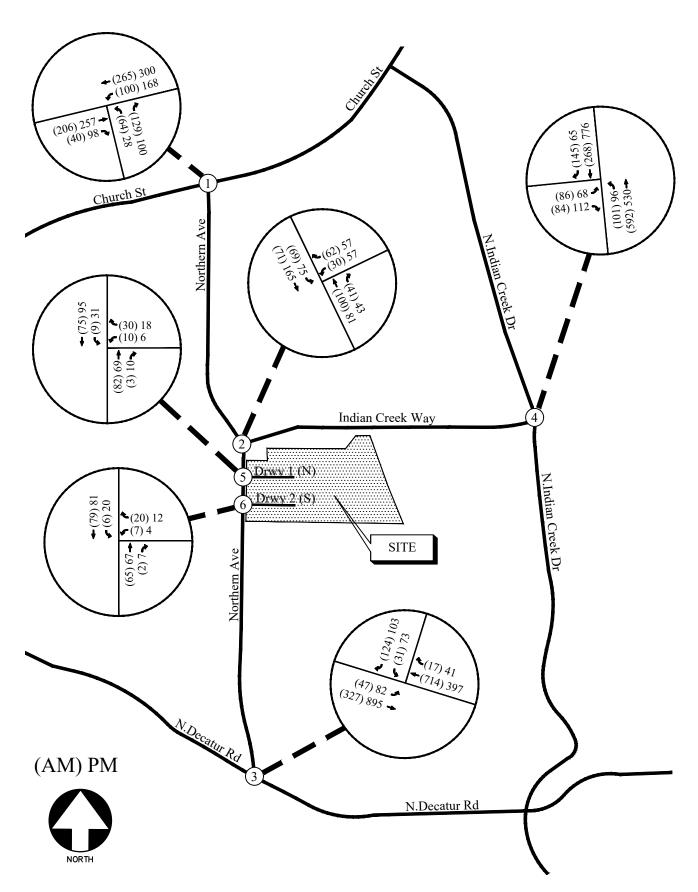
Since the projected number of right turning vehicles is below the threshold of 200 right turning vehicles, a deceleration lane is not warranted at both the site driveways on Northern Avenue as per GDOT standards.



FUTURE (NO-BUILD) WEEKDAY PEAK HOUR VOLUMES

FIGURE 7

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FUTURE (BUILD) WEEKDAY PEAK HOUR VOLUMES

FIGURE 8

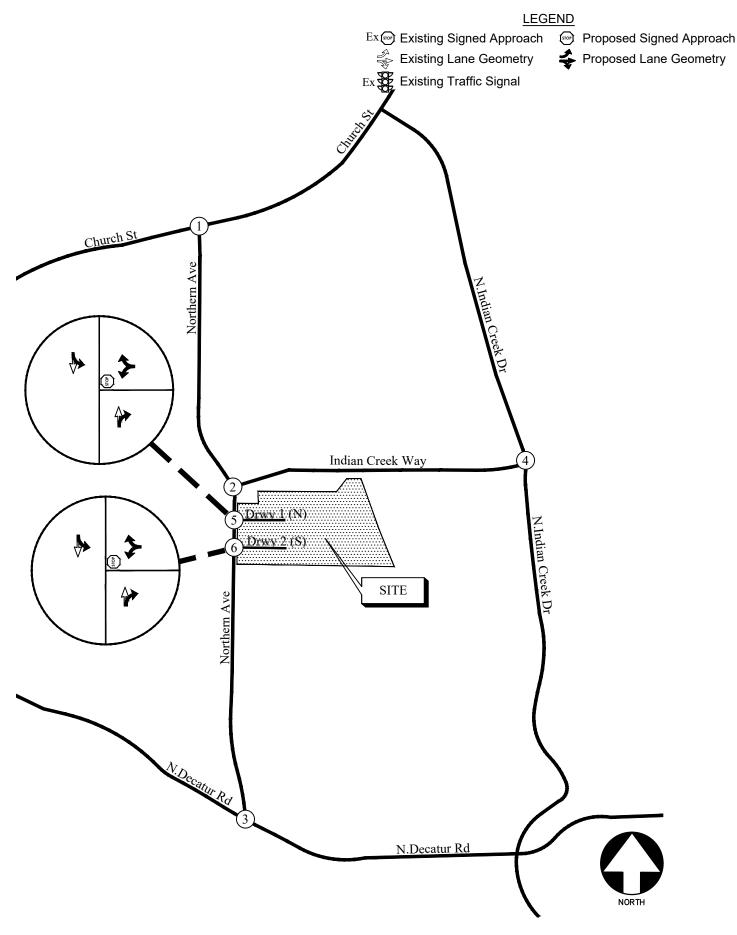
#### **6.4 Future Traffic Operations**

The future "No-Build" and "Build" traffic operations were analyzed using the volumes in Figure 7 and Figure 8, respectively. The results of the future traffic operations analysis are shown in Table 7.

| Table 7 – Future Intersection Operations |  |                               |                  |                 |                 |  |  |
|--|--|-------------------------------|------------------|-----------------|-----------------|--|--|
|  |  | Future Condition: LOS (Delay) |                  |                 |                 |  |  |
|  | Intersection                             | NO-B                          | UILD             | BU              | BUILD           |  |  |
|  |  | AM Peak                       | PM Peak          | AM Peak         | PM Peak         |  |  |
|  | Church Street @ Northern Avenue          |                               |                  |                 |                 |  |  |
| 1  | -Westbound Left                          | A (8.0)                       | A (8.6)          | A (8.1)         | A (8.7)         |  |  |
|  | -Northbound Approach                     | B (12.6)                      | B (12.7)         | B (13.0)        | B (13.5)        |  |  |
|  | Northern Avenue @ Indian Creek Way       |                               |                  |                 |                 |  |  |
| 2  | -Westbound Approach                      | A (9.8)                       | B (10.5)         | B (10.4)        | B (11.9)        |  |  |
|  | -Southbound Left                         | A (7.6)                       | A (7.6)          | A (7.7)         | A (7.7)         |  |  |
|  | N. Decatur Road St @ Northern Avenue     | <u>A (2.8)</u>                | <u>A (5.1)</u>   | <u>A (3.2)</u>  | <u>A (5.2)</u>  |  |  |
| 3  | -Eastbound Approach                      | A (1.1)                       | A (2.0)          | A (1.2)         | A (2.1)         |  |  |
| 3  | -Westbound Approach                      | A (1.5)                       | A (1.7)          | A (1.6)         | A (1.7)         |  |  |
|  | -Southbound Approach                     | E (64.8)                      | E (68.0)         | E (65.0)        | E (67.4)        |  |  |
|  | N. Indian Creek Drive @ Indian Creek Way | <u>B (10.9</u> )              | <u>B (11.7</u> ) | <u>B (12.4)</u> | <u>B (12.6)</u> |  |  |
| 4  | -Eastbound Approach                      | E (61.9)                      | E (61.1)         | E (60.5)        | E (60.3)        |  |  |
| 7  | -Northbound Approach                     | A (4.5)                       | A (5.4)          | A (5.2)         | A (6.1)         |  |  |
|  | -Southbound Approach                     | A (3.7)                       | A (6.5)          | A (4.3)         | A (7.2)         |  |  |
|  | Northern Avenue @ Site Drwy (North)      |                               |                  |                 |                 |  |  |
| 5  | -Westbound Approach                      |                               |                  | A (9.1)         | A (9.1)         |  |  |
|  | -Southbound Left                         | -                             | -                | A (7.4)         | A (7.4)         |  |  |
|  | Northern Avenue @ Site Drwy (South)      |                               |                  |                 |                 |  |  |
| 6  | -Westbound Approach                      |                               |                  | A (9.0)         | A (9.0)         |  |  |
|  | -Southbound Left                         | -                             | -                | A (7.4)         | A (7.4)         |  |  |

The future traffic operations analysis results show that all the study intersections will continue to operate at satisfactory levels of service in both the AM and PM peak hours in the future conditions. The impact of site generated traffic on traffic operations on study intersections is insignificant. No improvements are recommended to lane geometry and traffic controls at any study intersection.

Recommendations on traffic control and lane geometry at the site driveways are shown graphically in Figure 9.



**FUTURE TRAFFIC CONTROL AND LANE GEOMETRY** 

FIGURE 9

#### 7.0 CONCLUSIONS AND RECOMMENDATIONS

Traffic impacts were evaluated for the added traffic from the proposed residential development that will be located in the southeast corner of the intersection of Northern Avenue and Indian Creek Way in DeKalb County, Georgia. The proposed development will consist of:

- 26 Single-family detached housing units and
- 139 Multifamily Low-Rise Housing units

The AM and PM peak hours have been analyzed in this study. In addition to the site driveways, this study includes the evaluation of traffic operations at the intersections of:

- Church Street at Northern Avenue
- Northern Avenue at Indian Creek Way
- N. Decatur Road at Northern Avenue
- N. Indian Creek Drive at Indian Creek Way

#### 7.1 Site Access Configuration

The following access configuration is recommended for the proposed site driveway intersections:

- Site Driveway 1: Full-access (northern) driveway on Northern Avenue
  - To consist of one entering and one exiting lane. The westbound (driveway) approach to have a shared left/right-turn lane for exiting traffic.
  - o To be un-signalized with a STOP sign on the westbound approach.
  - o A left turn lane is not warranted based on GDOT standards (See Section 6.3.1).
  - o A deceleration lane is not warranted based on GDOT standards (See Section 6.3.2).
- Site Driveway 2: Full-access (southern) driveway on Northern Avenue
  - o To consist of one entering and one exiting lane. The westbound (driveway) approach to have a shared left/right-turn lane for exiting traffic.
  - o To be un-signalized with a STOP sign on the westbound approach.
  - o A left turn lane is not warranted based on GDOT standards (See Section 6.3.1).
  - o A deceleration lane is not warranted based on GDOT standards (See Section 6.3.2).

The future traffic operations analysis results show that all the study intersections will continue to operate at satisfactory levels of service in both the AM and PM peak hours. The impact of site generated traffic on traffic operations on study intersections is insignificant. No improvements are recommended to lane geometry and traffic controls at any study intersection.

### **Appendix**

| Existing Intersection Traffic Counts    |
|---|
| Linear Regression of Daily Traffic      |
| Existing Intersection Analysis          |
| Future "No-Build" Intersection Analysis |
| Future "Build" Intersection Analysis    |
| ·                                       |
| Traffic Volume Worksheets               |

| EXISTING | INTERSECTION | I TRAFFIC | COUNTS |
|----------|--------------|-----------|--------|
|          |              |           |        |

### A & R Engineering, In 2160 Kingston Court, Suite 'O',

Marietta, GA 30067

TMC Data Northern Ave @ Church St 7-9 am | 4-6 pm

File Name: 20210005 Site Code : 20210005 Start Date : 1/7/2021

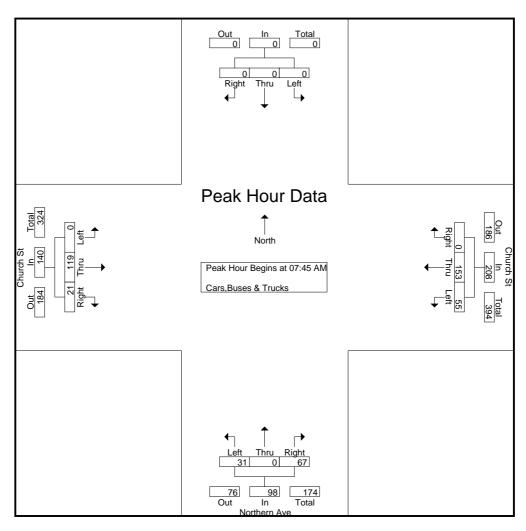
|               |              |      |       |            |      | Grou       | ps Prin | ted- Cars  | ,Buses    | & Tru     | cks   |            |           |      | rch St |            |            |
|---------------|--------------|------|-------|------------|------|------------|---------|------------|-----------|-----------|-------|------------|-----------|------|--------|------------|------------|
|               | Northern Ave |      |       |            |      |            |         |            | Church St |           |       |            |           |      |        |            |            |
|               |              |      | bound |            |      | Southbound |         |            |           | Eastbound |       |            | Westbound |      |        |            |            |
| Start Time    | Left         | Thru | Right | App. Total | Left | Thru       | Right   | App. Total | Left      | Thru      | Right | App. Total | Left      | Thru | Right  | App. Total | Int. Total |
| 07:00 AM      | 3            | 0    | 16    | 19         | 0    | 0          | 0       | 0          | 0         | 16        | 5     | 21         | 13        | 36   | 0      | 49         | 89         |
| 07:15 AM      | 11           | 0    | 17    | 28         | 0    | 0          | 0       | 0          | 0         | 22        | 6     | 28         | 15        | 44   | 0      | 59         | 115        |
| 07:30 AM      | 8            | 0    | 16    | 24         | 0    | 0          | 0       | 0          | 0         | 21        | 7     | 28         | 10        | 38   | 0      | 48         | 100        |
| 07:45 AM      | 11_          | 0    | 15    | 26         | 0    | 0          | 0       | 0          | 0         | 42        | 6     | 48         | 22        | 28   | 0      | 50         | 124        |
| Total         | 33           | 0    | 64    | 97         | 0    | 0          | 0       | 0          | 0         | 101       | 24    | 125        | 60        | 146  | 0      | 206        | 428        |
| 08:00 AM      | 4            | 0    | 15    | 19         | 0    | 0          | 0       | 0          | 0         | 23        | 8     | 31         | 14        | 35   | 0      | 49         | 99         |
| 08:15 AM      | 8            | 0    | 18    | 26         | 0    | 0          | 0       | 0          | 0         | 24        | 3     | 27         | 11        | 53   | 0      | 64         | 117        |
| 08:30 AM      | 8            | 0    | 19    | 27         | 0    | 0          | 0       | 0          | 0         | 30        | 4     | 34         | 8         | 37   | 0      | 45         | 106        |
| 08:45 AM      | 8            | 0    | 20    | 28         | 0    | 0          | 0       | 0          | 0         | 31        | 2     | 33         | 16        | 35   | 0      | 51         | 112        |
| Total         | 28           | 0    | 72    | 100        | 0    | 0          | 0       | 0          | 0         | 108       | 17    | 125        | 49        | 160  | 0      | 209        | 434        |
|               |              |      |       |            |      |            |         |            |           |           |       |            |           |      |        |            |            |
| *** BREAK *** |              |      |       |            |      |            |         |            |           |           |       |            |           |      |        |            |            |
| 04:00 PM      | 11           | 0    | 31    | 42         | 0    | 0          | 0       | 0          | 0         | 75        | 7     | 82         | 42        | 48   | 0      | 90         | 214        |
| 04:15 PM      | 9            | 0    | 22    | 31         | 0    | 0          | 0       | 0          | 0         | 60        | 6     | 66         | 43        | 62   | 0      | 105        | 202        |
| 04:30 PM      | 4            | 0    | 23    | 27         | 0    | 0          | 0       | 0          | 0         | 46        | 16    | 62         | 31        | 53   | 0      | 84         | 173        |
| 04:45 PM      | 4            | 0    | 15    | 19         | 0    | 0          | 0       | 0          | 0         | 48        | 9     | 57         | 36        | 51   | 0_     | 87         | 163        |
| Total         | 28           | 0    | 91    | 119        | 0    | 0          | 0       | 0          | 0         | 229       | 38    | 267        | 152       | 214  | 0      | 366        | 752        |
| 05:00 PM      | 5            | 0    | 17    | 22         | 0    | 0          | 0       | 0          | 0         | 49        | 25    | 74         | 35        | 83   | 0      | 118        | 214        |
| 05:15 PM      | 4            | 0    | 24    | 28         | 0    | 0          | 0       | 0          | 0         | 57        | 13    | 70         | 24        | 55   | 0      | 79         | 177        |
| 05:30 PM      | 5            | 0    | 16    | 21         | 0    | 0          | 0       | 0          | 0         | 60        | 21    | 81         | 34        | 63   | 0      | 97         | 199        |
| 05:45 PM      | 5            | 0    | 21    | 26         | 0    | 0          | 0       | 0          | 0         | 53        | 16    | 69         | 38        | 55   | 0      | 93         | 188        |
| Total         | 19           | 0    | 78    | 97         | 0    | 0          | 0       | 0          | 0         | 219       | 75    | 294        | 131       | 256  | 0      | 387        | 778        |
| Grand Total   | 108          | 0    | 305   | 413        | 0    | 0          | 0       | 0          | 0         | 657       | 154   | 811        | 392       | 776  | 0      | 1168       | 2392       |
| Apprch %      | 26.2         | 0    | 73.8  | 713        | 0    | 0          | 0       | ١          | 0         | 81        | 19    | 011        | 33.6      | 66.4 | 0      | 1100       | 2002       |
| Total %       | 4.5          | 0    | 12.8  | 17.3       | 0    | 0          | 0       | 0          | 0         | 27.5      | 6.4   | 33.9       | 16.4      | 32.4 | 0      | 48.8       |            |

# A & R Engineering, In 2160 Kingston Court, Suite 'O', Marietta, GA 30067

TMC Data Northern Ave @ Church St 7-9 am | 4-6 pm

File Name: 20210005 Site Code : 20210005 Start Date : 1/7/2021

|               | Northern Ave   |         |         |            |            |      |       |            | Chu       | rch St |       |            |           |      |       |            |            |
|---------------|--|---------|---------|------------|------------|------|-------|------------|-----------|--------|-------|------------|-----------|------|-------|------------|------------|
|               | Northbound   |         |         |            | Southbound |      |       |            | Eastbound |        |       |            | Westbound |      |       |            |            |
| Start Time    | Left   | Thru    | Right   | App. Total | Left       | Thru | Right | App. Total | Left      | Thru   | Right | App. Total | Left      | Thru | Right | App. Total | Int. Total |
| Peak Hour An  | Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 |         |         |            |            |      |       |            |           |        |       |            |           |      |       |            |            |
| Peak Hour for | Entire   | Interse | ction B | egins at   | 07:45 A    | ΑM   |       |            |           |        |       |            |           |      |       |            |            |
| 07:45 AM      | 11   | 0       | 15      | 26         | 0          | 0    | 0     | 0          | 0         | 42     | 6     | 48         | 22        | 28   | 0     | 50         | 124        |
| 08:00 AM      | 4  | 0       | 15      | 19         | 0          | 0    | 0     | 0          | 0         | 23     | 8     | 31         | 14        | 35   | 0     | 49         | 99         |
| 08:15 AM      | 8  | 0       | 18      | 26         | 0          | 0    | 0     | 0          | 0         | 24     | 3     | 27         | 11        | 53   | 0     | 64         | 117        |
| 08:30 AM      | 8  | 0       | 19      | 27         | 0          | 0    | 0     | 0          | 0         | 30     | 4     | 34         | 8         | 37   | 0     | 45         | 106        |
| Total Volume  | 31   | 0       | 67      | 98         | 0          | 0    | 0     | 0          | 0         | 119    | 21    | 140        | 55        | 153  | 0     | 208        | 446        |
| % App. Total  | 31.6   | 0       | 68.4    |            | 0          | 0    | 0     |            | 0         | 85     | 15_   |            | 26.4      | 73.6 | 0     |            |            |
| PHF           | .705   | .000    | .882    | .907       | .000       | .000 | .000  | .000       | .000      | .708   | .656  | .729       | .625      | .722 | .000  | .813       | .899       |

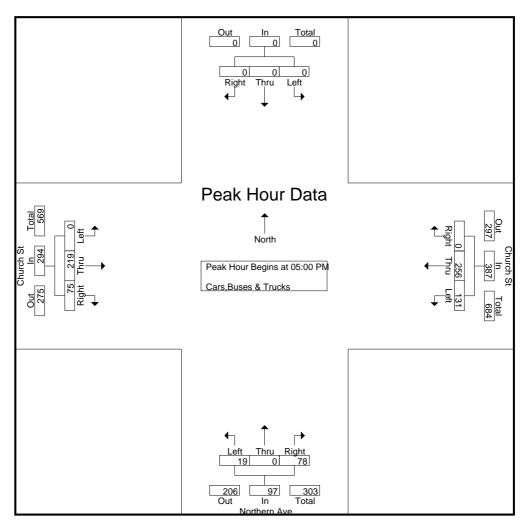


# A & R Engineering, In 2160 Kingston Court, Suite 'O', Marietta, GA 30067

TMC Data Northern Ave @ Church St 7-9 am | 4-6 pm

File Name : 20210005 Site Code : 20210005 Start Date : 1/7/2021

|               | Northern Ave   |         |         |            |            |      |       |            |           | Chu  | rch St |            |           |      |       |            |            |
|---------------|--|---------|---------|------------|------------|------|-------|------------|-----------|------|--------|------------|-----------|------|-------|------------|------------|
|               | Northbound   |         |         |            | Southbound |      |       |            | Eastbound |      |        |            | Westbound |      |       |            |            |
| Start Time    | Left   | Thru    | Right   | App. Total | Left       | Thru | Right | App. Total | Left      | Thru | Right  | App. Total | Left      | Thru | Right | App. Total | Int. Total |
| Peak Hour An  | Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 |         |         |            |            |      |       |            |           |      |        |            |           |      |       |            |            |
| Peak Hour for | Entire   | Interse | ction B | egins at   | 05:00 F    | PM   |       |            |           |      |        |            |           |      |       |            |            |
| 05:00 PM      | 5  | 0       | 17      | 22         | 0          | 0    | 0     | 0          | 0         | 49   | 25     | 74         | 35        | 83   | 0     | 118        | 214        |
| 05:15 PM      | 4  | 0       | 24      | 28         | 0          | 0    | 0     | 0          | 0         | 57   | 13     | 70         | 24        | 55   | 0     | 79         | 177        |
| 05:30 PM      | 5  | 0       | 16      | 21         | 0          | 0    | 0     | 0          | 0         | 60   | 21     | 81         | 34        | 63   | 0     | 97         | 199        |
| 05:45 PM      | 5  | 0       | 21      | 26         | 0          | 0    | 0     | 0          | 0         | 53   | 16     | 69         | 38        | 55   | 0     | 93         | 188        |
| Total Volume  | 19   | 0       | 78      | 97         | 0          | 0    | 0     | 0          | 0         | 219  | 75     | 294        | 131       | 256  | 0     | 387        | 778        |
| % App. Total  | 19.6   | 0       | 80.4    |            | 0          | 0    | 0     |            | 0         | 74.5 | 25.5   |            | 33.9      | 66.1 | 0     |            |            |
| PHF           | .950   | .000    | .813    | .866       | .000       | .000 | .000  | .000       | .000      | .913 | .750   | .907       | .862      | .771 | .000  | .820       | .909       |



# A & R Engineering, In 2160 Kingston Court, Suite 'O', Marietta, GA 30067

TMC Data Northern Avenue @ Indian Creek Way 7-9 am | 4-6 pm

File Name: 20210006 Site Code : 20210006

Start Date : 1/7/2021

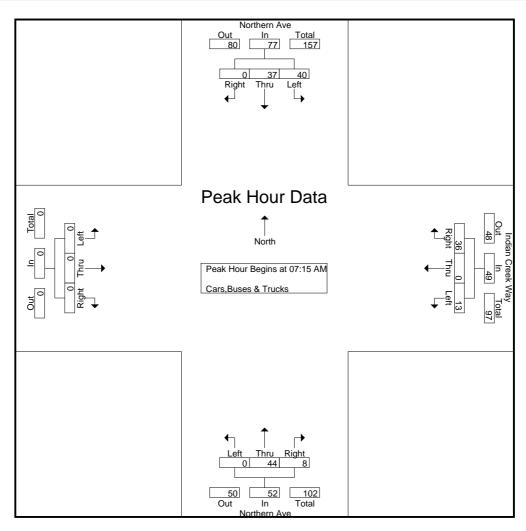
| Groups | Printed- | Cars | ,Buses | & | Trucks |
|--------|----------|------|--------|---|--------|
|--------|----------|------|--------|---|--------|

|                   | Northern Ave |      |       |            | Northern Ave |       |       |            | •    |      |       |            | In   |      |       |            |            |
|-------------------|--------------|------|-------|------------|--------------|-------|-------|------------|------|------|-------|------------|------|------|-------|------------|------------|
|                   |              |      | bound |            |              | South | bound |            |      |      | bound |            |      | West | bound |            |            |
| Start Time        | Left         | Thru | Right | App. Total | Left         | Thru  | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Int. Total |
| 07:00 AM          | 0            | 9    | 0     | 9          | 11           | 11    | 0     | 22         | 0    | 0    | 0     | 0          | 4    | 0    | 5     | 9          | 40         |
| 07:15 AM          | 0            | 16   | 2     | 18         | 8            | 9     | 0     | 17         | 0    | 0    | 0     | 0          | 5    | 0    | 12    | 17         | 52         |
| 07:30 AM          | 0            | 7    | 1     | 8          | 10           | 9     | 0     | 19         | 0    | 0    | 0     | 0          | 1    | 0    | 9     | 10         | 37         |
| 07:45 AM          | 0            | 10   | 2     | 12         | 14           | 9     | 0     | 23         | 0    | 0    | 0     | 0          | 3    | 0    | 7     | 10         | 45_        |
| Total             | 0            | 42   | 5     | 47         | 43           | 38    | 0     | 81         | 0    | 0    | 0     | 0          | 13   | 0    | 33    | 46         | 174        |
| 1                 |              |      |       |            |              |       |       |            |      |      |       |            |      |      |       |            |            |
| 08:00 AM          | 0            | 11   | 3     | 14         | 8            | 10    | 0     | 18         | 0    | 0    | 0     | 0          | 4    | 0    | 8     | 12         | 44         |
| 08:15 AM          | 0            | 14   | 2     | 16         | 1            | 11    | 0     | 12         | 0    | 0    | 0     | 0          | 2    | 0    | 8     | 10         | 38         |
| 08:30 AM          | 0            | 12   | 1     | 13         | 1            | 15    | 0     | 16         | 0    | 0    | 0     | 0          | 7    | 0    | 6     | 13         | 42         |
| 08:45 AM          | 0            | 11_  | 2     | 13         | 6            | 11    | 0     | 17         | 0    | 0    | 0     | 0          | 4    | 0    | 11    | 15         | 45_        |
| Total             | 0            | 48   | 8     | 56         | 16           | 47    | 0     | 63         | 0    | 0    | 0     | 0          | 17   | 0    | 33    | 50         | 169        |
| *** DDE \ \ \ *** |              |      |       |            |              |       |       |            |      |      |       |            |      |      |       |            |            |
| *** BREAK ***     |              |      |       |            |              |       |       |            |      |      |       |            |      |      |       |            |            |
| 04:00 PM          | 0            | 18   | 5     | 23         | 14           | 33    | 0     | 47         | 0    | 0    | 0     | 0          | 6    | 0    | 14    | 20         | 90         |
| 04:15 PM          | 0            | 18   | 3     | 21         | 15           | 35    | 0     | 50         | 0    | 0    | 0     | 0          | 2    | 0    | 9     | 11         | 82         |
| 04:30 PM          | 0            | 17   | 6     | 23         | 12           | 23    | 0     | 35         | 0    | 0    | 0     | 0          | 6    | 0    | 6     | 12         | 70         |
| 04:45 PM          | 0            | 11   | 3     | 14         | 16           | 30    | 0     | 46         | 0    | 0    | 0     | 0          | 7    | 0    | 11    | 18         | 78         |
| Total             | 0            | 64   | 17    | 81         | 57           | 121   | 0     | 178        | 0    | 0    | 0     | 0          | 21   | 0    | 40    | 61         | 320        |
|                   |              |      |       |            |              |       |       |            |      |      |       | 1          |      |      |       | 1          |            |
| 05:00 PM          | 0            | 16   | 5     | 21         | 15           | 29    | 0     | 44         | 0    | 0    | 0     | 0          | 8    | 0    | 14    | 22         | 87         |
| 05:15 PM          | 0            | 14   | 2     | 16         | 16           | 26    | 0     | 42         | 0    | 0    | 0     | 0          | 5    | 0    | 12    | 17         | 75         |
| 05:30 PM          | 0            | 16   | 13    | 29         | 17           | 35    | 0     | 52         | 0    | 0    | 0     | 0          | 5    | 0    | 12    | 17         | 98         |
| 05:45 PM          | 0            | 19   | 2     | 21         | 18           | 24    | 0     | 42         | 0    | 0    | 0     | 0          | 4    | 0    | 10    | 14         | 77_        |
| Total             | 0            | 65   | 22    | 87         | 66           | 114   | 0     | 180        | 0    | 0    | 0     | 0          | 22   | 0    | 48    | 70         | 337        |
| Grand Total       | 0            | 219  | 52    | 271        | 182          | 320   | 0     | 502        | 0    | 0    | 0     | 0          | 73   | 0    | 154   | 227        | 1000       |
| Apprch %          | 0            | 80.8 | 19.2  | -11        | 36.3         | 63.7  | 0     | 302        | 0    | 0    | 0     | U          | 32.2 | 0    | 67.8  | 221        | 1000       |
| Total %           | Ö            | 21.9 | 5.2   | 27.1       | 18.2         | 32    | Ö     | 50.2       | 0    | 0    | 0     | 0          | 7.3  | 0    | 15.4  | 22.7       |            |

TMC Data Northern Avenue @ Indian Creek Way 7-9 am | 4-6 pm

File Name: 20210006 Site Code : 20210006 Start Date : 1/7/2021

|               |          | North   |          | -          |         |        | ern Ave |            |      |      |       |            | In   |      | reek W | 'ay        |            |
|---------------|----------|---------|----------|------------|---------|--------|---------|------------|------|------|-------|------------|------|------|--------|------------|------------|
|               |          | North   | bound    |            |         | South  | bound   |            |      | East | bound |            |      | West | bound  |            |            |
| Start Time    | Left     | Thru    | Right    | App. Total | Left    | Thru   | Right   | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right  | App. Total | Int. Total |
| Peak Hour An  | alysis F | rom 07  | 7:00 AN  | 1 to 11:4  | 5 AM -  | Peak 1 | of 1    |            |      |      |       |            |      |      |        |            |            |
| Peak Hour for | Entire   | Interse | ction Be | egins at   | 07:15 A | M      |         |            |      |      |       |            |      |      |        |            |            |
| 07:15 AM      | 0        | 16      | 2        | 18         | 8       | 9      | 0       | 17         | 0    | 0    | 0     | 0          | 5    | 0    | 12     | 17         | 52         |
| 07:30 AM      | 0        | 7       | 1        | 8          | 10      | 9      | 0       | 19         | 0    | 0    | 0     | 0          | 1    | 0    | 9      | 10         | 37         |
| 07:45 AM      | 0        | 10      | 2        | 12         | 14      | 9      | 0       | 23         | 0    | 0    | 0     | 0          | 3    | 0    | 7      | 10         | 45         |
| MA 00:80      | 0        | 11      | 3        | 14         | 8       | 10     | 0       | 18         | 0    | 0    | 0     | 0          | 4    | 0    | 8      | 12         | 44         |
| Total Volume  | 0        | 44      | 8        | 52         | 40      | 37     | 0       | 77         | 0    | 0    | 0     | 0          | 13   | 0    | 36     | 49         | 178        |
| % App. Total  | 0        | 84.6    | 15.4     |            | 51.9    | 48.1   | 0       |            | 0    | 0    | 0     |            | 26.5 | 0    | 73.5   |            |            |
| PHF           | .000     | .688    | .667     | .722       | .714    | .925   | .000    | .837       | .000 | .000 | .000  | .000       | .650 | .000 | .750   | .721       | .856       |

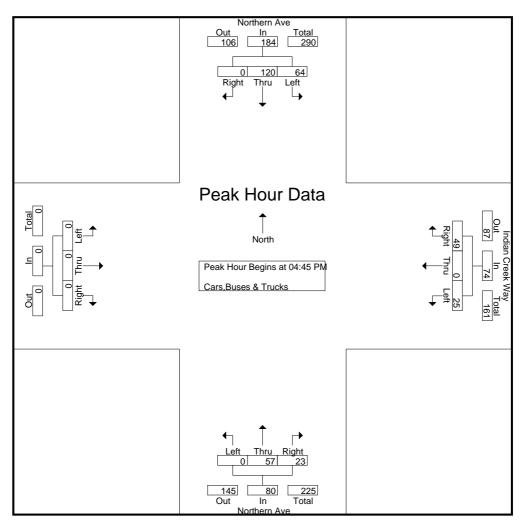


TMC Data Northern Avenue @ Indian Creek Way 7-9 am | 4-6 pm

Site Code : 20210006 Start Date : 1/7/2021

File Name: 20210006

|               |          |         | ern Avo | -          |         |        | ern Ave |            |      | East | bound |            | In   |      | reek W<br>bound | -          |            |
|---------------|----------|---------|---------|------------|---------|--------|---------|------------|------|------|-------|------------|------|------|-----------------|------------|------------|
| Start Time    | Left     | Thru    | Right   | App. Total | Left    | Thru   | Right   | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right           | App. Total | Int. Total |
| Peak Hour An  | alysis F | rom 04  | 1:00 PN | 1 to 05:4  | 5 PM -  | Peak 1 | of 1    |            |      |      |       |            |      |      |                 |            |            |
| Peak Hour for | Entire   | Interse | ction B | egins at   | 04:45 F | PM     |         |            |      |      |       |            |      |      |                 |            |            |
| 04:45 PM      | 0        | 11      | 3       | 14         | 16      | 30     | 0       | 46         | 0    | 0    | 0     | 0          | 7    | 0    | 11              | 18         | 78         |
| 05:00 PM      | 0        | 16      | 5       | 21         | 15      | 29     | 0       | 44         | 0    | 0    | 0     | 0          | 8    | 0    | 14              | 22         | 87         |
| 05:15 PM      | 0        | 14      | 2       | 16         | 16      | 26     | 0       | 42         | 0    | 0    | 0     | 0          | 5    | 0    | 12              | 17         | 75         |
| 05:30 PM      | 0        | 16      | 13      | 29         | 17      | 35     | 0       | 52         | 0    | 0    | 0     | 0          | 5    | 0    | 12              | 17         | 98         |
| Total Volume  | 0        | 57      | 23      | 80         | 64      | 120    | 0       | 184        | 0    | 0    | 0     | 0          | 25   | 0    | 49              | 74         | 338        |
| % App. Total  | 0        | 71.2    | 28.8    |            | 34.8    | 65.2   | 0       |            | 0    | 0    | 0     |            | 33.8 | 0    | 66.2            |            |            |
| PHF           | .000     | .891    | .442    | .690       | .941    | .857   | .000    | .885       | .000 | .000 | .000  | .000       | .781 | .000 | .875            | .841       | .862       |



TMC Data Indian Creek Way @ North Indian Creek Dr 7-9 am | 4-6 pm

File Name: 20210007 Site Code : 20210007 Start Date : 1/7/2021

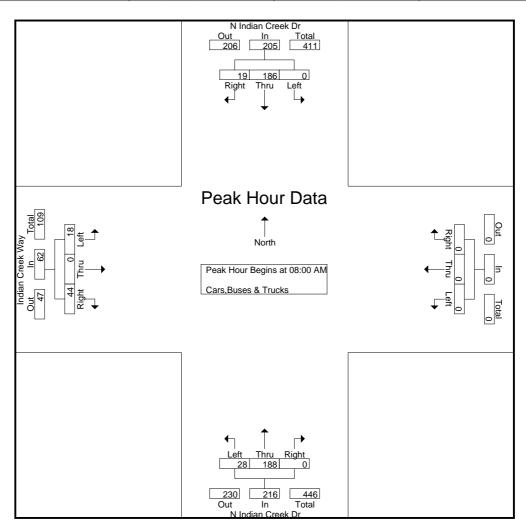
| Groups P | rinted- Car | s,Buses & | Trucks |
|----------|-------------|-----------|--------|
|----------|-------------|-----------|--------|

|                 | N    | Indian | Creek | Dr         | N    | Indian | Creek  | Dr         | In   | dian C | reek W | ay ay      |      |      |       |            |            |
|-----------------|------|--------|-------|------------|------|--------|--------|------------|------|--------|--------|------------|------|------|-------|------------|------------|
|                 |      | North  | bound |            |      | South  | nbound |            |      | East   | bound  |            |      | West | bound |            |            |
| Start Time      | Left | Thru   | Right | App. Total | Left | Thru   | Right  | App. Total | Left | Thru   | Right  | App. Total | Left | Thru | Right | App. Total | Int. Total |
| 07:00 AM        | 2    | 31     | 0     | 33         | 0    | 17     | 2      | 19         | 5    | 0      | 4      | 9          | 0    | 0    | 0     | 0          | 61         |
| 07:15 AM        | 10   | 34     | 0     | 44         | 0    | 28     | 4      | 32         | 4    | 0      | 5      | 9          | 0    | 0    | 0     | 0          | 85         |
| 07:30 AM        | 8    | 40     | 0     | 48         | 0    | 36     | 1      | 37         | 6    | 0      | 7      | 13         | 0    | 0    | 0     | 0          | 98         |
| 07:45 AM        | 11   | 42     | 0     | 53         | 0    | 36     | 5_     | 41         | 4    | 0      | 12_    | 16         | 0    | 0    | 0     | 0          | 110_       |
| Total           | 31   | 147    | 0     | 178        | 0    | 117    | 12     | 129        | 19   | 0      | 28     | 47         | 0    | 0    | 0     | 0          | 354        |
|                 | ı    |        |       |            |      |        |        |            |      |        |        |            |      |      |       |            |            |
| 08:00 AM        | 7    | 52     | 0     | 59         | 0    | 37     | 8      | 45         | 6    | 0      | 9      | 15         | 0    | 0    | 0     | 0          | 119        |
| 08:15 AM        | 10   | 45     | 0     | 55         | 0    | 48     | 3      | 51         | 3    | 0      | 12     | 15         | 0    | 0    | 0     | 0          | 121        |
| 08:30 AM        | 5    | 46     | 0     | 51         | 0    | 49     | 4      | 53         | 4    | 0      | 9      | 13         | 0    | 0    | 0     | 0          | 117        |
| 08:45 AM        | 6    | 45     | 00    | 51         | 0    | 52     | 4      | 56         | 5    | 0      | 14     | 19         | 0    | 0    | 0     | 0          | 126        |
| Total           | 28   | 188    | 0     | 216        | 0    | 186    | 19     | 205        | 18   | 0      | 44     | 62         | 0    | 0    | 0     | 0          | 483        |
| *** DDE ALC *** |      |        |       |            |      |        |        |            |      |        |        |            |      |      |       |            |            |
| *** BREAK ***   |      |        |       |            |      |        |        |            |      |        |        |            |      |      |       |            |            |
| 04:00 PM        | 17   | 96     | 0     | 113        | 0    | 136    | 13     | 149        | 15   | 0      | 20     | 35         | 0    | 0    | 0     | 0          | 297        |
| 04:15 PM        | 25   | 105    | 0     | 130        | 0    | 126    | 14     | 140        | 12   | 0      | 22     | 34         | 0    | 0    | 0     | 0          | 304        |
| 04:30 PM        | 13   | 110    | 0     | 123        | 0    | 139    | 11     | 150        | 5    | 0      | 26     | 31         | 0    | 0    | 0     | 0          | 304        |
| 04:45 PM        | 21   | 106    | 0     | 127        | 0    | 123    | 17_    | 140        | 6    | 0      | 19     | 25         | 0    | 0    | 0     | 0          | 292        |
| Total           | 76   | 417    | 0     | 493        | 0    | 524    | 55     | 579        | 38   | 0      | 87     | 125        | 0    | 0    | 0     | 0          | 1197       |
|                 | ii   |        |       |            |      |        |        | 1          |      |        |        | 1          |      |      |       | 1          |            |
| 05:00 PM        | 15   | 121    | 0     | 136        | 0    | 120    | 12     | 132        | 17   | 0      | 20     | 37         | 0    | 0    | 0     | 0          | 305        |
| 05:15 PM        | 20   | 93     | 0     | 113        | 0    | 116    | 11     | 127        | 9    | 0      | 21     | 30         | 0    | 0    | 0     | 0          | 270        |
| 05:30 PM        | 15   | 109    | 0     | 124        | 0    | 111    | 11     | 122        | 8    | 0      | 23     | 31         | 0    | 0    | 0     | 0          | 277        |
| 05:45 PM        | 25   | 105    | 0     | 130        | 0    | 99     | 12_    | 111        | 15   | 0      | 19_    | 34         | 0    | 0    | 0     | 0          | 275        |
| Total           | 75   | 428    | 0     | 503        | 0    | 446    | 46     | 492        | 49   | 0      | 83     | 132        | 0    | 0    | 0     | 0          | 1127       |
| Grand Total     | 210  | 1180   | 0     | 1390       | 0    | 1273   | 132    | 1405       | 124  | 0      | 242    | 366        | 0    | 0    | 0     | 0          | 3161       |
| Apprch %        | 15.1 | 84.9   | Ō     |            | Ö    | 90.6   | 9.4    |            | 33.9 | Ō      | 66.1   |            | 0    | 0    | Ō     |            |            |
| Total %         | 6.6  | 37.3   | 0     | 44         | 0    | 40.3   | 4.2    | 44.4       | 3.9  | 0      | 7.7    | 11.6       | 0    | 0    | 0     | 0          |            |

TMC Data Indian Creek Way @ North Indian Creek Dr 7-9 am | 4-6 pm

File Name: 20210007 Site Code : 20210007 Start Date : 1/7/2021

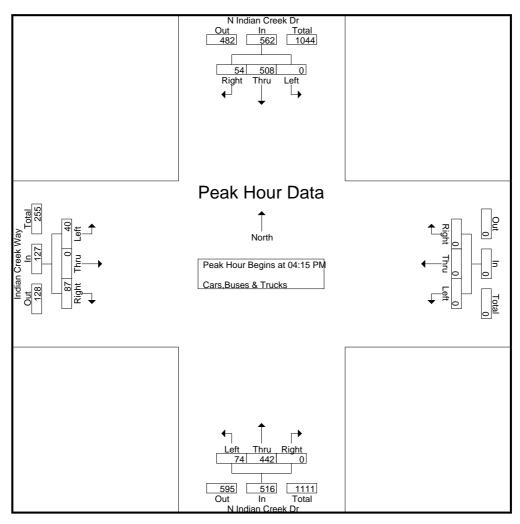
|               | N        |         | Creek   |            | N       |        | Creek  |            | In   |      | reek W | ay         |      | \A/( |       |            |            |
|---------------|----------|---------|---------|------------|---------|--------|--------|------------|------|------|--------|------------|------|------|-------|------------|------------|
|               |          | North   | bound   |            |         | Soutr  | nbound |            |      | ⊏ast | bound  |            |      | west | bound |            |            |
| Start Time    | Left     | Thru    | Right   | App. Total | Left    | Thru   | Right  | App. Total | Left | Thru | Right  | App. Total | Left | Thru | Right | App. Total | Int. Total |
| Peak Hour An  | alysis F | rom 07  | 7:00 AN | 1 to 08:4  | 5 AM -  | Peak 1 | of 1   |            |      |      |        |            |      |      |       |            |            |
| Peak Hour for | Entire I | Interse | ction B | egins at   | 08:00 A | M      |        |            |      |      |        |            |      |      |       |            |            |
| 08:00 AM      | 7        | 52      | 0       | 59         | 0       | 37     | 8      | 45         | 6    | 0    | 9      | 15         | 0    | 0    | 0     | 0          | 119        |
| 08:15 AM      | 10       | 45      | 0       | 55         | 0       | 48     | 3      | 51         | 3    | 0    | 12     | 15         | 0    | 0    | 0     | 0          | 121        |
| 08:30 AM      | 5        | 46      | 0       | 51         | 0       | 49     | 4      | 53         | 4    | 0    | 9      | 13         | 0    | 0    | 0     | 0          | 117        |
| 08:45 AM      | 6        | 45      | 0       | 51         | 0       | 52     | 4      | 56         | 5    | 0    | 14     | 19         | 0    | 0    | 0     | 0          | 126        |
| Total Volume  | 28       | 188     | 0       | 216        | 0       | 186    | 19     | 205        | 18   | 0    | 44     | 62         | 0    | 0    | 0     | 0          | 483        |
| % App. Total  | 13       | 87      | 0       |            | 0       | 90.7   | 9.3    |            | 29   | 0    | 71_    |            | 0    | 0    | 0     |            |            |
| PHF           | .700     | .904    | .000    | .915       | .000    | .894   | .594   | .915       | .750 | .000 | .786   | .816       | .000 | .000 | .000  | .000       | .958       |



Indian Creek Way @ North Indian Creek Dr 7-9 am | 4-6 pm

TMC Data File Name: 20210007 Site Code : 20210007 Start Date : 1/7/2021 Page No : 3

|               | N        |         | Creek   |            | N       |        | Creek |            | In   |      | reek W<br>bound | /ay        |      | West | bound |            |            |
|---------------|----------|---------|---------|------------|---------|--------|-------|------------|------|------|-----------------|------------|------|------|-------|------------|------------|
| Start Time    | Left     | Thru    | Right   | App. Total | Left    | Thru   | Right | App. Total | Left | Thru | Right           | App. Total | Left | Thru | Right | App. Total | Int. Total |
| Peak Hour An  | alysis F | rom 04  | 4:00 PN | 1 to 05:4  | 5 PM -  | Peak 1 | of 1  |            |      |      |                 |            |      |      |       |            |            |
| Peak Hour for | Entire   | Interse | ction B | egins at   | 04:15 F | PM     |       |            |      |      |                 |            |      |      |       |            |            |
| 04:15 PM      | 25       | 105     | 0       | 130        | 0       | 126    | 14    | 140        | 12   | 0    | 22              | 34         | 0    | 0    | 0     | 0          | 304        |
| 04:30 PM      | 13       | 110     | 0       | 123        | 0       | 139    | 11    | 150        | 5    | 0    | 26              | 31         | 0    | 0    | 0     | 0          | 304        |
| 04:45 PM      | 21       | 106     | 0       | 127        | 0       | 123    | 17    | 140        | 6    | 0    | 19              | 25         | 0    | 0    | 0     | 0          | 292        |
| 05:00 PM      | 15       | 121     | 0       | 136        | 0       | 120    | 12    | 132        | 17   | 0    | 20              | 37         | 0    | 0    | 0     | 0          | 305        |
| Total Volume  | 74       | 442     | 0       | 516        | 0       | 508    | 54    | 562        | 40   | 0    | 87              | 127        | 0    | 0    | 0     | 0          | 1205       |
| % App. Total  | 14.3     | 85.7    | 0       |            | 0       | 90.4   | 9.6   |            | 31.5 | 0    | 68.5            |            | 0    | 0    | 0     |            |            |
| PHF           | .740     | .913    | .000    | .949       | .000    | .914   | .794  | .937       | .588 | .000 | .837            | .858       | .000 | .000 | .000  | .000       | .988       |



TMC Data Northern Ave @ North Decatur Rd 7-9 am | 4-6 pm

File Name: 20210008 Site Code : 20210008 Start Date : 1/7/2021

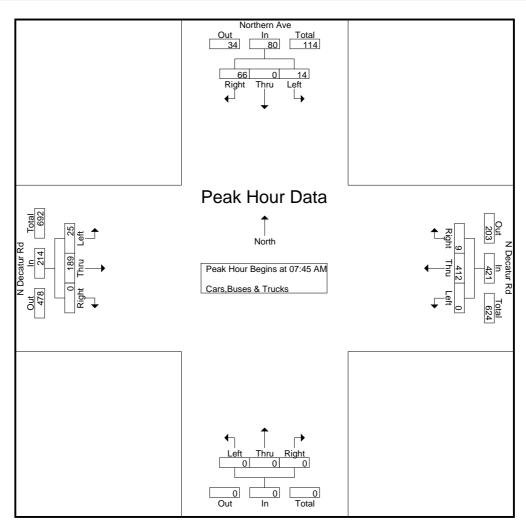
| Groups | Printed- | Cars, Buses | & Trucks |
|--------|----------|-------------|----------|
|--------|----------|-------------|----------|

|               |      |       |       |            |      |       | ern Ave |            | •    | N Dec | atur Rd | l          |      | N Dec | atur Ro | I          |            |
|---------------|------|-------|-------|------------|------|-------|---------|------------|------|-------|---------|------------|------|-------|---------|------------|------------|
|               |      | North | bound |            |      | South | nbound  |            |      | East  | bound   |            |      | West  | bound   |            |            |
| Start Time    | Left | Thru  | Right | App. Total | Left | Thru  | Right   | App. Total | Left | Thru  | Right   | App. Total | Left | Thru  | Right   | App. Total | Int. Total |
| 07:00 AM      | 0    | 0     | 0     | 0          | 4    | 0     | 9       | 13         | 5    | 35    | 0       | 40         | 0    | 98    | 4       | 102        | 155        |
| 07:15 AM      | 0    | 0     | 0     | 0          | 3    | 0     | 10      | 13         | 9    | 22    | 0       | 31         | 0    | 93    | 2       | 95         | 139        |
| 07:30 AM      | 0    | 0     | 0     | 0          | 7    | 0     | 10      | 17         | 7    | 32    | 0       | 39         | 0    | 98    | 3       | 101        | 157        |
| 07:45 AM      | 0    | 0     | 0     | 0          | 3    | 0     | 9       | 12         | 5    | 48    | 0       | 53         | 0    | 124   | 1       | 125        | 190        |
| Total         | 0    | 0     | 0     | 0          | 17   | 0     | 38      | 55         | 26   | 137   | 0       | 163        | 0    | 413   | 10      | 423        | 641        |
|               |      |       |       |            |      |       |         |            |      |       |         |            |      |       |         |            |            |
| 08:00 AM      | 0    | 0     | 0     | 0          | 2    | 0     | 13      | 15         | 5    | 48    | 0       | 53         | 0    | 108   | 2       | 110        | 178        |
| 08:15 AM      | 0    | 0     | 0     | 0          | 3    | 0     | 17      | 20         | 5    | 46    | 0       | 51         | 0    | 88    | 2       | 90         | 161        |
| 08:30 AM      | 0    | 0     | 0     | 0          | 6    | 0     | 27      | 33         | 10   | 47    | 0       | 57         | 0    | 92    | 4       | 96         | 186        |
| 08:45 AM      | 0    | 0     | 0     | 0          | 6    | 0     | 13      | 19         | 6    | 55    | 0       | 61         | 0    | 103   | 5       | 108        | 188        |
| Total         | 0    | 0     | 0     | 0          | 17   | 0     | 70      | 87         | 26   | 196   | 0       | 222        | 0    | 391   | 13      | 404        | 713        |
|               |      |       |       |            |      |       |         |            |      |       |         |            |      |       |         |            |            |
| *** BREAK *** | •    |       |       |            |      |       |         |            |      |       |         |            |      |       |         |            |            |
| 04.00 514     |      | •     | •     | ا م        | 40   | •     | 00      | 00         | 40   | 400   | •       | 400        | •    | 00    | •       | 00         | 007        |
| 04:00 PM      | 0    | 0     | 0     | 0          | 10   | 0     | 26      | 36         | 13   | 169   | 0       | 182        | 0    | 83    | 6       | 89         | 307        |
| 04:15 PM      | 0    | 0     | 0     | 0          | 15   | 0     | 16      | 31         | 17   | 210   | 0       | 227        | 0    | 86    | 6       | 92         | 350        |
| 04:30 PM      | 0    | 0     | 0     | 0          | 14   | 0     | 21      | 35         | 14   | 172   | 0       | 186        | 0    | 86    | 5       | 91         | 312        |
| 04:45 PM      | 0    | 0     | 0     | 0          | 15   | 0     | 25      | 40         | 16   | 200   | 0       | 216        | 0    | 82    | 6       | 88         | 344        |
| Total         | 0    | 0     | 0     | 0          | 54   | 0     | 88      | 142        | 60   | 751   | 0       | 811        | 0    | 337   | 23      | 360        | 1313       |
| 05:00 PM      | 0    | 0     | 0     | 0          | 15   | 0     | 21      | 36         | 15   | 181   | 0       | 196        | 0    | 84    | 12      | 96         | 328        |
| 05:15 PM      | 0    | 0     | 0     | 0          | 9    | 0     | 26      | 35         | 18   | 199   | 0       | 217        | 0    | 77    | 8       | 85         | 337        |
| 05:30 PM      | 0    | 0     | 0     | 0          | 11   | 0     | 24      | 35         | 24   | 174   | 0       | 198        | 0    | 75    | 15      | 90         | 323        |
| 05:45 PM      | 0    | 0     | 0     | 0          | 7    | 0     | 16      | 23         | 13   | 174   | 0       | 189        | 0    | 83    | 7       | 90         | 302        |
| Total         | 0    | 0     | 0     | 0          | 42   | 0     | 87      | 129        | 70   | 730   | 0       | 800        | 0    | 319   | 42      | 361        | 1290       |
| Total         | . 0  | U     | U     | U I        | 42   | U     | 01      | 129        | 70   | 130   | U       | 300        | U    | 519   | 42      | 301        | 1230       |
| Grand Total   | 0    | 0     | 0     | 0          | 130  | 0     | 283     | 413        | 182  | 1814  | 0       | 1996       | 0    | 1460  | 88      | 1548       | 3957       |
| Apprch %      | 0    | Ö     | 0     |            | 31.5 | 0     | 68.5    | 710        | 9.1  | 90.9  | 0       | .500       | 0    | 94.3  | 5.7     | .540       | 5501       |
| Total %       | 0    | 0     | 0     | 0          | 3.3  | 0     | 7.2     | 10.4       | 4.6  | 45.8  | 0       | 50.4       | 0    | 36.9  | 2.2     | 39.1       |            |
| . Otal 70     |      | U     | U     | 0          | 5.0  | U     |         |            | 7.0  | .5.0  | U       | 50.4       | U    | 00.0  |         | 55.1       |            |

TMC Data Northern Ave @ North Decatur Rd 7-9 am | 4-6 pm

File Name: 20210008 Site Code : 20210008 Start Date : 1/7/2021

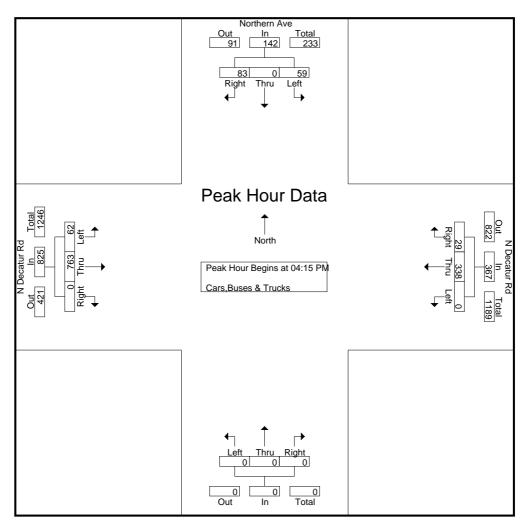
|               |          | North   | bound   |            |         |        | ern Ave |            |      |      | atur Rd<br>bound | l          |      |      | atur Ro | - 1        |            |
|---------------|----------|---------|---------|------------|---------|--------|---------|------------|------|------|------------------|------------|------|------|---------|------------|------------|
| Start Time    | Left     | Thru    | Right   | App. Total | Left    | Thru   | Right   | App. Total | Left | Thru | Right            | App. Total | Left | Thru | Right   | App. Total | Int. Total |
| Peak Hour An  | alysis F | rom 07  | 7:00 AN | 1 to 08:4  | 5 AM -  | Peak 1 | of 1    |            |      |      |                  |            |      |      |         |            |            |
| Peak Hour for | Entire   | Interse | ction B | egins at   | 07:45 A | M      |         |            |      |      |                  |            |      |      |         |            |            |
| 07:45 AM      | 0        | 0       | 0       | 0          | 3       | 0      | 9       | 12         | 5    | 48   | 0                | 53         | 0    | 124  | 1       | 125        | 190        |
| 08:00 AM      | 0        | 0       | 0       | 0          | 2       | 0      | 13      | 15         | 5    | 48   | 0                | 53         | 0    | 108  | 2       | 110        | 178        |
| 08:15 AM      | 0        | 0       | 0       | 0          | 3       | 0      | 17      | 20         | 5    | 46   | 0                | 51         | 0    | 88   | 2       | 90         | 161        |
| 08:30 AM      | 0        | 0       | 0       | 0          | 6       | 0      | 27      | 33         | 10   | 47   | 0                | 57         | 0    | 92   | 4       | 96         | 186        |
| Total Volume  | 0        | 0       | 0       | 0          | 14      | 0      | 66      | 80         | 25   | 189  | 0                | 214        | 0    | 412  | 9       | 421        | 715        |
| % App. Total  | 0        | 0       | 0       |            | 17.5    | 0      | 82.5    |            | 11.7 | 88.3 | 0                |            | 0    | 97.9 | 2.1     |            |            |
| PHF           | .000     | .000    | .000    | .000       | .583    | .000   | .611    | .606       | .625 | .984 | .000             | .939       | .000 | .831 | .563    | .842       | .941       |



TMC Data Northern Ave @ North Decatur Rd 7-9 am | 4-6 pm

File Name: 20210008 Site Code : 20210008 Start Date : 1/7/2021

|               |          | North  | bound   |            |         |        | ern Ave | - 1        |      |      | atur Ro | l          |      |      | atur Ro | -          |            |
|---------------|----------|--------|---------|------------|---------|--------|---------|------------|------|------|---------|------------|------|------|---------|------------|------------|
| Start Time    | Left     | Thru   | Right   | App. Total | Left    | Thru   | Right   | App. Total | Left | Thru | Right   | App. Total | Left | Thru | Right   | App. Total | Int. Total |
| Peak Hour An  | alysis F | rom 04 | 1:00 PM | 1 to 05:4  | 5 PM -  | Peak 1 | of 1    |            |      |      |         |            |      |      |         |            |            |
| Peak Hour for | Entire I | nterse | ction B | egins at   | 04:15 F | PM     |         |            |      |      |         |            |      |      |         |            |            |
| 04:15 PM      | 0        | 0      | 0       | 0          | 15      | 0      | 16      | 31         | 17   | 210  | 0       | 227        | 0    | 86   | 6       | 92         | 350        |
| 04:30 PM      | 0        | 0      | 0       | 0          | 14      | 0      | 21      | 35         | 14   | 172  | 0       | 186        | 0    | 86   | 5       | 91         | 312        |
| 04:45 PM      | 0        | 0      | 0       | 0          | 15      | 0      | 25      | 40         | 16   | 200  | 0       | 216        | 0    | 82   | 6       | 88         | 344        |
| 05:00 PM      | 0        | 0      | 0       | 0          | 15      | 0      | 21      | 36         | 15   | 181  | 0       | 196        | 0    | 84   | 12      | 96         | 328        |
| Total Volume  | 0        | 0      | 0       | 0          | 59      | 0      | 83      | 142        | 62   | 763  | 0       | 825        | 0    | 338  | 29      | 367        | 1334       |
| % App. Total  | 0        | 0      | 0       |            | 41.5    | 0      | 58.5    |            | 7.5  | 92.5 | 0       |            | 0    | 92.1 | 7.9     |            |            |
| PHF           | .000     | .000   | .000    | .000       | .983    | .000   | .830    | .888       | .912 | .908 | .000    | .909       | .000 | .983 | .604    | .956       | .953       |



### **Greater Traffic Company**

File Name: 01

Site Code : 00000000 Start Date : 9/5/2018

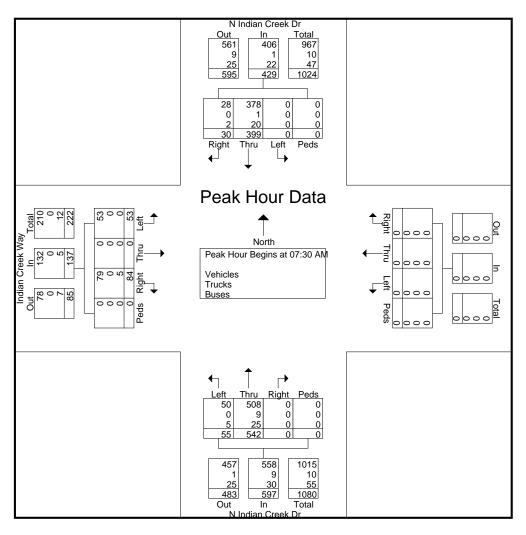
|                       |          |            |          |      |            | 1    |            |                |        | Vehicles   | s - Truc |      |           |        |            |      |      |        |      |            | 1           |
|-----------------------|----------|------------|----------|------|------------|------|------------|----------------|--------|------------|----------|------|-----------|--------|------------|------|------|--------|------|------------|-------------|
|                       |          |            | lian Cro |      |            |      |            |                | eek Dr |            |          |      |           | ek Way |            |      |      |        |      |            |             |
|                       |          | No         | rthbou   | nd   |            |      | So         | uthbou         | ınd    |            |          |      | astbou    |        |            |      |      | estbou |      |            |             |
| Start Time            | Left     | Thru       | Right    | Peds | App. Total | Left | Thru       | Right          | Peds   | App. Total | Left     | Thru | Right     | Peds   | App. Total | Left | Thru | Right  | Peds | App. Total | Int. Total  |
| 07:00 AM              | 19       | 122        | 0        | 0    | 141        | 0    | 38         | 7              | 0      | 45         | 15       | 0    | 6         | 0      | 21         | 0    | 0    | 0      | 0    | 0          | 207         |
| 07:15 AM              | 28       | 147        | 0        | 0    | 175        | 0    | 40         | 2              | 0      | 42         | 30       | 0    | 13        | 0      | 43         | 0    | 0    | 0      | 0    | 0          | 260         |
| 07:30 AM              | 17       | 151        | 0        | 0    | 168        | 0    | 69         | 4              | 0      | 73         | 29       | 0    | 20        | 0      | 49         | 0    | 0    | 0      | 0    | 0          | 290         |
| 07:45 AM              | 14       | 142        | 0        | 0    | 156        | 0    | 103        | 7              | 0      | 110        | 15       | 0    | 28        | 0      | 43         | 0    | 0    | 0      | 0    | 0          | 309         |
| Total                 | 78       | 562        | 0        | 0    | 640        | 0    | 250        | 20             | 0      | 270        | 89       | 0    | 67        | 0      | 156        | 0    | 0    | 0      | 0    | 0          | 1066        |
|                       | 1        |            |          |      |            |      |            |                |        |            | I        |      |           |        | _ 1        |      |      |        |      |            | 1           |
| 08:00 AM              | 15       | 118        | 0        | 0    | 133        | 0    | 118        | 10             | 0      | 128        | 6        | 0    | 20        | 0      | 26         | 0    | 0    | 0      | 0    | 0          | 287         |
| 08:15 AM              | 9        | 131        | 0        | 0    | 140        | 0    | 109        | 9              | 0      | 118        | 3        | 0    | 16        | 0      | 19         | 0    | 0    | 0      | 0    | 0          | 277         |
| 08:30 AM              | 12       | 137        | 0        | 0    | 149        | 0    | 97         | 7              | 0      | 104        | 12       | 0    | 13        | 0      | 25         | 0    | 0    | 0      | 0    | 0          | 278         |
| 08:45 AM              | 22       | 115        | 0        | 0    | 137        | 0    | 73         | 7              | 0      | 80         | 10       | 0    | 9         | 0      | 19         | 0    | 0    | 0      | 0    | 0          | 236         |
| Total                 | 58       | 501        | 0        | 0    | 559        | 0    | 397        | 33             | 0      | 430        | 31       | 0    | 58        | 0      | 89         | 0    | 0    | 0      | 0    | 0          | 1078        |
| *** BREAK *           | **       |            |          |      |            |      |            |                |        |            |          |      |           |        |            |      |      |        |      |            |             |
| 02:00 PM              | 24       | 134        | 0        | 0    | 158        | 0    | 118        | 19             | 0      | 137        | 15       | 0    | 18        | 0      | 33         | 0    | 0    | 0      | 0    | 0          | 328         |
| 02:00 I M<br>02:15 PM | 19       | 144        | 0        | 0    | 163        | 0    | 154        | 22             | 0      | 176        | 23       | 0    | 31        | 0      | 54         | 0    | 0    | 0      | 0    | 0          | 393         |
| 02:30 PM              | 29       | 120        | 0        | 0    | 149        | 0    | 94         | 15             | 0      | 109        | 23       | 0    | 25        | 0      | 46         | 0    | 0    | 0      | 0    | 0          | 393         |
| 02:45 PM              | 29       | 143        | 0        | 0    | 165        | 0    | 118        | 20             | 0      | 138        | 19       | 0    | 33        | 0      | 52         | 0    | 0    | 0      | 0    | 0          | 355         |
| -                     | 94       | 541        | 0        | 0    | 635        | 0    | 484        | 76             | 0      | 560        | 78       | 0    | 107       | 0      | 185        | 0    | 0    | 0      | 0    | 0          | 1380        |
| Total                 | 94       | 341        | U        | U    | 033        | 0    | 404        | 70             | U      | 300        | / / / /  | U    | 107       | U      | 165        | U    | U    | U      | U    | U          | 1360        |
| 03:00 PM              | 17       | 112        | 0        | 0    | 129        | 0    | 93         | 8              | 0      | 101        | 22       | 0    | 22        | 0      | 44         | 0    | 0    | 0      | 0    | 0          | 274         |
| 03:15 PM              | 14       | 130        | 0        | 0    | 144        | 0    | 116        | 12             | 0      | 128        | 19       | 0    | 23        | 0      | 42         | 0    | 0    | 0      | 0    | 0          | 314         |
| 03:30 PM              | 23       | 138        | 0        | 0    | 161        | 0    | 145        | 15             | 0      | 160        | 13       | 0    | 17        | 0      | 30         | 0    | 0    | 0      | 0    | 0          | 351         |
| 03:45 PM              | 20       | 149        | 0        | 0    | 169        | 0    | 147        | 8              | 0      | 155        | 15       | 0    | 41        | 0      | 56         | 0    | 0    | 0      | 0    | 0          | 380         |
| Total                 | 74       | 529        | 0        | 0    | 603        | 0    | 501        | 43             | 0      | 544        | 69       | 0    | 103       | 0      | 172        | 0    | 0    | 0      | 0    | 0          | 1319        |
| 04.00 <b>DM</b>       | 12       | 152        | 0        | 0    | 1.65       |      | 1.40       | _              | 0      | 151        | 1.5      | 0    | 22        | 0      | 47         | 0    | 0    | 0      | 0    | 0          | 200         |
| 04:00 PM              | 12<br>22 | 153<br>149 | 0        | 0    | 165        | 0    | 149<br>161 | 5              | 0      | 154        | 15<br>14 | 0    | 32<br>21  | 0      | 47<br>35   | 0    | 0    | 0      | 0    | 0          | 366         |
| 04:15 PM              |          |            | 0        | 0    | 171        | 0    |            | 10             | -      | 171        |          | -    |           | -      |            |      | -    | -      |      | -          | 377         |
| 04:30 PM              | 17       | 117        | 0        | 0    | 134        | 0    | 155        | 8              | 0      | 163        | 12       | 0    | 17        | 0      | 29         | 0    | 0    | 0      | 0    | 0          | 326         |
| 04:45 PM<br>Total     | 17<br>68 | 130<br>549 | 0        | 0    | 147<br>617 | 0    | 155<br>620 | <u>7</u><br>30 | 0      | 162<br>650 | 13<br>54 | 0    | 32<br>102 | 0      | 45<br>156  | 0    | 0    | 0      | 0    | 0          | 354<br>1423 |
| Total                 | 08       | 349        | U        | U    | 017        | 0    | 020        | 30             | U      | 030        | 34       | U    | 102       | U      | 130        | U    | U    | U      | U    | U          | 1423        |
| 05:00 PM              | 16       | 124        | 0        | 0    | 140        | 0    | 152        | 8              | 0      | 160        | 17       | 0    | 25        | 0      | 42         | 0    | 0    | 0      | 0    | 0          | 342         |
| 05:15 PM              | 17       | 137        | 0        | 0    | 154        | 0    | 169        | 14             | 0      | 183        | 13       | 0    | 25        | 0      | 38         | 0    | 0    | 0      | 0    | 0          | 375         |
| 05:30 PM              | 17       | 126        | 0        | 0    | 143        | 0    | 208        | 13             | 0      | 221        | 14       | 0    | 27        | 0      | 41         | 0    | 0    | 0      | 0    | 0          | 405         |
| 05:45 PM              | 30       | 108        | 0        | 0    | 138        | 0    | 196        | 7              | 0      | 203        | 10       | 0    | 21        | 0      | 31         | 0    | 0    | 0      | 0    | 0          | 372         |
| Total                 | 80       | 495        | 0        | 0    | 575        | 0    | 725        | 42             | 0      | 767        | 54       | 0    | 98        | 0      | 152        | 0    | 0    | 0      | 0    | 0          | 1494        |
| Grand Total           | 452      | 3177       | 0        | 0    | 3629       | 0    | 2977       | 244            | 0      | 3221       | 375      | 0    | 535       | 0      | 910        | 0    | 0    | 0      | 0    | 0          | 7760        |
| Apprch %              | 12.5     | 87.5       | 0        | 0    |            | 0    | 92.4       | 7.6            | 0      |            | 41.2     | 0    | 58.8      | 0      |            | 0    | 0    | 0      | 0    |            |             |
| Total %               | 5.8      | 40.9       | 0        | 0    | 46.8       | 0    | 38.4       | 3.1            | 0      | 41.5       | 4.8      | 0    | 6.9       | 0      | 11.7       | 0    | 0    | 0      | 0    | 0          |             |
| Vehicles              | 437      | 3036       | 0        | 0    | 3473       | 0    | 2871       | 238            | 0      | 3109       | 362      | 0    | 516       | 0      | 878        | 0    | 0    | 0      | 0    | 0          | 7460        |
| % Vehicles            | 96.7     | 95.6       | 0        | 0    | 95.7       | 0    | 96.4       | 97.5           | 0      | 96.5       | 96.5     | 0    | 96.4      | 0      | 96.5       | 0    | 0    | 0      | 0    | 0          | 96.1        |
| Trucks                | 1        | 27         | 0        | 0    | 28         | 0    | 28         | 1              | 0      | 29         | 0        | 0    | 1         | 0      | 1          | 0    | 0    | 0      | 0    | 0          | 58          |
| % Trucks              | 0.2      | 0.8        | 0        | 0    | 0.8        | 0    | 0.9        | 0.4            | 0      | 0.9        | 0        | 0    | 0.2       | 0      | 0.1        | 0    | 0    | 0      | 0    | 0          | 0.7         |
| Buses                 | 14       | 114        | 0        | 0    | 128        | 0    | 78         | 5              | 0      | 83         | 13       | 0    | 18        | 0      | 31         | 0    | 0    | 0      | 0    | 0          | 242         |
| % Buses               | 3.1      | 3.6        | 0        | 0    | 3.5        | 0    | 2.6        | 2              | 0      | 2.6        | 3.5      | 0    | 3.4       | 0      | 3.4        | 0    | 0    | 0      | 0    | 0          | 3.1         |
|                       |          |            | -        | -    |            | , ,  |            | _              | -      |            | ,        |      |           |        |            | -    | ,    |        | ·    | Ü          |             |

### **Greater Traffic Company**

File Name: 01

Site Code : 00000000 Start Date : 9/5/2018

|               |          | N Ind   | ian Cre  | eek Dr    |            |          | N Ind | ian Cr | eek Dr |            |      | India | n Cree | k Way |            |      |      |        |      |            |            |
|---------------|----------|---------|----------|-----------|------------|----------|-------|--------|--------|------------|------|-------|--------|-------|------------|------|------|--------|------|------------|------------|
|               |          | No      | rthbou   | nd        |            |          | So    | uthbou | nd     |            |      | E     | astbou | nd    |            |      | W    | estbou | nd   |            |            |
| Start Time    | Left     | Thru    | Right    | Peds      | App. Total | Left     | Thru  | Right  | Peds   | App. Total | Left | Thru  | Right  | Peds  | App. Total | Left | Thru | Right  | Peds | App. Total | Int. Total |
| Peak Hour Ana | alysis F | rom 07: | :00 AM   | to 08:4   | 5 AM - I   | Peak 1 o | f 1   |        |        |            |      |       |        |       |            |      |      |        |      |            |            |
| Peak Hour for | Entire I | ntersec | tion Beg | gins at C | 7:30 AN    | 1        |       |        |        |            |      |       |        |       |            |      |      |        |      |            |            |
| 07:30 AM      | 17       | 151     | 0        | 0         | 168        | 0        | 69    | 4      | 0      | 73         | 29   | 0     | 20     | 0     | 49         | 0    | 0    | 0      | 0    | 0          | 290        |
| 07:45 AM      | 14       | 142     | 0        | 0         | 156        | 0        | 103   | 7      | 0      | 110        | 15   | 0     | 28     | 0     | 43         | 0    | 0    | 0      | 0    | 0          | 309        |
| 08:00 AM      | 15       | 118     | 0        | 0         | 133        | 0        | 118   | 10     | 0      | 128        | 6    | 0     | 20     | 0     | 26         | 0    | 0    | 0      | 0    | 0          | 287        |
| 08:15 AM      | 9        | 131     | 0        | 0         | 140        | 0        | 109   | 9      | 0      | 118        | 3    | 0     | 16     | 0     | 19         | 0    | 0    | 0      | 0    | 0          | 277        |
| Total Volume  | 55       | 542     | 0        | 0         | 597        | 0        | 399   | 30     | 0      | 429        | 53   | 0     | 84     | 0     | 137        | 0    | 0    | 0      | 0    | 0          | 1163       |
| % App. Total  | 9.2      | 90.8    | 0        | 0         |            | 0        | 93    | 7      | 0      |            | 38.7 | 0     | 61.3   | 0     |            | 0    | 0    | 0      | 0    |            |            |
| PHF           | .809     | .897    | .000     | .000      | .888       | .000     | .845  | .750   | .000   | .838       | .457 | .000  | .750   | .000  | .699       | .000 | .000 | .000   | .000 | .000       | .941       |
| Vehicles      | 50       | 508     | 0        | 0         | 558        | 0        | 378   | 28     | 0      | 406        | 53   | 0     | 79     | 0     | 132        | 0    | 0    | 0      | 0    | 0          | 1096       |
| % Vehicles    |          |         |          |           |            |          |       |        |        |            |      |       |        |       |            |      |      |        |      |            |            |
| Trucks        | 0        | 9       | 0        | 0         | 9          | 0        | 1     | 0      | 0      | 1          | 0    | 0     | 0      | 0     | 0          | 0    | 0    | 0      | 0    | 0          | 10         |
| % Trucks      | 0        | 1.7     | 0        | 0         | 1.5        | 0        | 0.3   | 0      | 0      | 0.2        | 0    | 0     | 0      | 0     | 0          | 0    | 0    | 0      | 0    | 0          | 0.9        |
| Buses         | 5        | 25      | 0        | 0         | 30         | 0        | 20    | 2      | 0      | 22         | 0    | 0     | 5      | 0     | 5          | 0    | 0    | 0      | 0    | 0          | 57         |
| % Buses       | 9.1      | 4.6     | 0        | 0         | 5.0        | 0        | 5.0   | 6.7    | 0      | 5.1        | 0    | 0     | 6.0    | 0     | 3.6        | 0    | 0    | 0      | 0    | 0          | 4.9        |

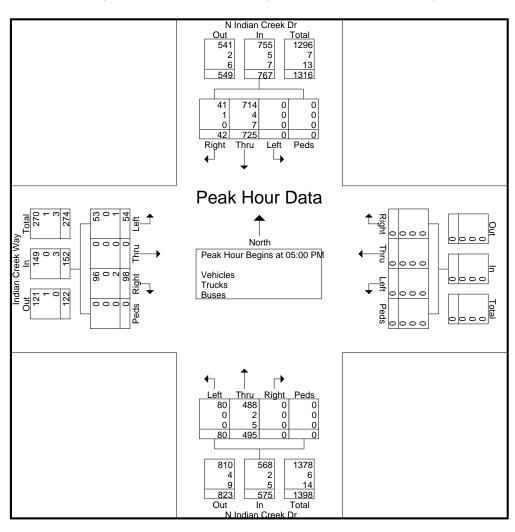


### **Greater Traffic Company**

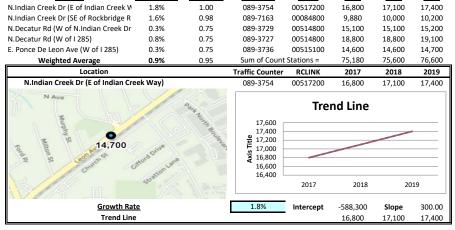
File Name: 01

Site Code : 00000000 Start Date : 9/5/2018

|               |          | N Ind    | ian Cr  | eek Dr    |            |          | N Ind | ian Cr | eek Dr |            |      | India | n Cree | k Way |            |      |      |        |      |            | ]          |
|---------------|----------|----------|---------|-----------|------------|----------|-------|--------|--------|------------|------|-------|--------|-------|------------|------|------|--------|------|------------|------------|
|               |          | No       | rthbou  | nd        |            |          | So    | uthbou | nd     |            |      | E     | astbou | nd    |            |      | W    | estbou | nd   |            |            |
| Start Time    | Left     | Thru     | Right   | Peds      | App. Total | Left     | Thru  | Right  | Peds   | App. Total | Left | Thru  | Right  | Peds  | App. Total | Left | Thru | Right  | Peds | App. Total | Int. Total |
| Peak Hour An  | alysis F | rom 04:  | 00 PM   | to 05:45  | 5 PM - P   | eak 1 of | f 1   |        |        |            |      |       |        |       |            |      |      |        |      |            |            |
| Peak Hour for | Entire 1 | Intersec | tion Be | gins at ( | )5:00 PM   | 1        |       |        |        |            |      |       |        |       |            |      |      |        |      |            |            |
| 05:00 PM      | 16       | 124      | 0       | 0         | 140        | 0        | 152   | 8      | 0      | 160        | 17   | 0     | 25     | 0     | 42         | 0    | 0    | 0      | 0    | 0          | 342        |
| 05:15 PM      | 17       | 137      | 0       | 0         | 154        | 0        | 169   | 14     | 0      | 183        | 13   | 0     | 25     | 0     | 38         | 0    | 0    | 0      | 0    | 0          | 375        |
| 05:30 PM      | 17       | 126      | 0       | 0         | 143        | 0        | 208   | 13     | 0      | 221        | 14   | 0     | 27     | 0     | 41         | 0    | 0    | 0      | 0    | 0          | 405        |
| 05:45 PM      | 30       | 108      | 0       | 0         | 138        | 0        | 196   | 7      | 0      | 203        | 10   | 0     | 21     | 0     | 31         | 0    | 0    | 0      | 0    | 0          | 372        |
| Total Volume  | 80       | 495      | 0       | 0         | 575        | 0        | 725   | 42     | 0      | 767        | 54   | 0     | 98     | 0     | 152        | 0    | 0    | 0      | 0    | 0          | 1494       |
| % App. Total  | 13.9     | 86.1     | 0       | 0         |            | 0        | 94.5  | 5.5    | 0      |            | 35.5 | 0     | 64.5   | 0     |            | 0    | 0    | 0      | 0    |            |            |
| PHF           | .667     | .903     | .000    | .000      | .933       | .000     | .871  | .750   | .000   | .868       | .794 | .000  | .907   | .000  | .905       | .000 | .000 | .000   | .000 | .000       | .922       |
| Vehicles      | 80       | 488      | 0       | 0         | 568        | 0        | 714   | 41     | 0      | 755        | 53   | 0     | 96     | 0     | 149        | 0    | 0    | 0      | 0    | 0          | 1472       |
| % Vehicles    |          |          |         |           |            |          |       |        |        |            |      |       |        |       |            |      |      |        |      |            |            |
| Trucks        | 0        | 2        | 0       | 0         | 2          | 0        | 4     | 1      | 0      | 5          | 0    | 0     | 0      | 0     | 0          | 0    | 0    | 0      | 0    | 0          | 7          |
| % Trucks      | 0        | 0.4      | 0       | 0         | 0.3        | 0        | 0.6   | 2.4    | 0      | 0.7        | 0    | 0     | 0      | 0     | 0          | 0    | 0    | 0      | 0    | 0          | 0.5        |
| Buses         | 0        | 5        | 0       | 0         | 5          | 0        | 7     | 0      | 0      | 7          | 1    | 0     | 2      | 0     | 3          | 0    | 0    | 0      | 0    | 0          | 15         |
| % Buses       | 0        | 1.0      | 0       | 0         | 0.9        | 0        | 1.0   | 0      | 0      | 0.9        | 1.9  | 0     | 2.0    | 0     | 2.0        | 0    | 0    | 0      | 0    | 0          | 1.0        |



| LINEAR | REGRESSION | OF DAILY | TRAFFIC |
|--------|------------|----------|---------|
|        |            |          |         |
|        |            |          |         |



Station ID

Route

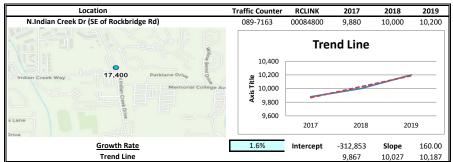
2017

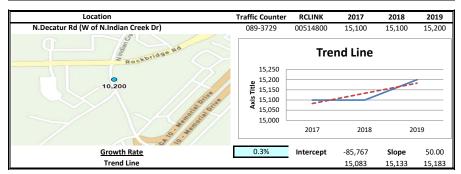
2018

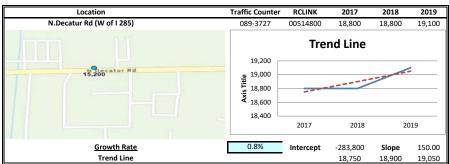
2019

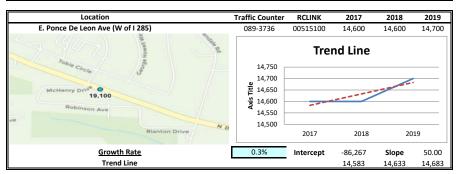
Growth Rate R Squared

Location









| EXISTING INTERSECTION A | ANALYSIS |
|-------------------------|----------|
|                         |          |
|                         |          |

| Intersection           |          |        |        |      |        |       |  |
|------------------------|----------|--------|--------|------|--------|-------|--|
| Int Delay, s/veh       | 3.7      |        |        |      |        |       |  |
| Movement               | EBT      | EBR    | WBL    | WBT  | NBL    | NBR   |  |
| Lane Configurations    | <u> </u> | 7      |        | 4    | ሻ      | 7     |  |
| Traffic Vol, veh/h     | 202      | 36     | 94     | 260  | 53     | 114   |  |
| Future Vol, veh/h      | 202      | 36     | 94     | 260  | 53     | 114   |  |
| Conflicting Peds, #/hr | 0        | 0      | 0      | 0    | 0      | 0     |  |
| Sign Control           | Free     | Free   | Free   | Free | Stop   | Stop  |  |
| RT Channelized         | -        | None   | -      |      | -      | None  |  |
| Storage Length         | _        | 270    | _      | -    | 0      | 30    |  |
| Veh in Median Storage  |          | 270    | _      | 0    | 0      | -     |  |
| Grade, %               | 0        | -      | -      | 0    | 0      | -     |  |
| Peak Hour Factor       | 90       | 90     | 90     | 90   | 90     | 90    |  |
|                        |          |        |        |      |        |       |  |
| Heavy Vehicles, %      | 2        | 2      | 2      | 2    | 2      | 2     |  |
| Mvmt Flow              | 224      | 40     | 104    | 289  | 59     | 127   |  |
|                        |          |        |        |      |        |       |  |
| Major/Minor            | Major1   | ſ      | Major2 | N    | Minor1 |       |  |
| Conflicting Flow All   | 0        | 0      | 264    | 0    | 721    | 224   |  |
| Stage 1                | -        | _      |        | _    | 224    |       |  |
| Stage 2                | -        | -      | -      | _    | 497    | -     |  |
| Critical Hdwy          | _        | _      | 4.12   | _    | 6.42   | 6.22  |  |
| Critical Hdwy Stg 1    |          | _      |        | _    | 5.42   | -     |  |
| Critical Hdwy Stg 2    | _        | _      | _      | _    | 5.42   | _     |  |
| Follow-up Hdwy         | _        | _      | 2.218  |      | 3.518  | 3 318 |  |
| Pot Cap-1 Maneuver     | _        |        | 1300   | _    | 394    | 815   |  |
| Stage 1                | -        | -      | 1300   | -    | 813    | 010   |  |
| Stage 2                |          |        |        |      | 611    | -     |  |
| Platoon blocked, %     | -        | -      | -      | -    | 011    | -     |  |
|                        | -        | -      | 1200   | -    | 257    | 015   |  |
| Mov Cap-1 Maneuver     | -        | -      | 1300   | -    | 357    | 815   |  |
| Mov Cap-2 Maneuver     | -        | -      | -      | -    | 357    | -     |  |
| Stage 1                | -        | -      | -      | -    | 813    | -     |  |
| Stage 2                | -        | -      | -      | -    | 553    | -     |  |
|                        |          |        |        |      |        |       |  |
| Approach               | EB       |        | WB     |      | NB     |       |  |
| HCM Control Delay, s   | 0        |        | 2.1    |      | 12.4   |       |  |
| HCM LOS                | U        |        | ۷. ۱   |      | В      |       |  |
| HOW LOS                |          |        |        |      | D      |       |  |
|                        |          |        |        |      |        |       |  |
| Minor Lane/Major Mvn   | nt I     | NBLn11 | VBLn2  | EBT  | EBR    | WBL   |  |
| Capacity (veh/h)       |          | 357    | 815    | -    | -      | 1300  |  |
| HCM Lane V/C Ratio     |          | 0.165  | 0.155  | -    | -      | 0.08  |  |
| HCM Control Delay (s)  |          | 17.1   | 10.2   | -    | -      | 8     |  |
| HCM Lane LOS           |          | С      | В      | -    | -      | Α     |  |
| HCM 95th %tile Q(veh   | )        | 0.6    | 0.5    | -    | -      | 0.3   |  |
|                        |          |        |        |      |        |       |  |

| Intersection            |        |       |          |          |        |      |
|-------------------------|--------|-------|----------|----------|--------|------|
| Int Delay, s/veh        | 4.4    |       |          |          |        |      |
| Movement                | WBL    | WBR   | NBT      | NBR      | SBL    | SBT  |
| Lane Configurations     | ¥      | WBIX  | <b>1</b> | NDIX     | ODL    | 4    |
| Traffic Vol, veh/h      | 22     | 61    | 75       | 14       | 68     | 63   |
| Future Vol, veh/h       | 22     | 61    | 75       | 14       | 68     | 63   |
| Conflicting Peds, #/hr  | 0      | 0     | 0        | 0        | 0      | 0    |
| Sign Control            | Stop   | Stop  | Free     | Free     | Free   | Free |
| RT Channelized          |        | None  | -        | None     | -      | None |
| Storage Length          | 0      | -     | -        | -        | -      | -    |
| Veh in Median Storage   |        | -     | 0        | -        | -      | 0    |
| Grade, %                | 0      | _     | 0        | _        | _      | 0    |
| Peak Hour Factor        | 86     | 86    | 86       | 86       | 86     | 86   |
| Heavy Vehicles, %       | 2      | 2     | 2        | 2        | 2      | 2    |
| Mvmt Flow               | 26     | 71    | 87       | 16       | 79     | 73   |
| WWW. Tiow               | 20     | 7.1   | 01       | 10       | , ,    | 70   |
|                         |        |       |          |          |        |      |
|                         | Minor1 |       | Major1   |          | Major2 |      |
| Conflicting Flow All    | 326    | 95    | 0        | 0        | 103    | 0    |
| Stage 1                 | 95     | -     | -        | -        | -      | -    |
| Stage 2                 | 231    | -     | -        | -        | -      | -    |
| Critical Hdwy           | 6.42   | 6.22  | -        | -        | 4.12   | -    |
| Critical Hdwy Stg 1     | 5.42   | -     | -        | -        | -      | -    |
| Critical Hdwy Stg 2     | 5.42   | -     | -        | -        | -      | -    |
| Follow-up Hdwy          | 3.518  | 3.318 | -        | -        | 2.218  | -    |
| Pot Cap-1 Maneuver      | 668    | 962   | -        | -        | 1489   | -    |
| Stage 1                 | 929    | -     | -        | -        | -      | -    |
| Stage 2                 | 807    | -     | -        | -        | -      | -    |
| Platoon blocked, %      |        |       | -        | -        |        | -    |
| Mov Cap-1 Maneuver      | 631    | 962   | -        | -        | 1489   | -    |
| Mov Cap-2 Maneuver      | 631    | -     | -        | -        | -      | -    |
| Stage 1                 | 929    | -     | -        | _        | -      | -    |
| Stage 2                 | 763    | -     | -        | -        | -      | _    |
| - · · g                 |        |       |          |          |        |      |
|                         | 10.00  |       |          |          |        |      |
| Approach                | WB     |       | NB       |          | SB     |      |
| HCM Control Delay, s    | 9.8    |       | 0        |          | 3.9    |      |
| HCM LOS                 | Α      |       |          |          |        |      |
|                         |        |       |          |          |        |      |
| Minor Lane/Major Mvm    | t      | NBT   | NBRV     | WBLn1    | SBL    | SBT  |
| Capacity (veh/h)        |        | -     | -        | 845      | 1489   | -    |
| HCM Lane V/C Ratio      |        | -     |          | 0.114    |        | _    |
| HCM Control Delay (s)   |        | _     | _        | 9.8      | 7.6    | 0    |
| HCM Lane LOS            |        | _     | _        | 7.0<br>A | Α.     | A    |
| HCM 95th %tile Q(veh)   |        | _     | _        | 0.4      | 0.2    |      |
| 115W1 75W1 76W1C Q(VCH) |        |       |          | 0.7      | 0.2    |      |

|                         | •     | -        | •          | -     |
|-------------------------|-------|----------|------------|-------|
| Lane Group              | EBL   | EBT      | WBT        | SBL   |
| Lane Configurations     | ሻ     | <b>^</b> | <b>↑</b> ↑ | W     |
| Traffic Volume (vph)    | 43    | 321      | 700        | 24    |
| Future Volume (vph)     | 43    | 321      | 700        | 24    |
| Lane Group Flow (vph)   | 46    | 341      | 761        | 145   |
| Turn Type               | Perm  | NA       | NA         | Prot  |
| Protected Phases        |       | 2        | 6          | 4     |
| Permitted Phases        | 2     |          |            |       |
| Detector Phase          | 2     | 2        | 6          | 4     |
| Switch Phase            |       |          |            |       |
| Minimum Initial (s)     | 15.0  | 15.0     | 15.0       | 6.0   |
| Minimum Split (s)       | 23.5  | 23.5     | 23.5       | 30.5  |
| Total Split (s)         | 71.4  | 71.4     | 71.4       | 48.6  |
| Total Split (%)         | 59.5% | 59.5%    | 59.5%      | 40.5% |
| Yellow Time (s)         | 3.5   | 3.5      | 3.5        | 3.5   |
| All-Red Time (s)        | 2.0   | 2.0      | 2.0        | 2.0   |
| Lost Time Adjust (s)    | 0.0   | 0.0      | 0.0        | 0.0   |
| Total Lost Time (s)     | 5.5   | 5.5      | 5.5        | 5.5   |
| Lead/Lag                |       |          |            |       |
| Lead-Lag Optimize?      |       |          |            |       |
| Recall Mode             | C-Min | C-Min    | C-Min      | None  |
| v/c Ratio               | 0.08  | 0.12     | 0.26       | 0.63  |
| Control Delay           | 2.6   | 2.1      | 2.5        | 26.4  |
| Queue Delay             | 0.0   | 0.0      | 0.0        | 0.0   |
| Total Delay             | 2.6   | 2.1      | 2.5        | 26.4  |
| Queue Length 50th (ft)  | 4     | 17       | 44         | 20    |
| Queue Length 95th (ft)  | 15    | 36       | 85         | 82    |
| Internal Link Dist (ft) |       | 586      | 931        | 454   |
| Turn Bay Length (ft)    | 100   |          |            |       |
| Base Capacity (vph)     | 562   | 2955     | 2947       | 665   |
| Starvation Cap Reductn  | 0     | 0        | 0          | 0     |
| Spillback Cap Reductn   | 0     | 0        | 0          | 0     |
| Storage Cap Reductn     | 0     | 0        | 0          | 0     |
| Reduced v/c Ratio       | 0.08  | 0.12     | 0.26       | 0.22  |
|                         |       |          |            |       |

### Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Splits and Phases: 3: N. Decatur Rd & Northern Ave



|                              | ۶    | <b>→</b> | <b>←</b>   | •    | <b>&gt;</b> | 4     |
|------------------------------|------|----------|------------|------|-------------|-------|
| Movement                     | EBL  | EBT      | WBT        | WBR  | SBL         | SBR   |
| Lane Configurations          | ሻ    | <b>^</b> | <b>↑</b> ↑ |      | W           |       |
| Traffic Volume (veh/h)       | 43   | 321      | 700        | 15   | 24          | 112   |
| Future Volume (veh/h)        | 43   | 321      | 700        | 15   | 24          | 112   |
| Initial Q (Qb), veh          | 0    | 0        | 0          | 0    | 0           | 0     |
| Ped-Bike Adj(A_pbT)          | 1.00 |          |            | 1.00 | 1.00        | 1.00  |
| Parking Bus, Adj             | 1.00 | 1.00     | 1.00       | 1.00 | 1.00        | 1.00  |
| Work Zone On Approach        |      | No       | No         |      | No          |       |
| Adj Sat Flow, veh/h/ln       | 1870 | 1870     | 1870       | 1870 | 1870        | 1870  |
| Adj Flow Rate, veh/h         | 46   | 341      | 745        | 16   | 26          | 0     |
| Peak Hour Factor             | 0.94 | 0.94     | 0.94       | 0.94 | 0.94        | 0.94  |
| Percent Heavy Veh, %         | 2    | 2        | 2          | 2    | 2           | 2     |
| Cap, veh/h                   | 657  | 3122     | 3125       | 67   | 51          |       |
| Arrive On Green              | 0.88 | 0.88     | 0.88       | 0.88 | 0.03        | 0.00  |
| Sat Flow, veh/h              | 705  | 3647     | 3651       | 76   | 1718        | 0.00  |
|                              |      |          |            |      |             |       |
| Grp Volume(v), veh/h         | 46   | 341      | 372        | 389  | 27          | 0     |
| Grp Sat Flow(s), veh/h/ln    | 705  | 1777     | 1777       | 1857 | 1784        | 0     |
| Q Serve(g_s), s              | 1.3  | 1.5      | 3.9        | 3.9  | 1.8         | 0.0   |
| Cycle Q Clear(g_c), s        | 5.1  | 1.5      | 3.9        | 3.9  | 1.8         | 0.0   |
| Prop In Lane                 | 1.00 |          |            | 0.04 | 0.96        | 0.00  |
| Lane Grp Cap(c), veh/h       | 657  | 3122     | 1561       | 1631 | 53          |       |
| V/C Ratio(X)                 | 0.07 | 0.11     | 0.24       | 0.24 | 0.51        |       |
| Avail Cap(c_a), veh/h        | 657  | 3122     | 1561       | 1631 | 641         |       |
| HCM Platoon Ratio            | 1.00 | 1.00     | 1.00       | 1.00 | 1.00        | 1.00  |
| Upstream Filter(I)           | 1.00 | 1.00     | 1.00       | 1.00 | 1.00        | 0.00  |
| Uniform Delay (d), s/veh     | 1.5  | 1.0      | 1.1        | 1.1  | 57.4        | 0.0   |
| Incr Delay (d2), s/veh       | 0.2  | 0.1      | 0.4        | 0.3  | 7.4         | 0.0   |
| Initial Q Delay(d3),s/veh    | 0.0  | 0.0      | 0.0        | 0.0  | 0.0         | 0.0   |
| %ile BackOfQ(50%),veh/ln     | 0.1  | 0.2      | 0.5        | 0.5  | 0.9         | 0.0   |
| Unsig. Movement Delay, s/veh |      |          |            |      |             |       |
| LnGrp Delay(d),s/veh         | 1.7  | 1.0      | 1.5        | 1.5  | 64.8        | 0.0   |
| LnGrp LOS                    | Α    | A        | A          | A    | E           | 3.0   |
| Approach Vol, veh/h          |      | 387      | 761        | ,,   | 27          | А     |
| Approach Delay, s/veh        |      | 1.1      | 1.5        |      | 64.8        | А     |
| Approach LOS                 |      | Α        | 1.5<br>A   |      | 04.0<br>E   |       |
| Approach LOS                 |      | А        | А          |      | E           |       |
| Timer - Assigned Phs         |      | 2        |            | 4    |             | 6     |
| Phs Duration (G+Y+Rc), s     |      | 110.9    |            | 9.1  |             | 110.9 |
| Change Period (Y+Rc), s      |      | 5.5      |            | 5.5  |             | 5.5   |
| Max Green Setting (Gmax), s  |      | 65.9     |            | 43.1 |             | 65.9  |
| Max Q Clear Time (g_c+l1), s |      | 7.1      |            | 3.8  |             | 5.9   |
| Green Ext Time (p_c), s      |      | 5.4      |            | 0.1  |             | 11.4  |
| •                            |      | 3.1      |            | 3.1  |             |       |
| Intersection Summary         |      |          |            |      |             |       |
| HCM 6th Ctrl Delay           |      |          | 2.8        |      |             |       |
| HCM 6th LOS                  |      |          | А          |      |             |       |
| Notos                        |      |          |            |      |             |       |

User approved volume balancing among the lanes for turning movement.
Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

|                                | •     | 4     | <b>†</b> | <b>↓</b> |
|--------------------------------|-------|-------|----------|----------|
| Lane Group                     | EBL   | NBL   | NBT      | SBT      |
| Lane Configurations            | ¥     | ሻ     | <b></b>  | <b>f</b> |
| Traffic Volume (vph)           | 68    | 96    | 580      | 263      |
| Future Volume (vph)            | 68    | 96    | 580      | 263      |
| Lane Group Flow (vph)          | 147   | 100   | 604      | 417      |
| Turn Type                      | Prot  | Perm  | NA       | NA       |
| Protected Phases               | 4     |       | 2        | 6        |
| Permitted Phases               |       | 2     |          |          |
| Detector Phase                 | 4     | 2     | 2        | 6        |
| Switch Phase                   |       |       |          |          |
| Minimum Initial (s)            | 6.0   | 15.0  | 15.0     | 15.0     |
| Minimum Split (s)              | 61.5  | 57.5  | 57.5     | 23.5     |
| Total Split (s)                | 61.5  | 58.5  | 58.5     | 58.5     |
| Total Split (%)                | 51.3% | 48.8% | 48.8%    | 48.8%    |
| Yellow Time (s)                | 3.5   | 3.5   | 3.5      | 3.5      |
| All-Red Time (s)               | 2.0   | 2.0   | 2.0      | 2.0      |
| Lost Time Adjust (s)           | 0.0   | 0.0   | 0.0      | 0.0      |
| Total Lost Time (s)            | 5.5   | 5.5   | 5.5      | 5.5      |
| Lead/Lag                       | ე.ე   | ე.ე   | ე.ე      | ე.ე      |
|                                |       |       |          |          |
| Lead-Lag Optimize? Recall Mode | None  | C Min | C Min    | C Min    |
|                                | None  | C-Min | C-Min    | C-Min    |
| v/c Ratio                      | 0.66  | 0.13  | 0.40     | 0.29     |
| Control Delay                  | 43.9  | 3.4   | 4.6      | 3.6      |
| Queue Delay                    | 0.0   | 0.0   | 0.0      | 0.0      |
| Total Delay                    | 43.9  | 3.4   | 4.6      | 3.6      |
| Queue Length 50th (ft)         | 64    | 13    | 105      | 58       |
| Queue Length 95th (ft)         | 128   | 34    | 201      | 118      |
| Internal Link Dist (ft)        | 1475  |       | 446      | 669      |
| Turn Bay Length (ft)           |       | 50    |          |          |
| Base Capacity (vph)            | 821   | 769   | 1506     | 1442     |
| Starvation Cap Reductn         | 0     | 0     | 0        | 0        |
| Spillback Cap Reductn          | 0     | 0     | 0        | 0        |
| Storage Cap Reductn            | 0     | 0     | 0        | 0        |
| Reduced v/c Ratio              | 0.18  | 0.13  | 0.40     | 0.29     |
|                                |       |       |          |          |
| Intersection Summary           |       |       |          |          |
| Cycle Lenath: 120              |       |       |          |          |

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 120

Control Type: Actuated-Coordinated

Splits and Phases: 4: N.Indian Creek Dr & Indian Creek Way



|                              | ۶    | •     | •         | <b>†</b> | ļ    | 4     |
|------------------------------|------|-------|-----------|----------|------|-------|
| Movement                     | EBL  | EBR   | NBL       | NBT      | SBT  | SBR   |
| Lane Configurations          | W    |       | ች         | <b>^</b> | 1    |       |
| Traffic Volume (veh/h)       | 68   | 73    | 96        | 580      | 263  | 137   |
| Future Volume (veh/h)        | 68   | 73    | 96        | 580      | 263  | 137   |
| Initial Q (Qb), veh          | 0    | 0     | 0         | 0        | 0    | 0     |
| Ped-Bike Adj(A_pbT)          | 1.00 | 1.00  | 1.00      |          |      | 1.00  |
| Parking Bus, Adj             | 1.00 | 1.00  | 1.00      | 1.00     | 1.00 | 1.00  |
| Work Zone On Approach        | No   |       |           | No       | No   |       |
| Adj Sat Flow, veh/h/ln       | 1870 | 1870  | 1870      | 1870     | 1870 | 1870  |
| Adj Flow Rate, veh/h         | 71   | 76    | 100       | 604      | 274  | 143   |
| Peak Hour Factor             | 0.96 | 0.96  | 0.96      | 0.96     | 0.96 | 0.96  |
| Percent Heavy Veh, %         | 2    | 2     | 2         | 2        | 2    | 2     |
| Cap, veh/h                   | 86   | 92    | 778       | 1500     | 928  | 484   |
| Arrive On Green              | 0.11 | 0.11  | 0.80      | 0.80     | 0.80 | 0.80  |
| Sat Flow, veh/h              | 804  | 860   | 969       | 1870     | 1158 | 604   |
|                              | 148  |       | 100       | 604      | 0    | 417   |
| Grp Volume(v), veh/h         |      | 0     |           |          |      |       |
| Grp Sat Flow(s), veh/h/ln    | 1675 | 0     | 969       | 1870     | 0    | 1762  |
| Q Serve(g_s), s              | 10.4 | 0.0   | 3.6       | 11.3     | 0.0  | 7.4   |
| Cycle Q Clear(g_c), s        | 10.4 | 0.0   | 11.0      | 11.3     | 0.0  | 7.4   |
| Prop In Lane                 | 0.48 | 0.51  | 1.00      | 4500     |      | 0.34  |
| Lane Grp Cap(c), veh/h       | 178  | 0     | 778       | 1500     | 0    | 1413  |
| V/C Ratio(X)                 | 0.83 | 0.00  | 0.13      | 0.40     | 0.00 | 0.30  |
| Avail Cap(c_a), veh/h        | 782  | 0     | 778       | 1500     | 0    | 1413  |
| HCM Platoon Ratio            | 1.00 | 1.00  | 1.00      | 1.00     | 1.00 | 1.00  |
| Upstream Filter(I)           | 1.00 | 0.00  | 1.00      | 1.00     | 0.00 | 1.00  |
| Uniform Delay (d), s/veh     | 52.5 | 0.0   | 4.5       | 3.5      | 0.0  | 3.1   |
| Incr Delay (d2), s/veh       | 9.5  | 0.0   | 0.3       | 0.8      | 0.0  | 0.5   |
| Initial Q Delay(d3),s/veh    | 0.0  | 0.0   | 0.0       | 0.0      | 0.0  | 0.0   |
| %ile BackOfQ(50%),veh/ln     | 4.9  | 0.0   | 0.7       | 3.5      | 0.0  | 2.1   |
| Unsig. Movement Delay, s/veh | 1    |       |           |          |      |       |
| LnGrp Delay(d),s/veh         | 62.0 | 0.0   | 4.8       | 4.3      | 0.0  | 3.6   |
| LnGrp LOS                    | Е    | Α     | Α         | Α        | Α    | Α     |
| Approach Vol, veh/h          | 148  |       |           | 704      | 417  |       |
| Approach Delay, s/veh        | 62.0 |       |           | 4.4      | 3.6  |       |
| Approach LOS                 | E    |       |           | A        | A    |       |
|                              |      |       |           |          | , ,  |       |
| Timer - Assigned Phs         |      | 2     |           | 4        |      | 6     |
| Phs Duration (G+Y+Rc), s     |      | 101.7 |           | 18.3     |      | 101.7 |
| Change Period (Y+Rc), s      |      | 5.5   |           | 5.5      |      | 5.5   |
| Max Green Setting (Gmax), s  |      | 53.0  |           | 56.0     |      | 53.0  |
| Max Q Clear Time (g_c+I1), s |      | 13.3  |           | 12.4     |      | 9.4   |
| Green Ext Time (p_c), s      |      | 5.1   |           | 0.5      |      | 2.9   |
| Intersection Summary         |      |       |           |          |      |       |
| HCM 6th Ctrl Delay           |      |       | 10.8      |          |      |       |
| HCM 6th LOS                  |      |       | 10.6<br>B |          |      |       |
| HOW OUT LOS                  |      |       | Ď         |          |      |       |
| Notes                        |      |       |           |          |      |       |

User approved volume balancing among the lanes for turning movement.

| Intersection           |          |        |        |          |           |       |
|------------------------|----------|--------|--------|----------|-----------|-------|
| Int Delay, s/veh       | 3        |        |        |          |           |       |
| Movement               | EBT      | EBR    | WBL    | WBT      | NBL       | NBR   |
| Lane Configurations    | <u> </u> | 7      | VVDL   | <u>₩</u> | <u> </u>  | T T   |
| Traffic Vol, veh/h     | 252      | 86     | 151    | 294      | 20        | 90    |
| Future Vol, veh/h      | 252      | 86     | 151    | 294      | 20        | 90    |
|                        |          | 00     |        |          |           |       |
| Conflicting Peds, #/hr | 0        |        | 0      | 0        | 0         | 0     |
| Sign Control           | Free     | Free   | Free   | Free     | Stop      | Stop  |
| RT Channelized         | -        | None   | -      | None     | -         | None  |
| Storage Length         | -        | 270    | -      | -        | 0         | 30    |
| Veh in Median Storage  |          | -      | -      | 0        | 0         | -     |
| Grade, %               | 0        | -      | -      | 0        | 0         | -     |
| Peak Hour Factor       | 91       | 91     | 91     | 91       | 91        | 91    |
| Heavy Vehicles, %      | 2        | 2      | 2      | 2        | 2         | 2     |
| Mvmt Flow              | 277      | 95     | 166    | 323      | 22        | 99    |
|                        | _,,      | , 0    |        | 020      |           | • •   |
| Major/Mina             | Molant   |        | Molera | n        | Ninc-1    |       |
|                        | Major1   |        | Major2 |          | Minor1    |       |
| Conflicting Flow All   | 0        | 0      | 372    | 0        | 932       | 277   |
| Stage 1                | -        | -      | -      | -        | 277       | -     |
| Stage 2                | -        | -      | -      | -        | 655       | -     |
| Critical Hdwy          | -        | -      | 4.12   | -        | 6.42      | 6.22  |
| Critical Hdwy Stg 1    | -        | -      | -      | -        | 5.42      | -     |
| Critical Hdwy Stg 2    | -        | -      | -      | -        | 5.42      | -     |
| Follow-up Hdwy         | -        | -      | 2.218  | -        | 3.518     | 3.318 |
| Pot Cap-1 Maneuver     | -        | -      | 1186   | -        | 296       | 762   |
| Stage 1                | _        | _      | _      | _        | 770       | _     |
| Stage 2                | _        | _      | _      | -        | 517       | _     |
| Platoon blocked, %     | _        | _      |        | _        | 317       |       |
| Mov Cap-1 Maneuver     | _        | _      | 1186   | _        | 245       | 762   |
|                        |          | -      | 1100   |          |           |       |
| Mov Cap-2 Maneuver     | -        | -      | -      | -        | 245       | -     |
| Stage 1                | -        | -      | -      | -        | 770       | -     |
| Stage 2                | -        | -      | -      | -        | 429       | -     |
|                        |          |        |        |          |           |       |
| Approach               | EB       |        | WB     |          | NB        |       |
| HCM Control Delay, s   |          |        | 2.9    |          | 12.3      |       |
| HCM LOS                | U        |        | Z.7    |          | 12.3<br>B |       |
| HOWI LUS               |          |        |        |          | D         |       |
|                        |          |        |        |          |           |       |
| Minor Lane/Major Mvr   | nt N     | NBLn11 | VBLn2  | EBT      | EBR       | WBL   |
| Capacity (veh/h)       |          | 245    | 762    | _        | -         | 1186  |
| HCM Lane V/C Ratio     |          | 0.09   | 0.13   | _        | -         | 0.14  |
| HCM Control Delay (s   | )        | 21.1   | 10.4   | _        | _         | 8.5   |
| HCM Lane LOS           | ,        | C C    | В      | -        | -         | Α     |
| HCM 95th %tile Q(veh   | 1)       | 0.3    | 0.4    | -        | -         | 0.5   |
| HOW FOUT JOINE Q(VEI   | 1)       | 0.3    | 0.4    | -        | -         | 0.5   |
|                        |          |        |        |          |           |       |

| Intersection  |            |             |          |                      |                      |             |
|---|------------|-------------|----------|----------------------|----------------------|-------------|
| Int Delay, s/veh  | 3.7        |             |          |                      |                      |             |
| Movement  |            | WDD         | NDT      | NDD                  | CDI                  | CDT         |
|   | WBL        | WBR         | NBT      | NBR                  | SBL                  | SBT         |
| Lane Configurations   | 70         | Γ/          | <b>₽</b> | 2/                   | 71                   | 4<br>120    |
| Traffic Vol, veh/h  | 29         | 56          | 66       | 26                   | 74                   | 138         |
| Future Vol, veh/h   | 29         | 56          | 66       | 26                   | 74                   | 138         |
| Conflicting Peds, #/hr  | 0          | 0           | 0        | 0                    | 0                    | 0           |
| Sign Control  | Stop       | Stop        | Free     | Free                 | Free                 | Free        |
| RT Channelized  | -          | None        | -        |                      | -                    | None        |
| Storage Length  | 0          | -           | -        | -                    | -                    | -           |
| Veh in Median Storage   |            | -           | 0        | -                    | -                    | 0           |
| Grade, %  | 0          | -           | 0        | -                    | -                    | 0           |
| Peak Hour Factor  | 86         | 86          | 86       | 86                   | 86                   | 86          |
| Heavy Vehicles, %   | 2          | 2           | 2        | 2                    | 2                    | 2           |
| Mvmt Flow   | 34         | 65          | 77       | 30                   | 86                   | 160         |
|   |            |             |          |                      |                      |             |
| Major/Minor I   | Minor1     | N           | Major1   |                      | Major2               |             |
| Conflicting Flow All  | 424        | 92          | 0        | 0                    | 107                  | 0           |
| Stage 1   | 92         | -           | -        | -                    | -                    | -           |
| Stage 2   | 332        | _           | _        | _                    | _                    | _           |
| Critical Hdwy   | 6.42       | 6.22        |          | _                    | 4.12                 | _           |
| Critical Hdwy Stg 1   | 5.42       | 0.22        | _        |                      | 4.12                 |             |
| Critical Hdwy Stg 2   | 5.42       |             | -        | -                    | -                    | -           |
| Follow-up Hdwy  | 3.518      | 3.318       | -        | -                    | 2.218                | -           |
|   |            | 965         | -        |                      | 1484                 |             |
| Pot Cap-1 Maneuver  | 587<br>932 |             | -        | -                    | 1404                 | -           |
| Stage 1   |            | -           | -        | -                    | -                    | -           |
| Stage 2   | 727        | -           | -        | -                    | -                    | -           |
| Platoon blocked, %  | F 40       | 0/5         | -        | -                    | 1404                 | -           |
| Mov Cap-1 Maneuver  | 549        | 965         | -        | -                    | 1484                 | -           |
| Mov Cap-2 Maneuver  | 549        | -           | -        | -                    | -                    | -           |
| Stage 1   | 932        | -           | -        | -                    | -                    | -           |
| Stage 2   | 680        | -           | -        | -                    | -                    | -           |
|   |            |             |          |                      |                      |             |
| Approach  | WB         |             | NB       |                      | SB                   |             |
| HCM Control Delay, s  | 10.4       |             | 0        |                      | 2.6                  |             |
|   | В          |             | U        |                      | 2.0                  |             |
| HCMTOS  | D          |             |          |                      |                      |             |
| HCM LOS   |            |             |          |                      |                      |             |
|   |            |             |          |                      |                      | CDT         |
| Minor Lane/Major Mvm  | nt         | NBT         | NBR\     | WBLn1                | SBL                  | SBT         |
|   | nt         | NBT<br>-    |          |                      | SBL<br>1484          | -<br>2R1    |
| Minor Lane/Major Mvm  | nt         |             | -        |                      | 1484                 |             |
| Minor Lane/Major Mvm<br>Capacity (veh/h)  |            | -           | -        | 767<br>0.129         | 1484                 | -           |
| Minor Lane/Major Mvm<br>Capacity (veh/h)<br>HCM Lane V/C Ratio                          |            | -           | -        | 767<br>0.129         | 1484<br>0.058        | -           |
| Minor Lane/Major Mvm<br>Capacity (veh/h)<br>HCM Lane V/C Ratio<br>HCM Control Delay (s) | )          | -<br>-<br>- | -        | 767<br>0.129<br>10.4 | 1484<br>0.058<br>7.6 | -<br>-<br>0 |

|                         | ۶     | -        | ←          | <b>&gt;</b> |
|-------------------------|-------|----------|------------|-------------|
| Lane Group              | EBL   | EBT      | WBT        | SBL         |
| Lane Configurations     | *     | <b>^</b> | <b>↑</b> ↑ | ¥           |
| Traffic Volume (vph)    | 71    | 877      | 389        | 68          |
| Future Volume (vph)     | 71    | 877      | 389        | 68          |
| Lane Group Flow (vph)   | 75    | 923      | 444        | 172         |
| Turn Type               | Perm  | NA       | NA         | Prot        |
| Protected Phases        |       | 2        | 6          | 4           |
| Permitted Phases        | 2     |          |            |             |
| Detector Phase          | 2     | 2        | 6          | 4           |
| Switch Phase            |       |          |            |             |
| Minimum Initial (s)     | 15.0  | 15.0     | 15.0       | 6.0         |
| Minimum Split (s)       | 23.5  | 23.5     | 23.5       | 30.5        |
| Total Split (s)         | 71.4  | 71.4     | 71.4       | 48.6        |
| Total Split (%)         | 59.5% | 59.5%    | 59.5%      | 40.5%       |
| Yellow Time (s)         | 3.5   | 3.5      | 3.5        | 3.5         |
| All-Red Time (s)        | 2.0   | 2.0      | 2.0        | 2.0         |
| Lost Time Adjust (s)    | 0.0   | 0.0      | 0.0        | 0.0         |
| Total Lost Time (s)     | 5.5   | 5.5      | 5.5        | 5.5         |
| Lead/Lag                |       |          |            |             |
| Lead-Lag Optimize?      |       |          |            |             |
| Recall Mode             | C-Min | C-Min    | C-Min      | None        |
| v/c Ratio               | 0.10  | 0.33     | 0.16       | 0.70        |
| Control Delay           | 3.8   | 4.1      | 3.3        | 46.3        |
| Queue Delay             | 0.0   | 0.0      | 0.0        | 0.0         |
| Total Delay             | 3.8   | 4.1      | 3.3        | 46.3        |
| Queue Length 50th (ft)  | 10    | 83       | 33         | 80          |
| Queue Length 95th (ft)  | 29    | 143      | 61         | 148         |
| Internal Link Dist (ft) |       | 586      | 931        | 454         |
| Turn Bay Length (ft)    | 100   |          |            |             |
| Base Capacity (vph)     | 734   | 2820     | 2789       | 645         |
| Starvation Cap Reductn  | 0     | 0        | 0          | 0           |
| Spillback Cap Reductn   | 0     | 0        | 0          | 0           |
| Storage Cap Reductn     | 0     | 0        | 0          | 0           |
| Reduced v/c Ratio       | 0.10  | 0.33     | 0.16       | 0.27        |
| Intersection Summary    |       |          |            |             |

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Splits and Phases: 3: N. Decatur Rd & Northern Ave



|                              | ۶    | <b>→</b> | <b>←</b> | •    | <b>&gt;</b> | 4     |
|------------------------------|------|----------|----------|------|-------------|-------|
| Movement                     | EBL  | EBT      | WBT      | WBR  | SBL         | SBR   |
| Lane Configurations          | ሻ    | <b>^</b> | ħβ       |      | ¥           |       |
| Traffic Volume (veh/h)       | 71   | 877      | 389      | 33   | 68          | 95    |
| Future Volume (veh/h)        | 71   | 877      | 389      | 33   | 68          | 95    |
| Initial Q (Qb), veh          | 0    | 0        | 0        | 0    | 0           | 0     |
| Ped-Bike Adj(A_pbT)          | 1.00 |          |          | 1.00 | 1.00        | 1.00  |
| Parking Bus, Adj             | 1.00 | 1.00     | 1.00     | 1.00 | 1.00        | 1.00  |
| Work Zone On Approach        |      | No       | No       |      | No          |       |
| Adj Sat Flow, veh/h/ln       | 1870 | 1870     | 1870     | 1870 | 1870        | 1870  |
| Adj Flow Rate, veh/h         | 75   | 923      | 409      | 35   | 72          | 0     |
| Peak Hour Factor             | 0.95 | 0.95     | 0.95     | 0.95 | 0.95        | 0.95  |
| Percent Heavy Veh, %         | 2    | 2        | 2        | 2    | 2           | 2     |
| Cap, veh/h                   | 849  | 3038     | 2833     | 241  | 94          |       |
| Arrive On Green              | 0.85 | 0.85     | 0.85     | 0.85 | 0.05        | 0.00  |
| Sat Flow, veh/h              | 946  | 3647     | 3408     | 282  | 1758        | 0.00  |
|                              |      |          |          |      |             |       |
| Grp Volume(v), veh/h         | 75   | 923      | 218      | 226  | 73          | 0     |
| Grp Sat Flow(s), veh/h/ln    | 946  | 1777     | 1777     | 1820 | 1782        | 0     |
| Q Serve(g_s), s              | 1.7  | 6.1      | 2.4      | 2.5  | 4.9         | 0.0   |
| Cycle Q Clear(g_c), s        | 4.2  | 6.1      | 2.4      | 2.5  | 4.9         | 0.0   |
| Prop In Lane                 | 1.00 | 0000     | 4510     | 0.16 | 0.99        | 0.00  |
| Lane Grp Cap(c), veh/h       | 849  | 3038     | 1519     | 1555 | 95          |       |
| V/C Ratio(X)                 | 0.09 | 0.30     | 0.14     | 0.14 | 0.77        |       |
| Avail Cap(c_a), veh/h        | 849  | 3038     | 1519     | 1555 | 640         |       |
| HCM Platoon Ratio            | 1.00 | 1.00     | 1.00     | 1.00 | 1.00        | 1.00  |
| Upstream Filter(I)           | 1.00 | 1.00     | 1.00     | 1.00 | 1.00        | 0.00  |
| Uniform Delay (d), s/veh     | 1.8  | 1.7      | 1.4      | 1.4  | 56.1        | 0.0   |
| Incr Delay (d2), s/veh       | 0.2  | 0.3      | 0.2      | 0.2  | 12.0        | 0.0   |
| Initial Q Delay(d3),s/veh    | 0.0  | 0.0      | 0.0      | 0.0  | 0.0         | 0.0   |
| %ile BackOfQ(50%),veh/ln     | 0.2  | 1.0      | 0.5      | 0.5  | 2.5         | 0.0   |
| Unsig. Movement Delay, s/veh |      |          |          |      |             |       |
| LnGrp Delay(d),s/veh         | 2.0  | 2.0      | 1.6      | 1.6  | 68.1        | 0.0   |
| LnGrp LOS                    | A    | A        | A        | A    | E           |       |
| Approach Vol, veh/h          |      | 998      | 444      |      | 73          | А     |
| Approach Delay, s/veh        |      | 2.0      | 1.6      |      | 68.1        | , ,   |
| Approach LOS                 |      | Α        | A        |      | E           |       |
|                              |      |          | 71       |      |             |       |
| Timer - Assigned Phs         |      | 2        |          | 4    |             | 6     |
| Phs Duration (G+Y+Rc), s     |      | 108.1    |          | 11.9 |             | 108.1 |
| Change Period (Y+Rc), s      |      | 5.5      |          | 5.5  |             | 5.5   |
| Max Green Setting (Gmax), s  |      | 65.9     |          | 43.1 |             | 65.9  |
| Max Q Clear Time (g_c+l1), s |      | 8.1      |          | 6.9  |             | 4.5   |
| Green Ext Time (p_c), s      |      | 17.8     |          | 0.2  |             | 5.7   |
| η = γ                        |      |          |          |      |             |       |
| Intersection Summary         |      |          |          |      |             |       |
| HCM 6th Ctrl Delay           |      |          | 5.1      |      |             |       |
| HCM 6th LOS                  |      |          | Α        |      |             |       |
| Notes                        |      |          |          |      |             |       |

User approved volume balancing among the lanes for turning movement.
Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

|                         | •     | 1     | <b>†</b> | <b>↓</b> |
|-------------------------|-------|-------|----------|----------|
| Lane Group              | EBL   | NBL   | NBT      | SBT      |
| Lane Configurations     | W     | ሻ     | <b>↑</b> | ĵ.       |
| Traffic Volume (vph)    | 57    | 84    | 520      | 761      |
| Future Volume (vph)     | 57    | 84    | 520      | 761      |
| Lane Group Flow (vph)   | 162   | 85    | 525      | 816      |
| Turn Type               | Prot  | Perm  | NA       | NA       |
| Protected Phases        | 4     | 2     | 2        | 6        |
| Permitted Phases        |       | 2     |          |          |
| Detector Phase          | 4     | 2     | 2        | 6        |
| Switch Phase            |       |       |          |          |
| Minimum Initial (s)     | 6.0   | 15.0  | 15.0     | 15.0     |
| Minimum Split (s)       | 61.5  | 57.5  | 57.5     | 23.5     |
| Total Split (s)         | 61.5  | 58.5  | 58.5     | 58.5     |
| Total Split (%)         | 51.3% | 48.8% | 48.8%    | 48.8%    |
| Yellow Time (s)         | 3.5   | 3.5   | 3.5      | 3.5      |
| All-Red Time (s)        | 2.0   | 2.0   | 2.0      | 2.0      |
| Lost Time Adjust (s)    | 0.0   | 0.0   | 0.0      | 0.0      |
| Total Lost Time (s)     | 5.5   | 5.5   | 5.5      | 5.5      |
| Lead/Lag                |       |       |          |          |
| Lead-Lag Optimize?      |       |       |          |          |
| Recall Mode             | None  | C-Min | C-Min    | C-Min    |
| v/c Ratio               | 0.68  | 0.18  | 0.34     | 0.54     |
| Control Delay           | 34.4  | 3.9   | 3.8      | 5.6      |
| Queue Delay             | 0.0   | 0.0   | 0.0      | 0.0      |
| Total Delay             | 34.4  | 3.9   | 3.8      | 5.6      |
| Queue Length 50th (ft)  | 46    | 10    | 77       | 153      |
| Queue Length 95th (ft)  | 113   | 31    | 156      | 310      |
| Internal Link Dist (ft) | 1475  |       | 446      | 669      |
| Turn Bay Length (ft)    |       | 50    |          |          |
| Base Capacity (vph)     | 833   | 475   | 1526     | 1514     |
| Starvation Cap Reductn  | 0     | 0     | 0        | 0        |
| Spillback Cap Reductn   | 0     | 0     | 0        | 0        |
| Storage Cap Reductn     | 0     | 0     | 0        | 0        |
| Reduced v/c Ratio       | 0.19  | 0.18  | 0.34     | 0.54     |
| Intersection Summary    |       |       |          |          |
|                         |       |       |          |          |

Cycle Length: 120

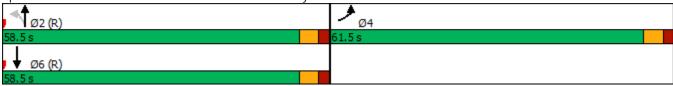
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 130

Control Type: Actuated-Coordinated

Splits and Phases: 4: N.Indian Creek Dr & Indian Creek Way



| <del></del>                  | ۶    | •     | 4    | <b>†</b> | ļ    | 4     |
|------------------------------|------|-------|------|----------|------|-------|
| Movement                     | EBL  | EBR   | NBL  | NBT      | SBT  | SBR   |
| Lane Configurations          | W    |       | ሻ    | <b>†</b> | ĵ.   |       |
| Traffic Volume (veh/h)       | 57   | 103   | 84   | 520      | 761  | 47    |
| Future Volume (veh/h)        | 57   | 103   | 84   | 520      | 761  | 47    |
| Initial Q (Qb), veh          | 0    | 0     | 0    | 0        | 0    | 0     |
| Ped-Bike Adj(A_pbT)          | 1.00 | 1.00  | 1.00 |          |      | 1.00  |
| Parking Bus, Adj             | 1.00 | 1.00  | 1.00 | 1.00     | 1.00 | 1.00  |
| Work Zone On Approach        | No   |       |      | No       | No   |       |
| Adj Sat Flow, veh/h/ln       | 1870 | 1870  | 1870 | 1870     | 1870 | 1870  |
| Adj Flow Rate, veh/h         | 58   | 104   | 85   | 525      | 769  | 47    |
| Peak Hour Factor             | 0.99 | 0.99  | 0.99 | 0.99     | 0.99 | 0.99  |
| Percent Heavy Veh, %         | 2    | 2     | 2    | 2        | 2    | 2     |
| Cap, veh/h                   | 69   | 124   | 479  | 1480     | 1380 | 84    |
| Arrive On Green              | 0.12 | 0.12  | 0.79 | 0.79     | 0.79 | 0.79  |
| Sat Flow, veh/h              | 588  | 1054  | 670  | 1870     | 1745 | 107   |
| Grp Volume(v), veh/h         | 163  | 0     | 85   | 525      | 0    | 816   |
| Grp Sat Flow(s),veh/h/ln     | 1651 | 0     | 670  | 1870     | 0    | 1851  |
| Q Serve(g_s), s              | 11.6 | 0.0   | 6.5  | 9.8      | 0.0  | 19.8  |
| Cycle Q Clear(g_c), s        | 11.6 | 0.0   | 26.3 | 9.8      | 0.0  | 19.8  |
| Prop In Lane                 | 0.36 | 0.64  | 1.00 |          |      | 0.06  |
| Lane Grp Cap(c), veh/h       | 194  | 0     | 479  | 1480     | 0    | 1464  |
| V/C Ratio(X)                 | 0.84 | 0.00  | 0.18 | 0.35     | 0.00 | 0.56  |
| Avail Cap(c_a), veh/h        | 771  | 0     | 479  | 1480     | 0    | 1464  |
| HCM Platoon Ratio            | 1.00 | 1.00  | 1.00 | 1.00     | 1.00 | 1.00  |
| Upstream Filter(I)           | 1.00 | 0.00  | 1.00 | 1.00     | 0.00 | 1.00  |
| Uniform Delay (d), s/veh     | 51.9 | 0.0   | 9.6  | 3.6      | 0.0  | 4.7   |
| Incr Delay (d2), s/veh       | 9.4  | 0.0   | 8.0  | 0.7      | 0.0  | 1.5   |
| Initial Q Delay(d3),s/veh    | 0.0  | 0.0   | 0.0  | 0.0      | 0.0  | 0.0   |
| %ile BackOfQ(50%),veh/ln     | 5.3  | 0.0   | 1.0  | 3.1      | 0.0  | 6.2   |
| Unsig. Movement Delay, s/veh |      |       |      |          |      |       |
| LnGrp Delay(d),s/veh         | 61.3 | 0.0   | 10.4 | 4.3      | 0.0  | 6.2   |
| LnGrp LOS                    | Е    | А     | В    | Α        | Α    | Α     |
| Approach Vol, veh/h          | 163  |       |      | 610      | 816  |       |
| Approach Delay, s/veh        | 61.3 |       |      | 5.2      | 6.2  |       |
| Approach LOS                 | Е    |       |      | Α        | Α    |       |
|                              |      | 2     |      |          |      | L     |
| Timer - Assigned Phs         |      | 2     |      | 4        |      | 6     |
| Phs Duration (G+Y+Rc), s     |      | 100.4 |      | 19.6     |      | 100.4 |
| Change Period (Y+Rc), s      |      | 5.5   |      | 5.5      |      | 5.5   |
| Max Green Setting (Gmax), s  |      | 53.0  |      | 56.0     |      | 53.0  |
| Max Q Clear Time (g_c+l1), s |      | 28.3  |      | 13.6     |      | 21.8  |
| Green Ext Time (p_c), s      |      | 4.2   |      | 0.5      |      | 6.8   |
| Intersection Summary         |      |       |      |          |      |       |
| HCM 6th Ctrl Delay           |      |       | 11.5 |          |      |       |
| HCM 6th LOS                  |      |       | В    |          |      |       |
|                              |      |       |      |          |      |       |

### FUTURE "NO-BUILD" INTERSECTION ANALYSIS

| Intersection           |          |         |         |          |        |       |
|------------------------|----------|---------|---------|----------|--------|-------|
| Int Delay, s/veh       | 3.7      |         |         |          |        |       |
| Movement               | EBT      | EBR     | WBL     | WBT      | NBL    | NBR   |
| Lane Configurations    | <u> </u> | 7       | 1100    | <u>₩</u> | ሻ      | T T   |
| Traffic Vol, veh/h     | 206      | 37      | 96      | 265      | 54     | 116   |
| Future Vol, veh/h      | 206      | 37      | 96      | 265      | 54     | 116   |
| Conflicting Peds, #/hr | 200      | 0       | 0       | 203      | 0      | 0     |
| Sign Control           | Free     | Free    | Free    | Free     | Stop   | Stop  |
| RT Channelized         |          |         |         |          |        |       |
|                        | -        | None    | -       | None     | -      | None  |
| Storage Length         | - " 0    | 270     | -       | -        | 0      | 30    |
| Veh in Median Storage  |          | -       | -       | 0        | 0      | -     |
| Grade, %               | 0        | -       | -       | 0        | 0      | -     |
| Peak Hour Factor       | 90       | 90      | 90      | 90       | 90     | 90    |
| Heavy Vehicles, %      | 2        | 2       | 2       | 2        | 2      | 2     |
| Mvmt Flow              | 229      | 41      | 107     | 294      | 60     | 129   |
|                        |          |         |         |          |        |       |
| Major/Minor N          | /lajor1  | 1       | Major2  | ľ        | Minor1 |       |
| Conflicting Flow All   | 0        | 0       | 270     | 0        | 737    | 229   |
| Stage 1                | -        | -       | 270     | -        | 229    | 227   |
|                        | -        | -       | -       |          | 508    | -     |
| Stage 2                | -        | -       | 110     | -        |        |       |
| Critical Hdwy          | -        | -       | 4.12    | -        | 6.42   | 6.22  |
| Critical Hdwy Stg 1    | -        | -       | -       | -        | 5.42   | -     |
| Critical Hdwy Stg 2    | -        | -       | -       | -        | 5.42   | -     |
| Follow-up Hdwy         | -        | -       | 2.218   | -        |        | 3.318 |
| Pot Cap-1 Maneuver     | -        | -       | 1293    | -        | 386    | 810   |
| Stage 1                | -        | -       | -       | -        | 809    | -     |
| Stage 2                | -        | -       | -       | -        | 604    | -     |
| Platoon blocked, %     | -        | -       |         | -        |        |       |
| Mov Cap-1 Maneuver     | -        | -       | 1293    | -        | 348    | 810   |
| Mov Cap-2 Maneuver     | -        | -       | -       | -        | 348    | -     |
| Stage 1                | -        | -       | -       | -        | 809    | -     |
| Stage 2                | -        | -       | -       | -        | 544    | -     |
| J                      |          |         |         |          |        |       |
| A                      |          |         | \A4D    |          | ND     |       |
| Approach               | EB       |         | WB      |          | NB     |       |
| HCM Control Delay, s   | 0        |         | 2.1     |          | 12.6   |       |
| HCM LOS                |          |         |         |          | В      |       |
|                        |          |         |         |          |        |       |
| Minor Lane/Major Mvm   | t N      | NBLn1 i | VIRI n2 | EBT      | EBR    | WBL   |
|                        | . 1      |         |         |          |        |       |
| Capacity (veh/h)       |          | 348     | 810     | -        |        | 1293  |
| HCM Lane V/C Ratio     |          | 0.172   |         | -        |        | 0.082 |
| HCM Control Delay (s)  |          | 17.5    | 10.3    | -        | -      | 8     |
| HCM Lane LOS           |          | С       | В       | -        | -      | Α     |
| HCM 95th %tile Q(veh)  |          | 0.6     | 0.6     | -        | -      | 0.3   |
|                        |          |         |         |          |        |       |

| Intersection           |           |        |          |        |           |           |
|------------------------|-----------|--------|----------|--------|-----------|-----------|
| Int Delay, s/veh       | 4.4       |        |          |        |           |           |
| Movement               | WBL       | WBR    | NBT      | NBR    | SBL       | SBT       |
| Lane Configurations    | ₩         |        | <b>1</b> |        | UDL       | <u> ન</u> |
| Traffic Vol, veh/h     | 22        | 62     | 77       | 14     | 69        | 64        |
| Future Vol, veh/h      | 22        | 62     | 77       | 14     | 69        | 64        |
| Conflicting Peds, #/hr | 0         | 02     | 0        | 0      | 0         | 0         |
| Sign Control           | Stop      | Stop   | Free     | Free   | Free      | Free      |
| RT Channelized         | Stop<br>- | None   | -        | None   | riee<br>- | None      |
| Storage Length         | 0         | None - |          | None - | -         | NOUG      |
| Veh in Median Storage  |           | -      | 0        | -      | -         | 0         |
|                        |           |        |          |        |           |           |
| Grade, %               | 0         | -      | 0        | -      | -         | 0         |
| Peak Hour Factor       | 86        | 86     | 86       | 86     | 86        | 86        |
| Heavy Vehicles, %      | 2         | 2      | 2        | 2      | 2         | 2         |
| Mvmt Flow              | 26        | 72     | 90       | 16     | 80        | 74        |
|                        |           |        |          |        |           |           |
| Major/Minor            | Minor1    | N      | Major1   | N      | Major2    |           |
| Conflicting Flow All   | 332       | 98     | 0        | 0      | 106       | 0         |
| Stage 1                | 98        | 70     |          | U      | 100       | -         |
|                        | 234       |        | -        | -      |           |           |
| Stage 2                |           | - / 22 | -        | -      | 112       | -         |
| Critical Hdwy          | 6.42      | 6.22   | -        | -      | 4.12      | -         |
| Critical Hdwy Stg 1    | 5.42      | -      | -        | -      | -         | -         |
| Critical Hdwy Stg 2    | 5.42      | -      | -        | -      | -         | -         |
| Follow-up Hdwy         | 3.518     |        | -        | -      | 2.218     | -         |
| Pot Cap-1 Maneuver     | 663       | 958    | -        | -      | 1485      | -         |
| Stage 1                | 926       | -      | -        | -      | -         | -         |
| Stage 2                | 805       | -      | -        | -      | -         | -         |
| Platoon blocked, %     |           |        | -        | -      |           | -         |
| Mov Cap-1 Maneuver     | 626       | 958    | -        | -      | 1485      | -         |
| Mov Cap-2 Maneuver     | 626       | -      | -        | -      | -         | -         |
| Stage 1                | 926       | -      | -        | _      | -         | -         |
| Stage 2                | 760       | _      | _        | _      | _         | _         |
| Olago 2                | 700       |        |          |        |           |           |
|                        |           |        |          |        |           |           |
| Approach               | WB        |        | NB       |        | SB        |           |
| HCM Control Delay, s   | 9.8       |        | 0        |        | 3.9       |           |
| HCM LOS                | Α         |        |          |        |           |           |
|                        |           |        |          |        |           |           |
| NA:                    |           | NDT    | NDD      | VDL 4  | CDI       | CDT       |
| Minor Lane/Major Mvn   | nt        | NBT    |          | VBLn1  | SBL       | SBT       |
| Capacity (veh/h)       |           | -      | -        | 841    | 1485      | -         |
| HCM Lane V/C Ratio     |           | -      | -        | 0.116  |           | -         |
| HCM Control Delay (s)  | )         | -      | -        | 9.8    | 7.6       | 0         |
| HCM Lane LOS           |           | -      | -        | Α      | Α         | Α         |
| HCM 95th %tile Q(veh   | )         | -      | -        | 0.4    | 0.2       | -         |
|                        |           |        |          |        |           |           |

|                         | •       | <b>→</b> | •          | -      |
|-------------------------|---------|----------|------------|--------|
| Lane Group              | EBL     | EBT      | WBT        | SBL    |
| Lane Configurations     | ች       | <b>^</b> | <b>∱</b> } | W      |
| Traffic Volume (vph)    | 44      | 327      | 714        | 24     |
| Future Volume (vph)     | 44      | 327      | 714        | 24     |
| Lane Group Flow (vph)   | 47      | 348      | 776        | 147    |
| Turn Type               | Perm    | NA       | NA         | Prot   |
| Protected Phases        | 1 01111 | 2        | 6          | 4      |
| Permitted Phases        | 2       | _        | J          | •      |
| Detector Phase          | 2       | 2        | 6          | 4      |
| Switch Phase            | 2       | 2        | U          | 7      |
| Minimum Initial (s)     | 15.0    | 15.0     | 15.0       | 6.0    |
| Minimum Split (s)       | 23.5    | 23.5     | 23.5       | 30.5   |
| Total Split (s)         | 75.0    | 75.0     | 75.0       | 45.0   |
| Total Split (%)         | 62.5%   | 62.5%    | 62.5%      | 37.5%  |
| Yellow Time (s)         | 3.5     | 3.5      | 3.5        | 37.576 |
| All-Red Time (s)        | 2.0     | 2.0      | 2.0        | 2.0    |
|                         | 0.0     | 0.0      | 0.0        | 0.0    |
| Lost Time Adjust (s)    | 5.5     | 5.5      | 5.5        | 5.5    |
| Total Lost Time (s)     | 5.5     | 5.5      | 5.5        | 5.5    |
| Lead/Lag                |         |          |            |        |
| Lead-Lag Optimize?      | O 141   | O M'     | O 14'      | Nisasa |
| Recall Mode             | C-Min   | C-Min    | C-Min      | None   |
| v/c Ratio               | 0.08    | 0.12     | 0.26       | 0.63   |
| Control Delay           | 2.6     | 2.1      | 2.5        | 26.3   |
| Queue Delay             | 0.0     | 0.0      | 0.0        | 0.0    |
| Total Delay             | 2.6     | 2.1      | 2.5        | 26.3   |
| Queue Length 50th (ft)  | 4       | 17       | 45         | 20     |
| Queue Length 95th (ft)  | 15      | 37       | 87         | 82     |
| Internal Link Dist (ft) |         | 586      | 931        | 454    |
| Turn Bay Length (ft)    | 100     |          |            |        |
| Base Capacity (vph)     | 555     | 2955     | 2947       | 621    |
| Starvation Cap Reductn  | 0       | 0        | 0          | 0      |
| Spillback Cap Reductn   | 0       | 0        | 0          | 0      |
| Storage Cap Reductn     | 0       | 0        | 0          | 0      |
| Reduced v/c Ratio       | 0.08    | 0.12     | 0.26       | 0.24   |
| Intersection Summary    |         |          |            |        |

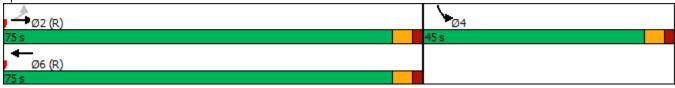
Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Splits and Phases: 3: N. Decatur Rd & Northern Ave



|                              | ۶    | <b>→</b> | •    | •    | <b>\</b>  | 4     |
|------------------------------|------|----------|------|------|-----------|-------|
| Movement                     | EBL  | EBT      | WBT  | WBR  | SBL       | SBR   |
| Lane Configurations          |      | <b>^</b> | ħβ   |      | W         |       |
| Traffic Volume (veh/h)       | 44   | 327      | 714  | 15   | 24        | 114   |
| Future Volume (veh/h)        | 44   | 327      | 714  | 15   | 24        | 114   |
| Initial Q (Qb), veh          | 0    | 0        | 0    | 0    | 0         | 0     |
| Ped-Bike Adj(A_pbT)          | 1.00 |          |      | 1.00 | 1.00      | 1.00  |
| Parking Bus, Adj             | 1.00 | 1.00     | 1.00 | 1.00 | 1.00      | 1.00  |
| Work Zone On Approach        |      | No       | No   |      | No        |       |
| Adj Sat Flow, veh/h/ln       | 1870 | 1870     | 1870 | 1870 | 1870      | 1870  |
| Adj Flow Rate, veh/h         | 47   | 348      | 760  | 16   | 26        | 0     |
| Peak Hour Factor             | 0.94 | 0.94     | 0.94 | 0.94 | 0.94      | 0.94  |
| Percent Heavy Veh, %         | 2    | 2        | 2    | 2    | 2         | 2     |
| Cap, veh/h                   | 648  | 3122     | 3127 | 66   | 51        |       |
| Arrive On Green              | 0.88 | 0.88     | 0.88 | 0.88 | 0.03      | 0.00  |
| Sat Flow, veh/h              | 695  | 3647     | 3652 | 75   | 1718      | 0     |
| Grp Volume(v), veh/h         | 47   | 348      | 379  | 397  | 27        | 0     |
| Grp Sat Flow(s), veh/h/ln    | 695  | 1777     | 1777 | 1857 | 1784      | 0     |
| Q Serve(g_s), s              | 1.3  | 1.6      | 4.0  | 4.0  | 1.8       | 0.0   |
| Cycle Q Clear(q_c), s        | 5.3  | 1.6      | 4.0  | 4.0  | 1.8       | 0.0   |
| Prop In Lane                 | 1.00 |          |      | 0.04 | 0.96      | 0.00  |
| Lane Grp Cap(c), veh/h       | 648  | 3122     | 1561 | 1632 | 53        |       |
| V/C Ratio(X)                 | 0.07 | 0.11     | 0.24 | 0.24 | 0.51      |       |
| Avail Cap(c_a), veh/h        | 648  | 3122     | 1561 | 1632 | 587       |       |
| HCM Platoon Ratio            | 1.00 | 1.00     | 1.00 | 1.00 | 1.00      | 1.00  |
| Upstream Filter(I)           | 1.00 | 1.00     | 1.00 | 1.00 | 1.00      | 0.00  |
| Uniform Delay (d), s/veh     | 1.5  | 1.0      | 1.1  | 1.1  | 57.4      | 0.0   |
| Incr Delay (d2), s/veh       | 0.2  | 0.1      | 0.4  | 0.4  | 7.4       | 0.0   |
| Initial Q Delay(d3),s/veh    | 0.0  | 0.0      | 0.0  | 0.0  | 0.0       | 0.0   |
| %ile BackOfQ(50%),veh/ln     | 0.1  | 0.2      | 0.5  | 0.5  | 0.9       | 0.0   |
| Unsig. Movement Delay, s/veh | 3.1  | J.2      |      | 3.0  | 317       | 3.0   |
| LnGrp Delay(d),s/veh         | 1.7  | 1.1      | 1.5  | 1.5  | 64.8      | 0.0   |
| LnGrp LOS                    | Α    | A        | Α    | Α    | E         | 3.0   |
| Approach Vol, veh/h          |      | 395      | 776  | ,,   | 27        | А     |
| Approach Delay, s/veh        |      | 1.1      | 1.5  |      | 64.8      | А     |
| Approach LOS                 |      | A        | Α    |      | 04.0<br>E |       |
|                              |      |          | Λ    |      |           | ,     |
| Timer - Assigned Phs         |      | 2        |      | 4    |           | 110.0 |
| Phs Duration (G+Y+Rc), s     |      | 110.9    |      | 9.1  |           | 110.9 |
| Change Period (Y+Rc), s      |      | 5.5      |      | 5.5  |           | 5.5   |
| Max Green Setting (Gmax), s  |      | 69.5     |      | 39.5 |           | 69.5  |
| Max Q Clear Time (g_c+I1), s |      | 7.3      |      | 3.8  |           | 6.0   |
| Green Ext Time (p_c), s      |      | 5.6      |      | 0.0  |           | 11.8  |
| Intersection Summary         |      |          |      |      |           |       |
| HCM 6th Ctrl Delay           |      |          | 2.8  |      |           |       |
| HCM 6th LOS                  |      |          | Α    |      |           |       |
|                              |      |          |      |      |           |       |

User approved volume balancing among the lanes for turning movement.
Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

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|                         | •     | 1     | <b>†</b> | <b>↓</b> |
|-------------------------|-------|-------|----------|----------|
| Lane Group              | EBL   | NBL   | NBT      | SBT      |
| Lane Configurations     | W     | ሻ     | <b></b>  | f)       |
| Traffic Volume (vph)    | 69    | 98    | 592      | 268      |
| Future Volume (vph)     | 69    | 98    | 592      | 268      |
| Lane Group Flow (vph)   | 149   | 102   | 617      | 425      |
| Turn Type               | Prot  | Perm  | NA       | NA       |
| Protected Phases        | 4     |       | 2        | 6        |
| Permitted Phases        |       | 2     |          |          |
| Detector Phase          | 4     | 2     | 2        | 6        |
| Switch Phase            |       |       |          |          |
| Minimum Initial (s)     | 6.0   | 15.0  | 15.0     | 15.0     |
| Minimum Split (s)       | 61.5  | 57.5  | 57.5     | 23.5     |
| Total Split (s)         | 61.5  | 58.5  | 58.5     | 58.5     |
| Total Split (%)         | 51.3% | 48.8% | 48.8%    | 48.8%    |
| Yellow Time (s)         | 3.5   | 3.5   | 3.5      | 3.5      |
| All-Red Time (s)        | 2.0   | 2.0   | 2.0      | 2.0      |
| Lost Time Adjust (s)    | 0.0   | 0.0   | 0.0      | 0.0      |
| Total Lost Time (s)     | 5.5   | 5.5   | 5.5      | 5.5      |
| Lead/Lag                | 2.0   |       |          | 2.0      |
| Lead-Lag Optimize?      |       |       |          |          |
| Recall Mode             | None  | C-Min | C-Min    | C-Min    |
| v/c Ratio               | 0.67  | 0.13  | 0.41     | 0.30     |
| Control Delay           | 44.3  | 3.5   | 4.7      | 3.7      |
| Queue Delay             | 0.0   | 0.0   | 0.0      | 0.0      |
| Total Delay             | 44.3  | 3.5   | 4.7      | 3.7      |
| Queue Length 50th (ft)  | 66    | 14    | 109      | 61       |
| Queue Length 95th (ft)  | 130   | 35    | 209      | 122      |
| Internal Link Dist (ft) | 1475  |       | 446      | 669      |
| Turn Bay Length (ft)    |       | 50    |          | 557      |
| Base Capacity (vph)     | 821   | 761   | 1504     | 1439     |
| Starvation Cap Reductn  | 0     | 0     | 0        | 0        |
| Spillback Cap Reductn   | 0     | 0     | 0        | 0        |
| Storage Cap Reductn     | 0     | 0     | 0        | 0        |
| Reduced v/c Ratio       | 0.18  | 0.13  | 0.41     | 0.30     |
|                         | 20    |       |          |          |

### Intersection Summary

Cycle Length: 120

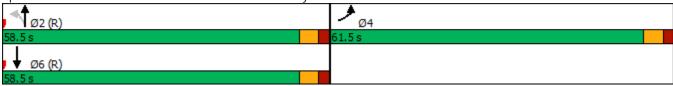
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 120

Control Type: Actuated-Coordinated

Splits and Phases: 4: N.Indian Creek Dr & Indian Creek Way



|                              | ۶    | •     | •         | <b>†</b> | <b>↓</b> | 4     |
|------------------------------|------|-------|-----------|----------|----------|-------|
| Movement                     | EBL  | EBR   | NBL       | NBT      | SBT      | SBR   |
| Lane Configurations          | W    |       | ች         | <b></b>  | <b>f</b> |       |
| Traffic Volume (veh/h)       | 69   | 74    | 98        | 592      | 268      | 140   |
| Future Volume (veh/h)        | 69   | 74    | 98        | 592      | 268      | 140   |
| Initial Q (Qb), veh          | 0    | 0     | 0         | 0        | 0        | 0     |
| Ped-Bike Adj(A_pbT)          | 1.00 | 1.00  | 1.00      |          |          | 1.00  |
| Parking Bus, Adj             | 1.00 | 1.00  | 1.00      | 1.00     | 1.00     | 1.00  |
| Work Zone On Approach        | No   |       |           | No       | No       |       |
| Adj Sat Flow, veh/h/ln       | 1870 | 1870  | 1870      | 1870     | 1870     | 1870  |
| Adj Flow Rate, veh/h         | 72   | 77    | 102       | 617      | 279      | 146   |
| Peak Hour Factor             | 0.96 | 0.96  | 0.96      | 0.96     | 0.96     | 0.96  |
| Percent Heavy Veh, %         | 2    | 2     | 2         | 2        | 2        | 2     |
| Cap, veh/h                   | 87   | 93    | 769       | 1497     | 926      | 484   |
| Arrive On Green              | 0.11 | 0.11  | 0.80      | 0.80     | 0.80     | 0.80  |
| Sat Flow, veh/h              | 804  | 860   | 962       | 1870     | 1156     | 605   |
|                              |      |       |           |          |          |       |
| Grp Volume(v), veh/h         | 150  | 0     | 102       | 617      | 0        | 425   |
| Grp Sat Flow(s), veh/h/ln    | 1675 | 0     | 962       | 1870     | 0        | 1761  |
| Q Serve(g_s), s              | 10.5 | 0.0   | 3.7       | 11.8     | 0.0      | 7.6   |
| Cycle Q Clear(g_c), s        | 10.5 | 0.0   | 11.3      | 11.8     | 0.0      | 7.6   |
| Prop In Lane                 | 0.48 | 0.51  | 1.00      | 4.407    | -        | 0.34  |
| Lane Grp Cap(c), veh/h       | 180  | 0     | 769       | 1497     | 0        | 1410  |
| V/C Ratio(X)                 | 0.83 | 0.00  | 0.13      | 0.41     | 0.00     | 0.30  |
| Avail Cap(c_a), veh/h        | 782  | 0     | 769       | 1497     | 0        | 1410  |
| HCM Platoon Ratio            | 1.00 | 1.00  | 1.00      | 1.00     | 1.00     | 1.00  |
| Upstream Filter(I)           | 1.00 | 0.00  | 1.00      | 1.00     | 0.00     | 1.00  |
| Uniform Delay (d), s/veh     | 52.5 | 0.0   | 4.6       | 3.6      | 0.0      | 3.1   |
| Incr Delay (d2), s/veh       | 9.4  | 0.0   | 0.4       | 0.8      | 0.0      | 0.5   |
| Initial Q Delay(d3),s/veh    | 0.0  | 0.0   | 0.0       | 0.0      | 0.0      | 0.0   |
| %ile BackOfQ(50%),veh/ln     | 4.9  | 0.0   | 0.7       | 3.6      | 0.0      | 2.2   |
| Unsig. Movement Delay, s/veh | 1    |       |           |          |          |       |
| LnGrp Delay(d),s/veh         | 61.9 | 0.0   | 5.0       | 4.4      | 0.0      | 3.7   |
| LnGrp LOS                    | Е    | А     | Α         | А        | Α        | Α     |
| Approach Vol, veh/h          | 150  |       |           | 719      | 425      |       |
| Approach Delay, s/veh        | 61.9 |       |           | 4.5      | 3.7      |       |
| Approach LOS                 | E    |       |           | A        | A        |       |
|                              |      |       |           |          | , ,      |       |
| Timer - Assigned Phs         |      | 2     |           | 4        |          | 6     |
| Phs Duration (G+Y+Rc), s     |      | 101.6 |           | 18.4     |          | 101.6 |
| Change Period (Y+Rc), s      |      | 5.5   |           | 5.5      |          | 5.5   |
| Max Green Setting (Gmax), s  |      | 53.0  |           | 56.0     |          | 53.0  |
| Max Q Clear Time (g_c+I1), s |      | 13.8  |           | 12.5     |          | 9.6   |
| Green Ext Time (p_c), s      |      | 5.2   |           | 0.5      |          | 3.0   |
| Intersection Summary         |      |       |           |          |          |       |
| HCM 6th Ctrl Delay           |      |       | 10.9      |          |          |       |
| HCM 6th LOS                  |      |       | 10.9<br>B |          |          |       |
| HOW OUT LOS                  |      |       | В         |          |          |       |
| Notes                        |      |       |           |          |          |       |

User approved volume balancing among the lanes for turning movement.

| Intersection           |          |           |           |           |        |        |
|------------------------|----------|-----------|-----------|-----------|--------|--------|
| Int Delay, s/veh       | 3        |           |           |           |        |        |
| Movement               | EBT      | EBR       | WBL       | WBT       | NBL    | NBR    |
| Lane Configurations    | <b>1</b> | 7         | 1,02      | <u>स्</u> | ive i  | T T    |
| Traffic Vol, veh/h     | 257      | 88        | 154       | 300       | 22     | 92     |
| Future Vol, veh/h      | 257      | 88        | 154       | 300       | 22     | 92     |
| Conflicting Peds, #/hr | 0        | 0         | 0         | 0         | 0      | 0      |
| Sign Control           | Free     | Free      | Free      | Free      | Stop   | Stop   |
| RT Channelized         | -        | None      | -         | None      | 310p   | None   |
| Storage Length         | _        | 270       | _         | -         | 0      | 30     |
| Veh in Median Storage, |          | 270       | _         | 0         | 0      | -      |
| Grade, %               |          | -         |           |           |        | -      |
|                        | 0        |           | - 01      | 0         | 0      |        |
| Peak Hour Factor       | 91       | 91        | 91        | 91        | 91     | 91     |
| Heavy Vehicles, %      | 2        | 2         | 2         | 2         | 2      | 2      |
| Mvmt Flow              | 282      | 97        | 169       | 330       | 24     | 101    |
|                        |          |           |           |           |        |        |
| Major/Minor N          | lajor1   |           | Major2    |           | Minor1 |        |
| Conflicting Flow All   | 0        | 0         | 379       | 0         | 950    | 282    |
| Stage 1                | -        | -         | -         | -         | 282    | -      |
| Stage 2                | _        | _         | _         | _         | 668    | _      |
| Critical Hdwy          | _        | _         | 4.12      | -         | 6.42   | 6.22   |
| Critical Hdwy Stg 1    | _        |           | 7.12      | _         | 5.42   | - 0.22 |
| Critical Hdwy Stg 2    | _        | _         | _         | -         | 5.42   | _      |
| Follow-up Hdwy         | -        | -         | 2.218     |           | 3.518  |        |
|                        | -        |           |           |           |        |        |
| Pot Cap-1 Maneuver     | -        | -         | 1179      | -         | 289    | 757    |
| Stage 1                | -        | -         | -         | -         | 766    | -      |
| Stage 2                | -        | -         | -         | -         | 510    | -      |
| Platoon blocked, %     | -        | -         |           | -         |        |        |
| Mov Cap-1 Maneuver     | -        | -         | 1179      | -         | 238    | 757    |
| Mov Cap-2 Maneuver     | -        | -         | -         | -         | 238    | -      |
| Stage 1                | -        | -         | -         | -         | 766    | -      |
| Stage 2                | -        | -         | -         | -         | 421    | -      |
|                        |          |           |           |           |        |        |
| Annroach               | EB       |           | WB        |           | NB     |        |
| Approach               |          |           |           |           |        |        |
| HCM Control Delay, s   | 0        |           | 2.9       |           | 12.7   |        |
| HCM LOS                |          |           |           |           | В      |        |
|                        |          |           |           |           |        |        |
| Minor Lane/Major Mvmt  |          | NBLn11    | NBLn2     | EBT       | EBR    | WBL    |
| Capacity (veh/h)       |          | 238       | 757       | -         | -      | 1179   |
| HCM Lane V/C Ratio     |          | 0.102     |           | _         |        | 0.144  |
| HCM Control Delay (s)  |          | 21.8      | 10.5      | -         |        | 8.6    |
| HCM Lane LOS           |          | 21.8<br>C | 10.5<br>B | -         | -      |        |
|                        |          |           |           | -         | -      | A      |
| HCM 95th %tile Q(veh)  |          | 0.3       | 0.5       | -         | -      | 0.5    |
|                        |          |           |           |           |        |        |

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| 3.7     |  |  |  |   |  |
|---------|--|--|--|---|--|
| W/RI    | WRR  | NRT  | NRR  | SRI   | SBT  |
|         | אטוע   |  | NON  | JDL   | <u> </u>   |
|         | <b>5</b> 7   |  | 27   | 75  | <b>식</b><br>141  |
|         |  |  |  |   |  |
|         |  |  |  |   | 141  |
|         |  |  |  |   | 0  |
|         |  |  |  |   | Free   |
|         | None   | -  | None   | -   | None   |
|         | -  | -  | -  | -   | -  |
|         | -  | 0  | -  | -   | 0  |
| 0       | -  | 0  | -  | -   | 0  |
| 86      | 86   | 86   | 86   | 86  | 86   |
| 2       | 2  | 2  | 2  | 2   | 2  |
| 35      | 66   | 78   | 31   | 87  | 164  |
|         |  |  |  |   |  |
| N.C1    |  | 1-!1   |  | 1-1   |  |
|         |  |  |  |   |  |
|         |  | 0  | 0  | 109   | 0  |
|         | -  | -  | -  | -   | -  |
|         | -  | -  | -  | -   | -  |
|         | 6.22   | -  | -  | 4.12  | -  |
|         | -  | -  | -  | -   | -  |
|         | -  | -  | -  | -   | -  |
| 3.518   | 3.318  | -  | -  | 2.218   | -  |
| 581     | 963  | -  | -  | 1481  | -  |
| 930     | -  | -  | -  | -   | -  |
| 722     | -  | -  | -  | -   | -  |
|         |  | -  | -  |   | -  |
| 543     | 963  | -  | -  | 1481  | -  |
|         | -  | -  | -  | -   | -  |
|         | _  | -  | _  | -   | _  |
|         | _  | _  | _  | _   | _  |
| 0,0     |  |  |  |   |  |
|         |  |  |  |   |  |
|         |  |  |  |   |  |
| 10.5    |  | 0  |  | 2.6   |  |
| В       |  |  |  |   |  |
|         |  |  |  |   |  |
|         |  |  | VDI1   | SBL   | SBT  |
| nt      | NRT  | NRRV   | VBIDI  |   |  |
| nt      | NBT  | NBRV   |  |   |  |
| nt      | -  | -  | 760  | 1481  | -  |
|         | -  | -  | 760<br>0.133   | 1481<br>0.059   | -  |
| nt<br>) | -<br>-<br>-  | -<br>-<br>-  | 760<br>0.133<br>10.5   | 1481<br>0.059<br>7.6  | -<br>-<br>0  |
|         | -  | -  | 760<br>0.133<br>10.5<br>B  | 1481<br>0.059   | -  |
|         | WBL 30 30 0 Stop 0 Stop 0 86 2 35  Minor1 432 94 338 6.42 5.42 5.42 3.518 581 930 722 543 543 930 675  WB 10.5 | WBL WBR  30 57 30 57 0 0 Stop Stop - None 0 e, # 0 86 86 2 2 35 66  Minor1 N 432 94 94 338 432 94 94 338 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.43 963 930 722  543 963 930 722  WB 10.5 | WBL         WBR         NBT           30         57         67           30         57         67           0         0         0           Stop         Stop         Free           None         -         0           0         -         0           86         86         86           2         2         2           35         66         78           Minor1         Major1           432         94         0           94         -         -           338         -         -           5.42         -         -           5.42         -         -           3.518         3.318         -           581         963         -           930         -         -           543         963         -           930         -         -           675         -         -           WB         NB           10.5         0 | WBL         WBR         NBT         NBR           30         57         67         27           0         0         0         0           Stop         Stop         Free         Free           -         None         -         None           0         -         -         -           0         -         0         -           86         86         86         86           2         2         2         2           35         66         78         31    Minor1  Major1  Major2  Major3  Major3 | WBL         WBR         NBT         NBR         SBL           Y         1         7           30         57         67         27         75           30         57         67         27         75           0         0         0         0         0           Stop         Stop         Free         Free         Free           - None         -         None         -           0         -         -         -         -           86         86         86         86         86           2         2         2         2         2         2           35         66         78         31         87           Minor1         Major1         Major2           432         94         0         0         109           94         -         -         -         -           338         -         -         -         -           5.42         -         -         -         -           5.42         -         -         -         -           581         963         -         1481 |

|                         | •     | -        | <b>←</b>   | -     |
|-------------------------|-------|----------|------------|-------|
| Lane Group              | EBL   | EBT      | WBT        | SBL   |
| Lane Configurations     | ሻ     | <b>^</b> | <b>↑</b> ↑ | W     |
| Traffic Volume (vph)    | 72    | 895      | 397        | 69    |
| Future Volume (vph)     | 72    | 895      | 397        | 69    |
| Lane Group Flow (vph)   | 76    | 942      | 454        | 175   |
| Turn Type               | Perm  | NA       | NA         | Prot  |
| Protected Phases        |       | 2        | 6          | 4     |
| Permitted Phases        | 2     |          |            |       |
| Detector Phase          | 2     | 2        | 6          | 4     |
| Switch Phase            |       |          |            |       |
| Minimum Initial (s)     | 15.0  | 15.0     | 15.0       | 6.0   |
| Minimum Split (s)       | 23.5  | 23.5     | 23.5       | 30.5  |
| Total Split (s)         | 77.0  | 77.0     | 77.0       | 43.0  |
| Total Split (%)         | 64.2% | 64.2%    | 64.2%      | 35.8% |
| Yellow Time (s)         | 3.5   | 3.5      | 3.5        | 3.5   |
| All-Red Time (s)        | 2.0   | 2.0      | 2.0        | 2.0   |
| Lost Time Adjust (s)    | 0.0   | 0.0      | 0.0        | 0.0   |
| Total Lost Time (s)     | 5.5   | 5.5      | 5.5        | 5.5   |
| Lead/Lag                |       |          |            |       |
| Lead-Lag Optimize?      |       |          |            |       |
| Recall Mode             | C-Min | C-Min    | C-Min      | None  |
| v/c Ratio               | 0.10  | 0.34     | 0.16       | 0.71  |
| Control Delay           | 3.9   | 4.3      | 3.4        | 47.7  |
| Queue Delay             | 0.0   | 0.0      | 0.0        | 0.0   |
| Total Delay             | 3.9   | 4.3      | 3.4        | 47.7  |
| Queue Length 50th (ft)  | 11    | 88       | 34         | 86    |
| Queue Length 95th (ft)  | 30    | 150      | 63         | 154   |
| Internal Link Dist (ft) |       | 586      | 931        | 454   |
| Turn Bay Length (ft)    | 100   |          |            |       |
| Base Capacity (vph)     | 724   | 2806     | 2775       | 567   |
| Starvation Cap Reductn  | 0     | 0        | 0          | 0     |
| Spillback Cap Reductn   | 0     | 0        | 0          | 0     |
| Storage Cap Reductn     | 0     | 0        | 0          | 0     |
| Reduced v/c Ratio       | 0.10  | 0.34     | 0.16       | 0.31  |
| Interesetion Cummers    |       |          |            |       |

### **Intersection Summary**

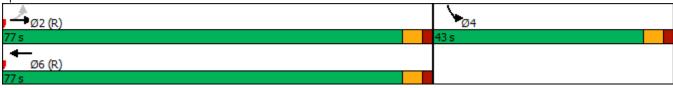
Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Splits and Phases: 3: N. Decatur Rd & Northern Ave



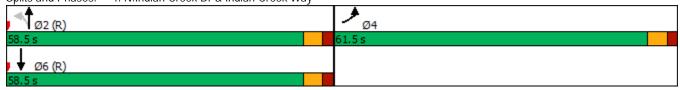
|                              | ۶    | <b>→</b> | <b>←</b>   | •    | -    | 4     |
|------------------------------|------|----------|------------|------|------|-------|
| Movement                     | EBL  | EBT      | WBT        | WBR  | SBL  | SBR   |
| Lane Configurations          | Ť    | <b>^</b> | <b>↑</b> ↑ | WEI  | ¥    | ODIN  |
| Traffic Volume (veh/h)       | 72   | 895      | 397        | 34   | 69   | 97    |
| Future Volume (veh/h)        | 72   | 895      | 397        | 34   | 69   | 97    |
| Initial Q (Qb), veh          | 0    | 0        | 0          | 0    | 0    | 0     |
| Ped-Bike Adj(A_pbT)          | 1.00 | U        | U          | 1.00 | 1.00 | 1.00  |
| Parking Bus, Adj             | 1.00 | 1.00     | 1.00       | 1.00 | 1.00 | 1.00  |
| Work Zone On Approach        | 1.00 | No       | No         | 1.00 | No   | 1.00  |
| Adj Sat Flow, veh/h/ln       | 1870 | 1870     | 1870       | 1870 | 1870 | 1870  |
| Adj Flow Rate, veh/h         | 76   | 942      | 418        | 36   | 73   | 0     |
|                              |      |          |            |      |      |       |
| Peak Hour Factor             | 0.95 | 0.95     | 0.95       | 0.95 | 0.95 | 0.95  |
| Percent Heavy Veh, %         | 2    | 2        | 2          | 2    | 2    | 2     |
| Cap, veh/h                   | 841  | 3036     | 2829       | 243  | 95   | 0.00  |
| Arrive On Green              | 0.85 | 0.85     | 0.85       | 0.85 | 0.05 | 0.00  |
| Sat Flow, veh/h              | 937  | 3647     | 3406       | 284  | 1758 | 0     |
| Grp Volume(v), veh/h         | 76   | 942      | 223        | 231  | 74   | 0     |
| Grp Sat Flow(s),veh/h/ln     | 937  | 1777     | 1777       | 1819 | 1782 | 0     |
| Q Serve(g_s), s              | 1.8  | 6.3      | 2.5        | 2.5  | 4.9  | 0.0   |
| Cycle Q Clear(g_c), s        | 4.3  | 6.3      | 2.5        | 2.5  | 4.9  | 0.0   |
| Prop In Lane                 | 1.00 |          |            | 0.16 | 0.99 | 0.00  |
| Lane Grp Cap(c), veh/h       | 841  | 3036     | 1518       | 1554 | 96   |       |
| V/C Ratio(X)                 | 0.09 | 0.31     | 0.15       | 0.15 | 0.77 |       |
| Avail Cap(c_a), veh/h        | 841  | 3036     | 1518       | 1554 | 557  |       |
| HCM Platoon Ratio            | 1.00 | 1.00     | 1.00       | 1.00 | 1.00 | 1.00  |
| Upstream Filter(I)           | 1.00 | 1.00     | 1.00       | 1.00 | 1.00 | 0.00  |
| Uniform Delay (d), s/veh     | 1.8  | 1.7      | 1.5        | 1.5  | 56.0 | 0.00  |
| Incr Delay (d2), s/veh       | 0.2  | 0.3      | 0.2        | 0.2  | 12.0 | 0.0   |
| Initial Q Delay(d3),s/veh    | 0.2  | 0.0      | 0.2        | 0.2  | 0.0  | 0.0   |
| %ile BackOfQ(50%),veh/ln     | 0.0  | 1.1      | 0.0        | 0.0  | 2.5  | 0.0   |
|                              |      | 1.1      | 0.5        | 0.5  | 2.5  | 0.0   |
| Unsig. Movement Delay, s/veh |      | 2.0      | 17         | 17   | /0.0 | 0.0   |
| LnGrp Delay(d),s/veh         | 2.0  | 2.0      | 1.7        | 1.7  | 68.0 | 0.0   |
| LnGrp LOS                    | A    | Α        | Α          | A    | E    |       |
| Approach Vol, veh/h          |      | 1018     | 454        |      | 74   | А     |
| Approach Delay, s/veh        |      | 2.0      | 1.7        |      | 68.0 |       |
| Approach LOS                 |      | Α        | Α          |      | Е    |       |
| Timer - Assigned Phs         |      | 2        |            | 4    |      | 6     |
| Phs Duration (G+Y+Rc), s     |      | 108.0    |            | 12.0 |      | 108.0 |
| Change Period (Y+Rc), s      |      | 5.5      |            | 5.5  |      | 5.5   |
| Max Green Setting (Gmax), s  |      | 71.5     |            | 37.5 |      | 71.5  |
| Max Q Clear Time (q_c+l1), s |      |          |            | 6.9  |      | 4.5   |
| Green Ext Time (p_c), s      |      | 8.3      |            | 0.9  |      | 5.9   |
| 4 - 7                        |      | 18.8     |            | 0.2  |      | 5.9   |
| Intersection Summary         |      |          |            |      |      |       |
| HCM 6th Ctrl Delay           |      |          | 5.1        |      |      |       |
| HCM 6th LOS                  |      |          | А          |      |      |       |
|                              |      |          |            |      |      |       |

User approved volume balancing among the lanes for turning movement.
Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

|   | •            | 4        | <b>†</b>  | ļ          |
|---|--------------|----------|-----------|------------|
| Lane Group  | EBL          | NBL      | NBT       | SBT        |
| Lane Configurations   | W            | ሻ        | <b>†</b>  | 1>         |
| Traffic Volume (vph)  | 58           | 86       | 530       | 776        |
| Future Volume (vph)   | 58           | 86       | 530       | 776        |
| Lane Group Flow (vph)   | 166          | 87       | 535       | 832        |
| Turn Type   | Prot         | Perm     | NA        | NA         |
| Protected Phases  | 4            | . 3      | 2         | 6          |
| Permitted Phases  |              | 2        |           |            |
| Detector Phase  | 4            | 2        | 2         | 6          |
| Switch Phase  |              | <u>=</u> |           |            |
| Minimum Initial (s)   | 6.0          | 15.0     | 15.0      | 15.0       |
| Minimum Split (s)   | 61.5         | 57.5     | 57.5      | 23.5       |
| Total Split (s)   | 61.5         | 58.5     | 58.5      | 58.5       |
| Total Split (%)   | 51.3%        | 48.8%    | 48.8%     | 48.8%      |
| Yellow Time (s)   | 3.5          | 3.5      | 3.5       | 3.5        |
| All-Red Time (s)  | 2.0          | 2.0      | 2.0       | 2.0        |
| Lost Time Adjust (s)  | 0.0          | 0.0      | 0.0       | 0.0        |
| Total Lost Time (s)   | 5.5          | 5.5      | 5.5       | 5.5        |
| Lead/Lag  | 5.5          | 5.5      | 5.5       | 5.5        |
| Lead-Lag Optimize?  |              |          |           |            |
| Recall Mode   | None         | C-Min    | C-Min     | C-Min      |
| v/c Ratio   | 0.68         | 0.19     | 0.35      | 0.55       |
| Control Delay   | 34.8         | 4.0      | 3.9       | 5.8        |
| Queue Delay   | 0.0          | 0.0      | 0.0       | 0.0        |
| Total Delay   | 34.8         | 4.0      | 3.9       | 5.8        |
| Queue Length 50th (ft)  | 48           | 11       | 80        | 160        |
| Queue Length 95th (ft)  | 115          | 33       | 163       | 326        |
| Internal Link Dist (ft)   | 1475         | - 33     | 446       | 669        |
| Turn Bay Length (ft)  | 1470         | 50       | 440       | 009        |
| Base Capacity (vph)   | 834          | 464      | 1523      | 1511       |
| Starvation Cap Reductn  | 0            | 404      | 1523      | 1511       |
| Spillback Cap Reductin  | 0            | 0        | 0         | 0          |
|   |              |          |           |            |
|   |              |          |           |            |
|   | 0.20         | U. 19    | U.35      | U.55       |
|   |              |          |           |            |
|   | 00           |          |           |            |
|   |              |          |           |            |
| , ,   | d to phase 2 | :NBTL ar | nd 6:SBT, | Start of C |
| Storage Cap Reductin Reduced v/c Ratio Intersection Summary Cycle Length: 120 Actuated Cycle Length: 12 Offset: 0 (0%), Referenced Natural Cycle: 130 | 0 0.20       | 0 0.19   | 0<br>0.35 | 0<br>0.55  |

Natural Cycle: 130 Control Type: Actuated-Coordinated

Splits and Phases: 4: N.Indian Creek Dr & Indian Creek Way



|                                       | ۶    | •     | •    | <b>†</b> | ļ        | 4     |
|---------------------------------------|------|-------|------|----------|----------|-------|
| Movement                              | EBL  | EBR   | NBL  | NBT      | SBT      | SBR   |
| Lane Configurations                   | W    |       | ች    | <b></b>  | <b>f</b> |       |
| Traffic Volume (veh/h)                | 58   | 106   | 86   | 530      | 776      | 48    |
| Future Volume (veh/h)                 | 58   | 106   | 86   | 530      | 776      | 48    |
| Initial Q (Qb), veh                   | 0    | 0     | 0    | 0        | 0        | 0     |
| Ped-Bike Adj(A_pbT)                   | 1.00 | 1.00  | 1.00 |          |          | 1.00  |
| Parking Bus, Adj                      | 1.00 | 1.00  | 1.00 | 1.00     | 1.00     | 1.00  |
| Work Zone On Approach                 | No   |       |      | No       | No       |       |
| Adj Sat Flow, veh/h/ln                | 1870 | 1870  | 1870 | 1870     | 1870     | 1870  |
| Adj Flow Rate, veh/h                  | 59   | 107   | 87   | 535      | 784      | 48    |
| Peak Hour Factor                      | 0.99 | 0.99  | 0.99 | 0.99     | 0.99     | 0.99  |
| Percent Heavy Veh, %                  | 2    | 2     | 2    | 2        | 2        | 2     |
| Cap, veh/h                            | 70   | 127   | 466  | 1475     | 1375     | 84    |
| Arrive On Green                       | 0.12 | 0.12  | 0.79 | 0.79     | 0.79     | 0.79  |
| Sat Flow, veh/h                       | 583  | 1058  | 660  | 1870     | 1744     | 107   |
| · · · · · · · · · · · · · · · · · · · | 167  |       | 87   | 535      |          | 832   |
| Grp Volume(v), veh/h                  |      | 0     |      |          | 0        |       |
| Grp Sat Flow(s), veh/h/ln             | 1651 | 0     | 660  | 1870     | 0        | 1851  |
| Q Serve(g_s), s                       | 11.9 | 0.0   | 7.0  | 10.2     | 0.0      | 20.7  |
| Cycle Q Clear(g_c), s                 | 11.9 | 0.0   | 27.7 | 10.2     | 0.0      | 20.7  |
| Prop In Lane                          | 0.35 | 0.64  | 1.00 | 4.75     |          | 0.06  |
| Lane Grp Cap(c), veh/h                | 198  | 0     | 466  | 1475     | 0        | 1460  |
| V/C Ratio(X)                          | 0.84 | 0.00  | 0.19 | 0.36     | 0.00     | 0.57  |
| Avail Cap(c_a), veh/h                 | 770  | 0     | 466  | 1475     | 0        | 1460  |
| HCM Platoon Ratio                     | 1.00 | 1.00  | 1.00 | 1.00     | 1.00     | 1.00  |
| Upstream Filter(I)                    | 1.00 | 0.00  | 1.00 | 1.00     | 0.00     | 1.00  |
| Uniform Delay (d), s/veh              | 51.7 | 0.0   | 10.2 | 3.8      | 0.0      | 4.9   |
| Incr Delay (d2), s/veh                | 9.4  | 0.0   | 0.9  | 0.7      | 0.0      | 1.6   |
| Initial Q Delay(d3),s/veh             | 0.0  | 0.0   | 0.0  | 0.0      | 0.0      | 0.0   |
| %ile BackOfQ(50%),veh/ln              | 5.5  | 0.0   | 1.1  | 3.2      | 0.0      | 6.6   |
| Unsig. Movement Delay, s/veh          |      |       |      |          |          |       |
| LnGrp Delay(d),s/veh                  | 61.1 | 0.0   | 11.1 | 4.5      | 0.0      | 6.5   |
| LnGrp LOS                             | E    | А     | В    | А        | А        | Α     |
| Approach Vol, veh/h                   | 167  |       |      | 622      | 832      |       |
| Approach Delay, s/veh                 | 61.1 |       |      | 5.4      | 6.5      |       |
| Approach LOS                          | E    |       |      | A        | A        |       |
|                                       |      |       |      |          | , ,      |       |
| Timer - Assigned Phs                  |      | 2     |      | 4        |          | 6     |
| Phs Duration (G+Y+Rc), s              |      | 100.1 |      | 19.9     |          | 100.1 |
| Change Period (Y+Rc), s               |      | 5.5   |      | 5.5      |          | 5.5   |
| Max Green Setting (Gmax), s           |      | 53.0  |      | 56.0     |          | 53.0  |
| Max Q Clear Time (g_c+I1), s          |      | 29.7  |      | 13.9     |          | 22.7  |
| Green Ext Time (p_c), s               |      | 4.3   |      | 0.6      |          | 7.0   |
| Intersection Summary                  |      |       |      |          |          |       |
| HCM 6th Ctrl Delay                    |      |       | 11.7 |          |          |       |
| HCM 6th LOS                           |      |       | В    |          |          |       |
|                                       |      |       |      |          |          |       |
| Notes                                 |      |       |      |          |          |       |

User approved volume balancing among the lanes for turning movement.

| Future | "BUILD" | INTERSECTIO | ON ANALYSIS |
|--------|---------|-------------|-------------|
|        |         |             |             |

| Intersection           |          |          |         |      |        |                 |  |
|------------------------|----------|----------|---------|------|--------|-----------------|--|
| Int Delay, s/veh       | 4.1      |          |         |      |        |                 |  |
| Movement               | EBT      | EBR      | WBL     | WBT  | NBL    | NBR             |  |
| Lane Configurations    | <b>†</b> | 7        |         | स    | *      | 7               |  |
| Traffic Vol, veh/h     | 206      | 40       | 100     | 265  | 64     | 129             |  |
| Future Vol, veh/h      | 206      | 40       | 100     | 265  | 64     | 129             |  |
| Conflicting Peds, #/hr | 0        | 0        | 0       | 0    | 0      | 0               |  |
| Sign Control           | Free     | Free     | Free    | Free | Stop   | Stop            |  |
| RT Channelized         | -        | None     | -       |      | -      | None            |  |
| Storage Length         | -        | 270      | -       | -    | 0      | 30              |  |
| Veh in Median Storage  |          | -        | -       | 0    | 0      | -               |  |
| Grade, %               | 0        | -        | -       | 0    | 0      | -               |  |
| Peak Hour Factor       | 90       | 90       | 90      | 90   | 90     | 90              |  |
| Heavy Vehicles, %      | 2        | 2        | 2       | 2    | 2      | 2               |  |
| Mymt Flow              | 229      | 44       | 111     | 294  | 71     | 143             |  |
| IVIVIII I IOVV         | LL /     |          | - 111   | 2/7  | 7 1    | 170             |  |
|                        |          |          |         |      |        |                 |  |
|                        | /lajor1  |          | Major2  |      | Vinor1 |                 |  |
| Conflicting Flow All   | 0        | 0        | 273     | 0    | 745    | 229             |  |
| Stage 1                | -        | -        | -       | -    | 229    | -               |  |
| Stage 2                | -        | -        | -       | -    | 516    | -               |  |
| Critical Hdwy          | -        | -        | 4.12    | -    | 6.42   | 6.22            |  |
| Critical Hdwy Stg 1    | -        | -        | -       | -    | 5.42   | -               |  |
| Critical Hdwy Stg 2    | -        | -        | -       | -    | 5.42   | -               |  |
| Follow-up Hdwy         | -        | -        | 2.218   | -    | 3.518  | 3.318           |  |
| Pot Cap-1 Maneuver     | -        | -        | 1290    | -    | 382    | 810             |  |
| Stage 1                | -        | -        | -       | -    | 809    | -               |  |
| Stage 2                | -        | -        | -       | -    | 599    | -               |  |
| Platoon blocked, %     | -        | -        |         | -    |        |                 |  |
| Mov Cap-1 Maneuver     | -        | -        | 1290    | -    | 343    | 810             |  |
| Mov Cap-2 Maneuver     | _        | _        | -       | _    | 343    | -               |  |
| Stage 1                | _        | _        | _       | -    | 809    | -               |  |
| Stage 2                | _        | _        | _       | _    | 537    | _               |  |
| Jiaye Z                |          | -        | _       | _    | 331    |                 |  |
|                        |          |          |         |      |        |                 |  |
| Approach               | EB       |          | WB      |      | NB     |                 |  |
| HCM Control Delay, s   | 0        |          | 2.2     |      | 13     |                 |  |
| HCM LOS                |          |          |         |      | В      |                 |  |
|                        |          |          |         |      |        |                 |  |
| Minor Lanc/Major Mum   | + N      | UDI 51 N | \IDI ~2 | EDT  | EDD    | WDI             |  |
| Minor Lane/Major Mvm   | t ľ      | VBLn1 N  |         | EBT  | EBR    | WBL             |  |
| Capacity (veh/h)       |          | 343      | 810     | -    | -      | 1290            |  |
|                        |          | いつハフ     | 0.177   | -    | -      | 0.086           |  |
| HCM Lane V/C Ratio     |          |          |         |      |        | 0.4             |  |
| HCM Control Delay (s)  |          | 18.2     | 10.4    | -    | -      | 8.1             |  |
|                        |          |          |         | -    | -      | 8.1<br>A<br>0.3 |  |

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| Intersection                          |        |      |         |          |          |          |
|---------------------------------------|--------|------|---------|----------|----------|----------|
| Int Delay, s/veh                      | 4      |      |         |          |          |          |
| Movement                              | WBL    | WBR  | NBT     | NBR      | SBL      | SBT      |
| Lane Configurations                   | ¥      | WDIX | 4       | NDIX     | ODL      | <u>ુ</u> |
| Traffic Vol, veh/h                    | 30     | 62   | 100     | 41       | 69       | 71       |
| Future Vol, veh/h                     | 30     | 62   | 100     | 41       | 69       | 71       |
| Conflicting Peds, #/hr                | 0      | 0    | 0       | 0        | 0        | 0        |
| Sign Control                          | Stop   | Stop | Free    | Free     | Free     | Free     |
| RT Channelized                        | -      | None | -       | None     | -        | None     |
| Storage Length                        | 0      | -    | -       | -        | -        | -        |
| Veh in Median Storage                 |        | -    | 0       | -        | -        | 0        |
| Grade, %                              | 0      | -    | 0       | -        | -        | 0        |
| Peak Hour Factor                      | 86     | 86   | 86      | 86       | 86       | 86       |
| Heavy Vehicles, %                     | 2      | 2    | 2       | 2        | 2        | 2        |
| Mvmt Flow                             | 35     | 72   | 116     | 48       | 80       | 83       |
|                                       |        |      |         |          |          |          |
| Major/Minor                           | Ninar1 |      | Notor1  |          | Majora   |          |
|                                       | Minor1 |      | /lajor1 |          | Major2   |          |
| Conflicting Flow All                  | 383    | 140  | 0       | 0        | 164      | 0        |
| Stage 1                               | 140    | -    | -       | -        | -        | -        |
| Stage 2                               | 243    | -    | -       | -        | -        | -        |
| Critical Hdwy                         | 6.42   | 6.22 | -       | -        | 4.12     | -        |
| Critical Hdwy Stg 1                   | 5.42   | -    | -       | -        | -        | -        |
| Critical Hdwy Stg 2                   | 5.42   | -    | -       | -        | -        | -        |
| Follow-up Hdwy                        | 3.518  |      | -       |          | 2.218    | -        |
| Pot Cap-1 Maneuver                    | 620    | 908  | -       | -        | 1414     | -        |
| Stage 1                               | 887    | -    | -       | -        | -        | -        |
| Stage 2                               | 797    | -    | -       | -        | -        | -        |
| Platoon blocked, %                    | F00    | 000  | -       | -        | 4444     | -        |
| Mov Cap-1 Maneuver                    | 583    | 908  | -       | -        | 1414     | -        |
| Mov Cap-2 Maneuver                    | 583    | -    | -       | -        | -        | -        |
| Stage 1                               | 887    | -    | -       | -        | -        | -        |
| Stage 2                               | 750    | -    | -       | -        | -        | -        |
|                                       |        |      |         |          |          |          |
| Approach                              | WB     |      | NB      |          | SB       |          |
| HCM Control Delay, s                  | 10.4   |      | 0       |          | 3.8      |          |
| HCM LOS                               | В      |      | U       |          | 0.0      |          |
| TIOW EGG                              |        |      |         |          |          |          |
|                                       |        |      |         |          |          |          |
| Minor Lane/Major Mvm                  | ıt     | NBT  | NBRV    | VBLn1    | SBL      | SBT      |
| Capacity (veh/h)                      |        | -    | -       | 768      | 1414     | -        |
| HCM Lane V/C Ratio                    |        | -    | -       | 0.139    |          | -        |
| HCM Control Delay (s)                 |        | -    | -       | 10.4     | 7.7      | 0        |
|                                       |        |      |         |          |          |          |
| HCM Lane LOS<br>HCM 95th %tile Q(veh) |        | -    | -       | B<br>0.5 | A<br>0.2 | Α        |

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|                         | •        | -        | •          | <b>&gt;</b> |
|-------------------------|----------|----------|------------|-------------|
| Lane Group              | EBL      | EBT      | WBT        | SBL         |
| Lane Configurations     | <u> </u> | <b>^</b> | <b>†</b> } | ¥           |
| Traffic Volume (vph)    | 47       | 327      | 714        | 31          |
| Future Volume (vph)     | 47       | 327      | 714        | 31          |
| Lane Group Flow (vph)   | 50       | 348      | 778        | 165         |
| Turn Type               | Perm     | NA       | NA         | Prot        |
| Protected Phases        |          | 2        | 6          | 4           |
| Permitted Phases        | 2        |          |            |             |
| Detector Phase          | 2        | 2        | 6          | 4           |
| Switch Phase            |          |          |            |             |
| Minimum Initial (s)     | 15.0     | 15.0     | 15.0       | 6.0         |
| Minimum Split (s)       | 23.5     | 23.5     | 23.5       | 30.5        |
| Total Split (s)         | 74.0     | 74.0     | 74.0       | 46.0        |
| Total Split (%)         | 61.7%    | 61.7%    | 61.7%      | 38.3%       |
| Yellow Time (s)         | 3.5      | 3.5      | 3.5        | 3.5         |
| All-Red Time (s)        | 2.0      | 2.0      | 2.0        | 2.0         |
| Lost Time Adjust (s)    | 0.0      | 0.0      | 0.0        | 0.0         |
| Total Lost Time (s)     | 5.5      | 5.5      | 5.5        | 5.5         |
| Lead/Lag                |          |          |            |             |
| Lead-Lag Optimize?      |          |          |            |             |
| Recall Mode             | C-Min    | C-Min    | C-Min      | None        |
| v/c Ratio               | 0.09     | 0.12     | 0.27       | 0.67        |
| Control Delay           | 2.8      | 2.2      | 2.6        | 27.3        |
| Queue Delay             | 0.0      | 0.0      | 0.0        | 0.0         |
| Total Delay             | 2.8      | 2.2      | 2.6        | 27.3        |
| Queue Length 50th (ft)  | 5        | 18       | 47         | 25          |
| Queue Length 95th (ft)  | 17       | 39       | 92         | 92          |
| Internal Link Dist (ft) |          | 586      | 931        | 2806        |
| Turn Bay Length (ft)    | 100      |          |            |             |
| Base Capacity (vph)     | 549      | 2942     | 2934       | 642         |
| Starvation Cap Reductn  | 0        | 0        | 0          | 0           |
| Spillback Cap Reductn   | 0        | 0        | 0          | 0           |
| Storage Cap Reductn     | 0        | 0        | 0          | 0           |
| Reduced v/c Ratio       | 0.09     | 0.12     | 0.27       | 0.26        |
| Intonocation Commence   |          |          |            |             |

## Intersection Summary

Cycle Length: 120

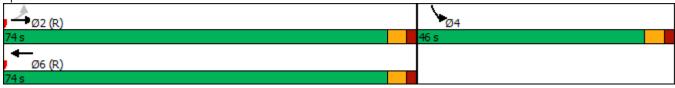
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Splits and Phases: 3: N. Decatur Rd & Northern Ave



|                                       | ᄼ    | <b>→</b> | <b>←</b>   | •      | <b>&gt;</b> | 4     |
|---------------------------------------|------|----------|------------|--------|-------------|-------|
| Movement                              | EBL  | EBT      | WBT        | WBR    | SBL         | SBR   |
| Lane Configurations                   | ሻ    | <b>^</b> | <b>↑</b> ↑ |        | ¥           |       |
| Traffic Volume (veh/h)                | 47   | 327      | 714        | 17     | 31          | 124   |
| Future Volume (veh/h)                 | 47   | 327      | 714        | 17     | 31          | 124   |
| Initial Q (Qb), veh                   | 0    | 0        | 0          | 0      | 0           | 0     |
| Ped-Bike Adj(A_pbT)                   | 1.00 |          |            | 1.00   | 1.00        | 1.00  |
| Parking Bus, Adj                      | 1.00 | 1.00     | 1.00       | 1.00   | 1.00        | 1.00  |
| Work Zone On Approach                 |      | No       | No         | .,,,,, | No          |       |
| Adj Sat Flow, veh/h/ln                | 1870 | 1870     | 1870       | 1870   | 1870        | 1870  |
| Adj Flow Rate, veh/h                  | 50   | 348      | 760        | 18     | 33          | 0     |
| Peak Hour Factor                      | 0.94 | 0.94     | 0.94       | 0.94   | 0.94        | 0.94  |
| Percent Heavy Veh, %                  | 2    | 2        | 2          | 2      | 2           | 2     |
| Cap, veh/h                            | 643  | 3107     | 3103       | 73     | 59          |       |
| Arrive On Green                       | 0.87 | 0.87     | 0.87       | 0.87   | 0.03        | 0.00  |
| Sat Flow, veh/h                       | 694  | 3647     | 3642       | 84     | 1731        | 0.00  |
| · · · · · · · · · · · · · · · · · · · |      |          |            |        |             |       |
| Grp Volume(v), veh/h                  | 50   | 348      | 381        | 397    | 34          | 0     |
| Grp Sat Flow(s), veh/h/ln             | 694  | 1777     | 1777       | 1855   | 1784        | 0     |
| Q Serve(g_s), s                       | 1.5  | 1.6      | 4.1        | 4.1    | 2.3         | 0.0   |
| Cycle Q Clear(g_c), s                 | 5.6  | 1.6      | 4.1        | 4.1    | 2.3         | 0.0   |
| Prop In Lane                          | 1.00 |          |            | 0.05   | 0.97        | 0.00  |
| Lane Grp Cap(c), veh/h                | 643  | 3107     | 1554       | 1622   | 60          |       |
| V/C Ratio(X)                          | 0.08 | 0.11     | 0.24       | 0.25   | 0.56        |       |
| Avail Cap(c_a), veh/h                 | 643  | 3107     | 1554       | 1622   | 602         |       |
| HCM Platoon Ratio                     | 1.00 | 1.00     | 1.00       | 1.00   | 1.00        | 1.00  |
| Upstream Filter(I)                    | 1.00 | 1.00     | 1.00       | 1.00   | 1.00        | 0.00  |
| Uniform Delay (d), s/veh              | 1.7  | 1.0      | 1.2        | 1.2    | 57.1        | 0.0   |
| Incr Delay (d2), s/veh                | 0.2  | 0.1      | 0.4        | 0.4    | 7.9         | 0.0   |
| Initial Q Delay(d3),s/veh             | 0.0  | 0.0      | 0.0        | 0.0    | 0.0         | 0.0   |
| %ile BackOfQ(50%),veh/ln              | 0.1  | 0.2      | 0.6        | 0.6    | 1.1         | 0.0   |
| Unsig. Movement Delay, s/veh          |      |          |            |        |             |       |
| LnGrp Delay(d),s/veh                  | 1.9  | 1.1      | 1.6        | 1.6    | 65.0        | 0.0   |
| LnGrp LOS                             | Α    | А        | A          | A      | E           | - 0.0 |
| Approach Vol, veh/h                   | - 1  | 398      | 778        | **     | 34          | A     |
| Approach Delay, s/veh                 |      | 1.2      | 1.6        |        | 65.0        | А     |
| Approach LOS                          |      | Α        | Α          |        | E           |       |
| Approach E03                          |      | А        | A          |        | L           |       |
| Timer - Assigned Phs                  |      | 2        |            | 4      |             | 6     |
| Phs Duration (G+Y+Rc), s              |      | 110.4    |            | 9.6    |             | 110.4 |
| Change Period (Y+Rc), s               |      | 5.5      |            | 5.5    |             | 5.5   |
| Max Green Setting (Gmax), s           |      | 68.5     |            | 40.5   |             | 68.5  |
| Max Q Clear Time (q_c+l1), s          |      | 7.6      |            | 4.3    |             | 6.1   |
| Green Ext Time (p_c), s               |      | 5.7      |            | 0.1    |             | 11.8  |
|                                       |      |          |            |        |             |       |
| Intersection Summary                  |      |          |            |        |             |       |
| HCM 6th Ctrl Delay                    |      |          | 3.2        |        |             |       |
| HCM 6th LOS                           |      |          | Α          |        |             |       |
| Notos                                 |      |          |            |        |             |       |

User approved volume balancing among the lanes for turning movement.
Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

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|                              | •     | 4     | <b>†</b> | ļ     |
|------------------------------|-------|-------|----------|-------|
| Lane Group                   | EBL   | NBL   | NBT      | SBT   |
| Lane Configurations          | ¥     | *     | <b></b>  | 4     |
| Traffic Volume (vph)         | 86    | 101   | 592      | 268   |
| Future Volume (vph)          | 86    | 101   | 592      | 268   |
| Lane Group Flow (vph)        | 178   | 105   | 617      | 430   |
| Turn Type                    | Prot  | Perm  | NA       | NA    |
| Protected Phases             | 4     | . 5   | 2        | 6     |
| Permitted Phases             |       | 2     | <b>_</b> |       |
| Detector Phase               | 4     | 2     | 2        | 6     |
| Switch Phase                 |       |       | <b>_</b> |       |
| Minimum Initial (s)          | 6.0   | 15.0  | 15.0     | 15.0  |
| Minimum Split (s)            | 61.5  | 57.5  | 57.5     | 23.5  |
| Total Split (s)              | 61.5  | 58.5  | 58.5     | 58.5  |
| Total Split (%)              | 51.3% | 48.8% | 48.8%    | 48.8% |
| Yellow Time (s)              | 3.5   | 3.5   | 3.5      | 3.5   |
| All-Red Time (s)             | 2.0   | 2.0   | 2.0      | 2.0   |
| Lost Time Adjust (s)         | 0.0   | 0.0   | 0.0      | 0.0   |
| Total Lost Time (s)          | 5.5   | 5.5   | 5.5      | 5.5   |
| Lead/Lag                     | 0.0   | 0.0   | 0.0      | 0.0   |
| Lead-Lag Optimize?           |       |       |          |       |
| Recall Mode                  | None  | C-Min | C-Min    | C-Min |
| v/c Ratio                    | 0.70  | 0.14  | 0.42     | 0.31  |
| Control Delay                | 48.6  | 4.3   | 5.7      | 4.5   |
| Queue Delay                  | 0.0   | 0.0   | 0.0      | 0.0   |
| Total Delay                  | 48.6  | 4.3   | 5.7      | 4.5   |
| Queue Length 50th (ft)       | 92    | 16    | 125      | 70    |
| Queue Length 95th (ft)       | 159   | 41    | 237      | 141   |
| Internal Link Dist (ft)      | 1475  | 41    | 446      | 669   |
|                              | 14/5  | 50    | 440      | 009   |
| Turn Bay Length (ft)         | 010   |       | 1// [    | 1/02  |
| Base Capacity (vph)          | 819   | 730   | 1465     | 1402  |
| Starvation Cap Reductn       | 0     | 0     | 0        | 0     |
| Spillback Cap Reductn        | 0     | 0     | 0        | 0     |
| Storage Cap Reductn          | 0     | 0     | 0        | 0     |
| Reduced v/c Ratio            | 0.22  | 0.14  | 0.42     | 0.31  |
| Intersection Summary         |       |       |          |       |
| Cycle Length: 120            |       |       |          |       |
| Astronta d Corola Lameth 120 |       |       |          |       |

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 120

Control Type: Actuated-Coordinated

Splits and Phases: 4: N.Indian Creek Dr & Indian Creek Way



|                              | ۶    | •    | •         | <b>†</b> | ļ    | 4    |
|------------------------------|------|------|-----------|----------|------|------|
| Movement                     | EBL  | EBR  | NBL       | NBT      | SBT  | SBR  |
| Lane Configurations          | W    |      | ች         | <b></b>  | 1    |      |
| Traffic Volume (veh/h)       | 86   | 84   | 101       | 592      | 268  | 145  |
| Future Volume (veh/h)        | 86   | 84   | 101       | 592      | 268  | 145  |
| Initial Q (Qb), veh          | 0    | 0    | 0         | 0        | 0    | 0    |
| Ped-Bike Adj(A_pbT)          | 1.00 | 1.00 | 1.00      |          |      | 1.00 |
| Parking Bus, Adj             | 1.00 | 1.00 | 1.00      | 1.00     | 1.00 | 1.00 |
| Work Zone On Approach        | No   |      |           | No       | No   |      |
| Adj Sat Flow, veh/h/ln       | 1870 | 1870 | 1870      | 1870     | 1870 | 1870 |
| Adj Flow Rate, veh/h         | 90   | 88   | 105       | 617      | 279  | 151  |
| Peak Hour Factor             | 0.96 | 0.96 | 0.96      | 0.96     | 0.96 | 0.96 |
| Percent Heavy Veh, %         | 2    | 2    | 2         | 2        | 2    | 2    |
| Cap, veh/h                   | 106  | 104  | 743       | 1464     | 894  | 484  |
| Arrive On Green              | 0.13 | 0.13 | 0.78      | 0.78     | 0.78 | 0.78 |
| Sat Flow, veh/h              | 844  | 826  | 958       | 1870     | 1141 | 618  |
| Grp Volume(v), veh/h         | 179  |      | 105       | 617      |      | 430  |
|                              |      | 0    |           |          | 0    |      |
| Grp Sat Flow(s), veh/h/ln    | 1680 | 0    | 958       | 1870     | 0    | 1759 |
| Q Serve(g_s), s              | 12.5 | 0.0  | 4.2       | 12.8     | 0.0  | 8.4  |
| Cycle Q Clear(g_c), s        | 12.5 | 0.0  | 12.7      | 12.8     | 0.0  | 8.4  |
| Prop In Lane                 | 0.50 | 0.49 | 1.00      | 444      |      | 0.35 |
| Lane Grp Cap(c), veh/h       | 211  | 0    | 743       | 1464     | 0    | 1377 |
| V/C Ratio(X)                 | 0.85 | 0.00 | 0.14      | 0.42     | 0.00 | 0.31 |
| Avail Cap(c_a), veh/h        | 784  | 0    | 743       | 1464     | 0    | 1377 |
| HCM Platoon Ratio            | 1.00 | 1.00 | 1.00      | 1.00     | 1.00 | 1.00 |
| Upstream Filter(I)           | 1.00 | 0.00 | 1.00      | 1.00     | 0.00 | 1.00 |
| Uniform Delay (d), s/veh     | 51.4 | 0.0  | 5.6       | 4.2      | 0.0  | 3.7  |
| Incr Delay (d2), s/veh       | 9.1  | 0.0  | 0.4       | 0.9      | 0.0  | 0.6  |
| Initial Q Delay(d3),s/veh    | 0.0  | 0.0  | 0.0       | 0.0      | 0.0  | 0.0  |
| %ile BackOfQ(50%),veh/ln     | 5.8  | 0.0  | 0.8       | 4.2      | 0.0  | 2.6  |
| Unsig. Movement Delay, s/veh | 1    |      |           |          |      |      |
| LnGrp Delay(d),s/veh         | 60.5 | 0.0  | 6.0       | 5.1      | 0.0  | 4.3  |
| LnGrp LOS                    | Ε    | Α    | Α         | Α        | Α    | Α    |
| Approach Vol, veh/h          | 179  |      |           | 722      | 430  |      |
| Approach Delay, s/veh        | 60.5 |      |           | 5.2      | 4.3  |      |
| Approach LOS                 | E    |      |           | A        | Α.5  |      |
| •                            |      |      |           |          | ,,   |      |
| Timer - Assigned Phs         |      | 2    |           | 4        |      | 6    |
| Phs Duration (G+Y+Rc), s     |      | 99.4 |           | 20.6     |      | 99.4 |
| Change Period (Y+Rc), s      |      | 5.5  |           | 5.5      |      | 5.5  |
| Max Green Setting (Gmax), s  |      | 53.0 |           | 56.0     |      | 53.0 |
| Max Q Clear Time (g_c+l1), s |      | 14.8 |           | 14.5     |      | 10.4 |
| Green Ext Time (p_c), s      |      | 5.2  |           | 0.6      |      | 3.0  |
| Intersection Summary         |      |      |           |          |      |      |
| HCM 6th Ctrl Delay           |      |      | 12.4      |          |      |      |
| HCM 6th LOS                  |      |      | 12.4<br>B |          |      |      |
| TICIVI OUI LUS               |      |      | Ď         |          |      |      |
| Notes                        |      |      |           |          |      |      |

User approved volume balancing among the lanes for turning movement.

| Interception   |           |          |                |                     |                                    |             |
|--|-----------|----------|----------------|---------------------|------------------------------------|-------------|
| Intersection Int Delay, s/veh  | 2.1       |          |                |                     |                                    |             |
|  |           |          |                |                     |                                    |             |
| Movement   | WBL       | WBR      | NBT            | NBR                 | SBL                                | SBT         |
| Lane Configurations  | ¥         |          | ₽              |                     |                                    | ની          |
| Traffic Vol, veh/h   | 10        | 30       | 82             | 3                   | 9                                  | 75          |
| Future Vol, veh/h  | 10        | 30       | 82             | 3                   | 9                                  | 75          |
| Conflicting Peds, #/hr   | 0         | 0        | 0              | 0                   | 0                                  | 0           |
| Sign Control   | Stop      | Stop     | Free           | Free                | Free                               | Free        |
| RT Channelized   | -         | None     | -              | None                | -                                  | None        |
| Storage Length   | 0         | -        | -              | -                   | -                                  | -           |
| Veh in Median Storage,   | , # 0     | -        | 0              | -                   | -                                  | 0           |
| Grade, %   | 0         | -        | 0              | -                   | -                                  | 0           |
| Peak Hour Factor   | 92        | 92       | 92             | 92                  | 92                                 | 92          |
| Heavy Vehicles, %  | 2         | 2        | 2              | 2                   | 2                                  | 2           |
| Mvmt Flow  | 11        | 33       | 89             | 3                   | 10                                 | 82          |
| WWIIICTIOW   |           | 00       | 07             | J                   | 10                                 | 02          |
|  |           |          |                |                     |                                    |             |
| Major/Minor N  | /linor1   |          | /lajor1        | 1                   | Major2                             |             |
| Conflicting Flow All   | 193       | 91       | 0              | 0                   | 92                                 | 0           |
| Stage 1  | 91        | -        | -              | -                   | -                                  | -           |
| Stage 2  | 102       | -        | -              | -                   | -                                  | -           |
| Critical Hdwy  | 6.42      | 6.22     | -              | -                   | 4.12                               | -           |
| Critical Hdwy Stg 1  | 5.42      | -        | -              | -                   | -                                  | -           |
| Critical Hdwy Stg 2  | 5.42      | -        | -              | -                   | -                                  | -           |
|  | 3.518     | 3.318    | _              | -                   | 2.218                              | -           |
| Pot Cap-1 Maneuver   | 796       | 967      | -              | -                   | 1503                               | -           |
| Stage 1  | 933       | -        | _              | _                   | -                                  | _           |
| Stage 2  | 922       | _        | _              | _                   | _                                  | _           |
| Platoon blocked, %   | ,,,       |          | _              | _                   |                                    | _           |
| Mov Cap-1 Maneuver   | 790       | 967      |                |                     | 1503                               | _           |
| •  | 790       | 907      |                |                     | 1003                               | -           |
| Mov Cap-2 Maneuver   |           |          | -              | -                   | -                                  | -           |
| Stage 1  | 933       | -        | -              | -                   | -                                  | -           |
|  |           | -        | -              | -                   | -                                  | -           |
| Stage 2  | 916       | _        |                |                     |                                    |             |
| Stage 2  | 910       | -        |                |                     |                                    |             |
| Stage 2 Approach   | WB        |          | NB             |                     | SB                                 |             |
| Approach   | WB        |          | NB<br>0        |                     |                                    |             |
|  |           |          |                |                     | SB<br>0.8                          |             |
| Approach HCM Control Delay, s  | WB<br>9.1 |          |                |                     |                                    |             |
| Approach HCM Control Delay, s HCM LOS  | 9.1<br>A  |          | 0              | A/DI 1              | 0.8                                | CDT         |
| Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt  | 9.1<br>A  | NBT      | 0<br>NBRV      | WBLn1               | 0.8<br>SBL                         | SBT         |
| Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h)  | 9.1<br>A  |          | 0<br>NBRV      | 916                 | 0.8<br>SBL<br>1503                 | -           |
| Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio                       | 9.1<br>A  |          | 0<br>NBRV      | 916<br>0.047        | 0.8<br>SBL<br>1503<br>0.007        | -           |
| Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvml Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) | 9.1<br>A  | NBT<br>- | 0<br>NBRV      | 916<br>0.047<br>9.1 | 0.8<br>SBL<br>1503<br>0.007<br>7.4 | -<br>-<br>0 |
| Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio                       | 9.1<br>A  | NBT<br>- | 0<br>NBRV<br>- | 916<br>0.047        | 0.8<br>SBL<br>1503<br>0.007        | -           |

| Intersection  |           |          |          |                   |                      |             |
|---|-----------|----------|----------|-------------------|----------------------|-------------|
| Int Delay, s/veh  | 1.6       |          |          |                   |                      |             |
|   |           | 14/55    |          | NES               | 051                  | 05=         |
| Movement  | WBL       | WBR      | NBT      | NBR               | SBL                  | SBT         |
| Lane Configurations   | ¥         |          | <b>₽</b> |                   |                      | र्स         |
| Traffic Vol, veh/h  | 7         | 20       | 65       | 2                 | 6                    | 79          |
| Future Vol, veh/h   | 7         | 20       | 65       | 2                 | 6                    | 79          |
| Conflicting Peds, #/hr  | 0         | 0        | 0        | 0                 | 0                    | 0           |
| Sign Control  | Stop      | Stop     | Free     | Free              | Free                 | Free        |
| RT Channelized  | -         | None     | -        | None              | -                    | None        |
| Storage Length  | 0         | -        | -        | -                 | -                    | -           |
| Veh in Median Storage   | e, # 0    | -        | 0        | -                 | -                    | 0           |
| Grade, %  | 0         | -        | 0        | -                 | -                    | 0           |
| Peak Hour Factor  | 92        | 92       | 92       | 92                | 92                   | 92          |
| Heavy Vehicles, %   | 2         | 2        | 2        | 2                 | 2                    | 2           |
| Mvmt Flow   | 8         | 22       | 71       | 2                 | 7                    | 86          |
|   |           |          |          |                   |                      |             |
| N.A. i. a. a/N.Aira.  | N A!      |          | 1-1-1    |                   | Malana               |             |
|   | Minor1    |          | //ajor1  |                   | Major2               |             |
| Conflicting Flow All  | 172       | 72       | 0        | 0                 | 73                   | 0           |
| Stage 1   | 72        | -        | -        | -                 | -                    | -           |
| Stage 2   | 100       | -        | -        | -                 | -                    | -           |
| Critical Hdwy   | 6.42      | 6.22     | -        | -                 | 4.12                 | -           |
| Critical Hdwy Stg 1   | 5.42      | -        | -        | -                 | -                    | -           |
| Critical Hdwy Stg 2   | 5.42      | -        | -        | -                 | -                    | -           |
| Follow-up Hdwy  | 3.518     | 3.318    | -        | -                 | 2.218                | -           |
| Pot Cap-1 Maneuver  | 818       | 990      | -        | -                 | 1527                 | -           |
| Stage 1   | 951       | -        | -        | -                 | -                    | -           |
| Stage 2   | 924       | -        | -        | -                 | -                    | -           |
| Platoon blocked, %  |           |          | _        | -                 |                      | -           |
| Mov Cap-1 Maneuver  | 814       | 990      | _        | _                 | 1527                 | -           |
| Mov Cap-2 Maneuver  | 814       | -        | _        | _                 | -                    | _           |
| Stage 1   | 951       | _        | _        | _                 | _                    | _           |
| Stage 2   | 919       | _        |          |                   | _                    |             |
| Jiage 2   | /17       | -        |          |                   |                      | -           |
|   |           |          |          |                   |                      |             |
| Approach  | WB        |          | NB       |                   | SB                   |             |
| HCM Control Delay, s  | 9         |          | 0        |                   | 0.5                  |             |
|   | Α         |          |          |                   |                      |             |
| HCM LOS   |           |          |          |                   |                      |             |
| HCM LOS   |           |          |          |                   |                      |             |
|   |           | NDT      | NDDV     | N/D1 ∞1           | CDI                  | CDT         |
| Minor Lane/Major Mvn  |           | NBT      |          | WBLn1             | SBL                  | SBT         |
| Minor Lane/Major Mvm<br>Capacity (veh/h)  |           | -        | -        | 937               | 1527                 | -           |
| Minor Lane/Major Mvm<br>Capacity (veh/h)<br>HCM Lane V/C Ratio                          | nt        | NBT<br>- | -        | 937<br>0.031      | 1527<br>0.004        | -           |
| Minor Lane/Major Mvn<br>Capacity (veh/h)<br>HCM Lane V/C Ratio<br>HCM Control Delay (s) | nt        | -        | -        | 937<br>0.031<br>9 | 1527<br>0.004<br>7.4 | -<br>-<br>0 |
| Minor Lane/Major Mvm<br>Capacity (veh/h)<br>HCM Lane V/C Ratio                          | <u>nt</u> | -        | -        | 937<br>0.031      | 1527<br>0.004        | -           |

| Intersection           |          |         |        |      |        |       |
|------------------------|----------|---------|--------|------|--------|-------|
| Int Delay, s/veh       | 3.3      |         |        |      |        |       |
| Movement               | EBT      | EBR     | WBL    | WBT  | NBL    | NBR   |
| Lane Configurations    | <b>†</b> | 7       |        | 4    | *      | 7     |
| Traffic Vol, veh/h     | 257      | 98      | 168    | 300  | 28     | 100   |
| Future Vol, veh/h      | 257      | 98      | 168    | 300  | 28     | 100   |
| Conflicting Peds, #/hr | 0        | 0       | 0      | 0    | 0      | 0     |
| Sign Control           | Free     | Free    | Free   | Free | Stop   | Stop  |
| RT Channelized         | -        | None    | -      | None | -      | None  |
| Storage Length         | _        | 270     | -      | -    | 0      | 30    |
| Veh in Median Storage  | , # 0    | -       | -      | 0    | 0      | -     |
| Grade, %               | 0        | _       | _      | 0    | 0      | _     |
| Peak Hour Factor       | 91       | 91      | 91     | 91   | 91     | 91    |
| Heavy Vehicles, %      | 2        | 2       | 2      | 2    | 2      | 2     |
| Mvmt Flow              | 282      | 108     | 185    | 330  | 31     | 110   |
| IVIVIIIL FIOW          | 202      | 100     | 100    | 330  | JI     | 110   |
|                        |          |         |        |      |        |       |
| Major/Minor N          | Najor1   | ľ       | Major2 | N    | Minor1 |       |
| Conflicting Flow All   | 0        | 0       | 390    | 0    | 982    | 282   |
| Stage 1                | -        | -       | -      | -    | 282    | -     |
| Stage 2                | -        | -       | -      | -    | 700    | -     |
| Critical Hdwy          | -        | -       | 4.12   | -    | 6.42   | 6.22  |
| Critical Hdwy Stg 1    | -        | -       | -      | -    | 5.42   | -     |
| Critical Hdwy Stg 2    | -        | -       | -      | -    | 5.42   | -     |
| Follow-up Hdwy         | _        | -       | 2.218  | -    | 3.518  | 3.318 |
| Pot Cap-1 Maneuver     | _        | -       | 1169   | -    | 276    | 757   |
| Stage 1                | _        | _       | -      | _    | 766    | -     |
| Stage 2                | _        | _       | _      | _    | 493    | _     |
| Platoon blocked, %     | _        | _       |        | _    | 770    |       |
| Mov Cap-1 Maneuver     | -        | _       | 1169   | _    | 222    | 757   |
| Mov Cap-1 Maneuver     |          | -       | - 1107 | -    | 222    | 131   |
|                        | -        | _       |        |      | 766    |       |
| Stage 1                | -        | -       | -      | -    |        | -     |
| Stage 2                | -        | -       | -      | -    | 397    | -     |
|                        |          |         |        |      |        |       |
| Approach               | EB       |         | WB     |      | NB     |       |
| HCM Control Delay, s   | 0        |         | 3.1    |      | 13.5   |       |
| HCM LOS                | Ū        |         | 0.1    |      | В      |       |
|                        |          |         |        |      |        |       |
|                        |          |         |        |      |        |       |
| Minor Lane/Major Mvm   | t I      | NBLn1 N | VBLn2  | EBT  | EBR    | WBL   |
| Capacity (veh/h)       |          | 222     | 757    | -    | -      | 1169  |
| HCM Lane V/C Ratio     |          | 0.139   | 0.145  | -    | -      | 0.158 |
| HCM Control Delay (s)  |          | 23.8    | 10.6   | -    | -      | 8.7   |
| HCM Lane LOS           |          | С       | В      | -    | -      | Α     |
| HCM 95th %tile Q(veh)  |          | 0.5     | 0.5    | -    | -      | 0.6   |
|                        |          |         |        |      |        |       |

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| Intersection                    |           |       |          |       |        |             |
|---------------------------------|-----------|-------|----------|-------|--------|-------------|
| Int Delay, s/veh                | 4         |       |          |       |        |             |
|                                 |           | WIDD  | NDT      | NDD   | CDI    | CDT         |
| Movement                        | WBL       | WBR   | NBT      | NBR   | SBL    | SBT         |
| Lane Configurations             | ¥         | F-7   | <b>^</b> | 40    | 75     | ्र <b>ी</b> |
| Traffic Vol, veh/h              | 57        | 57    | 81       | 43    | 75     | 165         |
| Future Vol, veh/h               | 57        | 57    | 81       | 43    | 75     | 165         |
| Conflicting Peds, #/hr          | 0         | 0     | 0        | 0     | 0      | 0           |
| Sign Control                    | Stop      | Stop  | Free     | Free  | Free   | Free        |
| RT Channelized                  | -         | None  | -        | None  | -      | None        |
| Storage Length                  | 0         | -     | -        | -     | -      | -           |
| Veh in Median Storage           |           | -     | 0        | -     | -      | 0           |
| Grade, %                        | 0         | -     | 0        | -     | -      | 0           |
| Peak Hour Factor                | 86        | 86    | 86       | 86    | 86     | 86          |
| Heavy Vehicles, %               | 2         | 2     | 2        | 2     | 2      | 2           |
| Mvmt Flow                       | 66        | 66    | 94       | 50    | 87     | 192         |
|                                 |           |       |          |       |        |             |
| Major/Minor I                   | Minor1    | N     | /lajor1  |       | Major2 |             |
| Conflicting Flow All            | 485       | 119   | 0        | 0     | 144    | 0           |
| Stage 1                         | 119       | 117   | -        | U     | 144    | -           |
| Stage 2                         | 366       | -     | -        | -     | -      | -           |
| Critical Hdwy                   | 6.42      | 6.22  |          | -     | 4.12   |             |
|                                 | 5.42      | 0.22  | -        | -     | 4.12   | -           |
| Critical Hdwy Stg 1             | 5.42      |       | -        | -     | -      |             |
| Critical Hdwy Stg 2             |           | 2 210 | -        | -     | 2 210  | -           |
| Follow-up Hdwy                  | 3.518     | 3.318 | -        | -     | 2.218  | -           |
| Pot Cap-1 Maneuver              | 541       | 933   | -        | -     | 1438   | -           |
| Stage 1                         | 906       | -     | -        | -     | -      | -           |
| Stage 2                         | 702       | -     | -        | -     | -      | -           |
| Platoon blocked, %              |           |       | -        | -     |        | -           |
| Mov Cap-1 Maneuver              | 504       | 933   | -        | -     | 1438   | -           |
| Mov Cap-2 Maneuver              | 504       | -     | -        | -     | -      | -           |
| Stage 1                         | 906       | -     | -        | -     | -      | -           |
| Stage 2                         | 654       | -     | -        | -     | -      | -           |
|                                 |           |       |          |       |        |             |
| Approach                        | WB        |       | NB       |       | SB     |             |
|                                 | 11.9      |       | 0        |       | 2.4    |             |
| HCM Control Delay, s<br>HCM LOS | 11.9<br>B |       | U        |       | 2.4    |             |
| HCIVI LU3                       | D         |       |          |       |        |             |
|                                 |           |       |          |       |        |             |
| Minor Lane/Major Mvm            | nt        | NBT   | NBRV     | VBLn1 | SBL    | SBT         |
| Capacity (veh/h)                |           | -     | -        | 654   | 1438   | -           |
| HCM Lane V/C Ratio              |           | -     | -        | 0.203 |        | -           |
| HCM Control Delay (s)           |           | -     | -        |       | 7.7    | 0           |
| HCM Lane LOS                    |           | -     | -        | В     | Α      | A           |
| HCM 95th %tile Q(veh            | )         | -     | -        | 0.8   | 0.2    | -           |
|                                 |           |       |          |       |        |             |

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|                         | •     | <b>→</b> | •          | -     |
|-------------------------|-------|----------|------------|-------|
| Lane Group              | EBL   | EBT      | WBT        | SBL   |
| Lane Configurations     | ች     | <b>^</b> | <b>∱</b> } | W     |
| Traffic Volume (vph)    | 82    | 895      | 397        | 73    |
| Future Volume (vph)     | 82    | 895      | 397        | 73    |
| Lane Group Flow (vph)   | 86    | 942      | 461        | 185   |
| Turn Type               | Perm  | NA       | NA         | Prot  |
| Protected Phases        |       | 2        | 6          | 4     |
| Permitted Phases        | 2     |          |            |       |
| Detector Phase          | 2     | 2        | 6          | 4     |
| Switch Phase            |       |          |            |       |
| Minimum Initial (s)     | 15.0  | 15.0     | 15.0       | 6.0   |
| Minimum Split (s)       | 23.5  | 23.5     | 23.5       | 30.5  |
| Total Split (s)         | 76.0  | 76.0     | 76.0       | 44.0  |
| Total Split (%)         | 63.3% | 63.3%    | 63.3%      | 36.7% |
| Yellow Time (s)         | 3.5   | 3.5      | 3.5        | 3.5   |
| All-Red Time (s)        | 2.0   | 2.0      | 2.0        | 2.0   |
| Lost Time Adjust (s)    | 0.0   | 0.0      | 0.0        | 0.0   |
| Total Lost Time (s)     | 5.5   | 5.5      | 5.5        | 5.5   |
| Lead/Lag                |       |          |            |       |
| Lead-Lag Optimize?      |       |          |            |       |
| Recall Mode             | C-Min | C-Min    | C-Min      | None  |
| v/c Ratio               | 0.12  | 0.34     | 0.17       | 0.72  |
| Control Delay           | 4.2   | 4.5      | 3.6        | 48.4  |
| Queue Delay             | 0.0   | 0.0      | 0.0        | 0.0   |
| Total Delay             | 4.2   | 4.5      | 3.6        | 48.4  |
| Queue Length 50th (ft)  | 13    | 91       | 36         | 93    |
| Queue Length 95th (ft)  | 34    | 155      | 66         | 162   |
| Internal Link Dist (ft) |       | 586      | 931        | 2806  |
| Turn Bay Length (ft)    | 100   |          |            |       |
| Base Capacity (vph)     | 714   | 2786     | 2751       | 581   |
| Starvation Cap Reductn  | 0     | 0        | 0          | 0     |
| Spillback Cap Reductn   | 0     | 0        | 0          | 0     |
| Storage Cap Reductn     | 0     | 0        | 0          | 0     |
| Reduced v/c Ratio       | 0.12  | 0.34     | 0.17       | 0.32  |
| Intersection Summary    |       |          |            |       |

### **Intersection Summary**

Cycle Length: 120

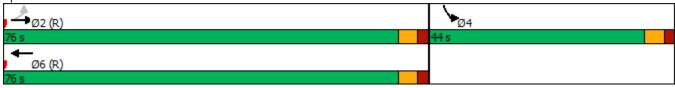
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Splits and Phases: 3: N. Decatur Rd & Northern Ave



|                              | ၨ    | <b>→</b> | •    | •    | <b>\</b> | 4     |
|------------------------------|------|----------|------|------|----------|-------|
| Movement                     | EBL  | EBT      | WBT  | WBR  | SBL      | SBR   |
| Lane Configurations          |      | <b>^</b> | ħβ   |      | */*      |       |
| Traffic Volume (veh/h)       | 82   | 895      | 397  | 41   | 73       | 103   |
| Future Volume (veh/h)        | 82   | 895      | 397  | 41   | 73       | 103   |
| Initial Q (Qb), veh          | 0    | 0        | 0    | 0    | 0        | 0     |
| Ped-Bike Adj(A_pbT)          | 1.00 |          |      | 1.00 | 1.00     | 1.00  |
| Parking Bus, Adj             | 1.00 | 1.00     | 1.00 | 1.00 | 1.00     | 1.00  |
| Work Zone On Approach        |      | No       | No   |      | No       |       |
| Adj Sat Flow, veh/h/ln       | 1870 | 1870     | 1870 | 1870 | 1870     | 1870  |
| Adj Flow Rate, veh/h         | 86   | 942      | 418  | 43   | 77       | 0     |
| Peak Hour Factor             | 0.95 | 0.95     | 0.95 | 0.95 | 0.95     | 0.95  |
| Percent Heavy Veh, %         | 2    | 2        | 2    | 2    | 2        | 2     |
| Cap, veh/h                   | 832  | 3026     | 2771 | 284  | 100      |       |
| Arrive On Green              | 0.85 | 0.85     | 0.85 | 0.85 | 0.06     | 0.00  |
| Sat Flow, veh/h              | 931  | 3647     | 3348 | 333  | 1760     | 0     |
| Grp Volume(v), veh/h         | 86   | 942      | 227  | 234  | 78       | 0     |
| Grp Sat Flow(s),veh/h/ln     | 931  | 1777     | 1777 | 1810 | 1782     | 0     |
| Q Serve(g_s), s              | 2.1  | 6.4      | 2.6  | 2.6  | 5.2      | 0.0   |
| Cycle Q Clear(g_c), s        | 4.7  | 6.4      | 2.6  | 2.6  | 5.2      | 0.0   |
| Prop In Lane                 | 1.00 |          |      | 0.18 | 0.99     | 0.00  |
| Lane Grp Cap(c), veh/h       | 832  | 3026     | 1513 | 1542 | 101      |       |
| V/C Ratio(X)                 | 0.10 | 0.31     | 0.15 | 0.15 | 0.77     |       |
| Avail Cap(c_a), veh/h        | 832  | 3026     | 1513 | 1542 | 572      |       |
| HCM Platoon Ratio            | 1.00 | 1.00     | 1.00 | 1.00 | 1.00     | 1.00  |
| Upstream Filter(I)           | 1.00 | 1.00     | 1.00 | 1.00 | 1.00     | 0.00  |
| Uniform Delay (d), s/veh     | 1.9  | 1.8      | 1.5  | 1.5  | 55.8     | 0.0   |
| Incr Delay (d2), s/veh       | 0.2  | 0.3      | 0.2  | 0.2  | 11.6     | 0.0   |
| Initial Q Delay(d3),s/veh    | 0.0  | 0.0      | 0.0  | 0.0  | 0.0      | 0.0   |
| %ile BackOfQ(50%),veh/ln     | 0.3  | 1.2      | 0.5  | 0.5  | 2.6      | 0.0   |
| Unsig. Movement Delay, s/veh |      |          |      |      |          |       |
| LnGrp Delay(d),s/veh         | 2.2  | 2.1      | 1.7  | 1.7  | 67.4     | 0.0   |
| LnGrp LOS                    | А    | А        | А    | А    | E        |       |
| Approach Vol, veh/h          |      | 1028     | 461  |      | 78       | Α     |
| Approach Delay, s/veh        |      | 2.1      | 1.7  |      | 67.4     |       |
| Approach LOS                 |      | A        | А    |      | E        |       |
| Timer - Assigned Phs         |      | 2        |      | 4    |          | 6     |
| Phs Duration (G+Y+Rc), s     |      | 107.7    |      | 12.3 |          | 107.7 |
| Change Period (Y+Rc), s      |      | 5.5      |      | 5.5  |          | 5.5   |
| Max Green Setting (Gmax), s  |      | 70.5     |      | 38.5 |          | 70.5  |
| Max Q Clear Time (g_c+l1), s |      | 8.4      |      | 7.2  |          | 4.6   |
| Green Ext Time (p_c), s      |      | 18.9     |      | 0.2  |          | 6.0   |
| · ·                          |      | 10.7     |      | 0.2  |          | 0.0   |
| Intersection Summary         |      |          |      |      |          |       |
| HCM 6th Ctrl Delay           |      |          | 5.2  |      |          |       |
| HCM 6th LOS                  |      |          | Α    |      |          |       |
|                              |      |          |      |      |          |       |

User approved volume balancing among the lanes for turning movement.
Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

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|                         | •     | 4     | <b>†</b> | ļ     |
|-------------------------|-------|-------|----------|-------|
| Lane Group              | EBL   | NBL   | NBT      | SBT   |
| Lane Configurations     | ¥     | ሻ     | <b></b>  | 1>    |
| Traffic Volume (vph)    | 68    | 96    | 530      | 776   |
| Future Volume (vph)     | 68    | 96    | 530      | 776   |
| Lane Group Flow (vph)   | 182   | 97    | 535      | 850   |
| Turn Type               | Prot  | Perm  | NA       | NA    |
| Protected Phases        | 4     |       | 2        | 6     |
| Permitted Phases        |       | 2     |          |       |
| Detector Phase          | 4     | 2     | 2        | 6     |
| Switch Phase            |       |       |          |       |
| Minimum Initial (s)     | 6.0   | 15.0  | 15.0     | 15.0  |
| Minimum Split (s)       | 61.5  | 57.5  | 57.5     | 23.5  |
| Total Split (s)         | 61.5  | 58.5  | 58.5     | 58.5  |
| Total Split (%)         | 51.3% | 48.8% | 48.8%    | 48.8% |
| Yellow Time (s)         | 3.5   | 3.5   | 3.5      | 3.5   |
| All-Red Time (s)        | 2.0   | 2.0   | 2.0      | 2.0   |
| Lost Time Adjust (s)    | 0.0   | 0.0   | 0.0      | 0.0   |
| Total Lost Time (s)     | 5.5   | 5.5   | 5.5      | 5.5   |
| Lead/Lag                | 2,0   | 2.0   | 2.0      | 2.0   |
| Lead-Lag Optimize?      |       |       |          |       |
| Recall Mode             | None  | C-Min | C-Min    | C-Min |
| v/c Ratio               | 0.71  | 0.22  | 0.36     | 0.57  |
| Control Delay           | 39.6  | 5.0   | 4.5      | 6.8   |
| Queue Delay             | 0.0   | 0.0   | 0.0      | 0.0   |
| Total Delay             | 39.6  | 5.0   | 4.5      | 6.8   |
| Queue Length 50th (ft)  | 67    | 14    | 90       | 188   |
| Queue Length 95th (ft)  | 138   | 42    | 180      | 376   |
| Internal Link Dist (ft) | 1475  | 12    | 446      | 669   |
| Turn Bay Length (ft)    | 1170  | 50    | 110      | 007   |
| Base Capacity (vph)     | 830   | 435   | 1497     | 1482  |
| Starvation Cap Reductn  | 0     | 0     | 0        | 0     |
| Spillback Cap Reductn   | 0     | 0     | 0        | 0     |
| Storage Cap Reductn     | 0     | 0     | 0        | 0     |
| Reduced v/c Ratio       | 0.22  | 0.22  | 0.36     | 0.57  |
|                         | 0.22  | 0.22  | 0.50     | 0.57  |
| Intersection Summary    |       |       |          |       |
| Cycle Length: 120       |       |       |          |       |

Cycle Length: 120

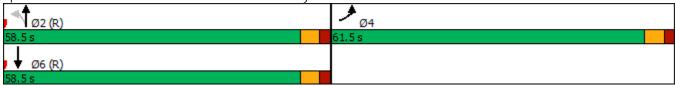
Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 140

Control Type: Actuated-Coordinated

Splits and Phases: 4: N.Indian Creek Dr & Indian Creek Way



|                               | ၨ         | •    | 4        | <b>†</b> | ļ        | 4     |
|-------------------------------|-----------|------|----------|----------|----------|-------|
| Movement                      | EBL       | EBR  | NBL      | NBT      | SBT      | SBR   |
| Lane Configurations           | W         |      | ሻ        | <b>†</b> | 7        |       |
| Traffic Volume (veh/h)        | 68        | 112  | 96       | 530      | 776      | 65    |
| Future Volume (veh/h)         | 68        | 112  | 96       | 530      | 776      | 65    |
| Initial Q (Qb), veh           | 0         | 0    | 0        | 0        | 0        | 0     |
| Ped-Bike Adj(A_pbT)           | 1.00      | 1.00 | 1.00     |          |          | 1.00  |
| Parking Bus, Adj              | 1.00      | 1.00 | 1.00     | 1.00     | 1.00     | 1.00  |
| Work Zone On Approach         | No        | 1.00 | 1.00     | No       | No       | 1.00  |
| Adj Sat Flow, veh/h/ln        | 1870      | 1870 | 1870     | 1870     | 1870     | 1870  |
| Adj Flow Rate, veh/h          | 69        | 113  | 97       | 535      | 784      | 66    |
| Peak Hour Factor              | 0.99      | 0.99 | 0.99     | 0.99     | 0.99     | 0.99  |
| Percent Heavy Veh, %          | 2         | 2    | 2        | 2        | 2        | 2     |
|                               | 81        |      | 442      |          | 1325     | 112   |
| Cap, veh/h<br>Arrive On Green |           | 132  |          | 1456     |          | 0.78  |
|                               | 0.13      | 0.13 | 0.78     | 0.78     | 0.78     |       |
| Sat Flow, veh/h               | 624       | 1022 | 649      | 1870     | 1701     | 143   |
| Grp Volume(v), veh/h          | 183       | 0    | 97       | 535      | 0        | 850   |
| Grp Sat Flow(s), veh/h/ln     | 1655      | 0    | 649      | 1870     | 0        | 1845  |
| Q Serve(g_s), s               | 13.0      | 0.0  | 8.7      | 10.6     | 0.0      | 22.7  |
| Cycle Q Clear(g_c), s         | 13.0      | 0.0  | 31.4     | 10.6     | 0.0      | 22.7  |
| Prop In Lane                  | 0.38      | 0.62 | 1.00     |          |          | 0.08  |
| Lane Grp Cap(c), veh/h        | 215       | 0    | 442      | 1456     | 0        | 1436  |
| V/C Ratio(X)                  | 0.85      | 0.00 | 0.22     | 0.37     | 0.00     | 0.59  |
| Avail Cap(c_a), veh/h         | 772       | 0    | 442      | 1456     | 0        | 1436  |
| HCM Platoon Ratio             | 1.00      | 1.00 | 1.00     | 1.00     | 1.00     | 1.00  |
| Upstream Filter(I)            | 1.00      | 0.00 | 1.00     | 1.00     | 0.00     | 1.00  |
| Uniform Delay (d), s/veh      | 51.1      | 0.0  | 11.9     | 4.1      | 0.0      | 5.4   |
| Incr Delay (d2), s/veh        | 9.2       | 0.0  | 1.1      | 0.7      | 0.0      | 1.8   |
| Initial Q Delay(d3),s/veh     | 0.0       | 0.0  | 0.0      | 0.0      | 0.0      | 0.0   |
| %ile BackOfQ(50%),veh/ln      | 6.0       | 0.0  | 1.3      | 3.5      | 0.0      | 7.4   |
| Unsig. Movement Delay, s/veh  |           | 3.0  |          | 3.0      | 3.0      | , , , |
| LnGrp Delay(d),s/veh          | 60.3      | 0.0  | 13.0     | 4.8      | 0.0      | 7.2   |
| LnGrp LOS                     | E         | Α    | В        | Α.       | Α        | Α.2   |
| Approach Vol, veh/h           | 183       | /\   | <u> </u> | 632      | 850      |       |
|                               | 60.3      |      |          | 6.1      | 7.2      |       |
| Approach LOS                  | 00.3<br>F |      |          |          | 7.2<br>A |       |
| Approach LOS                  | E         |      |          | А        | А        |       |
| Timer - Assigned Phs          |           | 2    |          | 4        |          | 6     |
| Phs Duration (G+Y+Rc), s      |           | 98.9 |          | 21.1     |          | 98.9  |
| Change Period (Y+Rc), s       |           | 5.5  |          | 5.5      |          | 5.5   |
| Max Green Setting (Gmax), s   |           | 53.0 |          | 56.0     |          | 53.0  |
| Max Q Clear Time (q_c+l1), s  |           | 33.4 |          | 15.0     |          | 24.7  |
| Green Ext Time (p_c), s       |           | 4.2  |          | 0.6      |          | 7.1   |
|                               |           | 1.2  |          | 0.0      |          | 7.1   |
| Intersection Summary          |           |      |          |          |          |       |
| HCM 6th Ctrl Delay            |           |      | 12.6     |          |          |       |
| HCM 6th LOS                   |           |      | В        |          |          |       |
| Notes                         |           |      |          |          |          |       |

User approved volume balancing among the lanes for turning movement.

| Intersection           |        |       |        |       |        |      |
|------------------------|--------|-------|--------|-------|--------|------|
| Int Delay, s/veh       | 1.9    |       |        |       |        |      |
| Movement               | WBL    | WBR   | NBT    | NBR   | SBL    | SBT  |
| Lane Configurations    | W      |       | ĵ.     |       |        | 4    |
| Traffic Vol, veh/h     | 6      | 18    | 69     | 10    | 31     | 95   |
| Future Vol, veh/h      | 6      | 18    | 69     | 10    | 31     | 95   |
| Conflicting Peds, #/hr | 0      | 0     | 0      | 0     | 0      | 0    |
| Sign Control           | Stop   | Stop  | Free   | Free  | Free   | Free |
| RT Channelized         | -<br>- | None  | -      | None  | -      | None |
| Storage Length         | 0      | -     | _      | -     | _      | -    |
| Veh in Median Storage  |        | _     | 0      | _     | -      | 0    |
| Grade, %               | 0      | _     | 0      | _     | _      | 0    |
| Peak Hour Factor       | 92     | 92    | 92     | 92    | 92     | 92   |
| Heavy Vehicles, %      | 2      | 2     | 2      | 2     | 2      | 2    |
| Mymt Flow              | 7      | 20    | 75     | 11    | 34     | 103  |
| IVIVIIIL I IOW         | ,      | 20    | 13     | - 11  | J4     | 103  |
|                        |        |       |        |       |        |      |
|                        | Minor1 |       | Major1 |       | Major2 |      |
| Conflicting Flow All   | 252    | 81    | 0      | 0     | 86     | 0    |
| Stage 1                | 81     | -     | -      | -     | -      | -    |
| Stage 2                | 171    | -     | -      | -     | -      | -    |
| Critical Hdwy          | 6.42   | 6.22  | -      | -     | 4.12   | -    |
| Critical Hdwy Stg 1    | 5.42   | -     | -      | -     | -      | -    |
| Critical Hdwy Stg 2    | 5.42   | -     | -      | -     | -      | -    |
| Follow-up Hdwy         | 3.518  | 3.318 | -      | -     | 2.218  | -    |
| Pot Cap-1 Maneuver     | 737    | 979   | -      | -     | 1510   | -    |
| Stage 1                | 942    | -     | -      | -     | -      | -    |
| Stage 2                | 859    | -     | -      | -     | -      | -    |
| Platoon blocked, %     |        |       | -      | -     |        | -    |
| Mov Cap-1 Maneuver     | 719    | 979   | -      | -     | 1510   | -    |
| Mov Cap-2 Maneuver     | 719    | -     | -      | _     | -      | -    |
| Stage 1                | 942    | -     | -      | -     | -      | -    |
| Stage 2                | 838    | _     | _      | _     | _      | _    |
| Olago Z                | 000    |       |        |       |        |      |
|                        |        |       |        |       |        |      |
| Approach               | WB     |       | NB     |       | SB     |      |
| HCM Control Delay, s   | 9.1    |       | 0      |       | 1.8    |      |
| HCM LOS                | Α      |       |        |       |        |      |
|                        |        |       |        |       |        |      |
| Minor Lane/Major Mvm   | nt     | NBT   | NBRV   | WBLn1 | SBL    | SBT  |
| Capacity (veh/h)       |        | _     |        | 898   | 1510   |      |
| HCM Lane V/C Ratio     |        | _     | _      | 0.029 |        | -    |
| HCM Control Delay (s)  |        | _     | _      | 9.1   | 7.4    | 0    |
| HCM Lane LOS           |        | _     | _      | A     | Α      | A    |
| HCM 95th %tile Q(veh)  | )      | _     | _      | 0.1   | 0.1    | -    |
| TOWN JOHN JOHN CELVEN  | ,      |       |        | 0.1   | J. I   |      |

| Intersection           |           |       |        |        |          |          |
|------------------------|-----------|-------|--------|--------|----------|----------|
| Int Delay, s/veh       | 1.5       |       |        |        |          |          |
| Movement               | WBL       | WBR   | NBT    | NBR    | SBL      | SBT      |
| Lane Configurations    | ₩.        | אטוע  | 1\D1   | NDI    | JUL      | <u></u>  |
| Traffic Vol, veh/h     | 4         | 12    | 67     | 7      | 20       | 81       |
| Future Vol, veh/h      | 4         | 12    | 67     | 7      | 20       | 81       |
| Conflicting Peds, #/hr | 0         | 0     | 0      | 0      | 0        | 0        |
| Sign Control           | Stop      | Stop  | Free   | Free   | Free     | Free     |
| RT Channelized         | Jiop<br>- | None  | -      | None   | -        | None     |
| Storage Length         | 0         | -     | _      | -      | _        | - INOIIC |
| Veh in Median Storage  |           | _     | 0      | _      | _        | 0        |
| Grade, %               | 0         | _     | 0      | _      | _        | 0        |
| Peak Hour Factor       | 92        | 92    | 92     | 92     | 92       | 92       |
| Heavy Vehicles, %      | 2         | 2     | 2      | 2      | 2        | 2        |
| Mymt Flow              | 4         | 13    | 73     | 8      | 22       | 88       |
| IVIVIIIL FIUW          | 4         | 13    | 13     | 0      | ZZ       | 00       |
|                        |           |       |        |        |          |          |
| Major/Minor N          | Minor1    | N     | Major1 | 1      | Major2   |          |
| Conflicting Flow All   | 209       | 77    | 0      | 0      | 81       | 0        |
| Stage 1                | 77        | -     | -      | -      | -        | -        |
| Stage 2                | 132       | -     | -      | -      | -        | -        |
| Critical Hdwy          | 6.42      | 6.22  | -      | -      | 4.12     | -        |
| Critical Hdwy Stg 1    | 5.42      | -     | -      | -      | -        | -        |
| Critical Hdwy Stg 2    | 5.42      | -     | -      | -      | -        | -        |
| Follow-up Hdwy         | 3.518     | 3.318 | -      | -      | 2.218    | -        |
| Pot Cap-1 Maneuver     | 779       | 984   | -      | -      | 1517     | -        |
| Stage 1                | 946       | -     | -      | -      | -        | -        |
| Stage 2                | 894       | -     | -      | -      | -        | -        |
| Platoon blocked, %     |           |       | -      | -      |          | -        |
| Mov Cap-1 Maneuver     | 767       | 984   | -      | _      | 1517     | -        |
| Mov Cap-2 Maneuver     | 767       | -     | _      | -      | -        | _        |
| Stage 1                | 946       | _     | _      | _      | _        | -        |
| Stage 2                | 881       | _     | _      | _      | _        | _        |
| Stage 2                | 001       |       |        |        |          |          |
|                        |           |       |        |        |          |          |
| Approach               | WB        |       | NB     |        | SB       |          |
| HCM Control Delay, s   | 9         |       | 0      |        | 1.5      |          |
| HCM LOS                | Α         |       |        |        |          |          |
|                        |           |       |        |        |          |          |
| Minor Lane/Major Mvm   | nt        | NBT   | NRRV   | WBLn1  | SBL      | SBT      |
| Capacity (veh/h)       | IL        | וטוו  | -      |        | 1517     | JD1<br>- |
| HCM Lane V/C Ratio     |           | -     |        | 0.019  |          | -        |
| HCM Control Delay (s)  |           | -     | -      | 0.019  | 7.4      | 0        |
| HCM Lane LOS           |           | -     | -      | 9<br>A | 7.4<br>A | A        |
| HCM 95th %tile Q(veh)  | 1         | -     | -      | 0.1    | 0        | - A      |
| HOW FOUT WITH Q(VEH)   |           | _     |        | 0.1    | U        |          |



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#### 1. Church St @ Northern Ave

#### A.M. Peak Hour

|   |    | North | nern Ave |     |  |       |       |   | Church Street |   |     |    |     |  | Church Street |     |   |     |  |
|---|----|-------|----------|-----|--|-------|-------|---|---------------|---|-----|----|-----|--|---------------|-----|---|-----|--|
|   |    | Nort  | hbound   |     |  | South | bound |   | Eastbound     |   |     |    |     |  | Westbound     |     |   |     |  |
| Condition                                   | L  | T     | R        | Tot |  | L     | T     | R | Tot           | L | T   | R  | Tot |  | L             | T   | R | Tot |  |
| 2020 Volumes from 18-120 Project Build:     | 0  | 0     | 0        | 0   |  | 0     | 0     | 0 | 0             | 0 | 0   | 0  | 0   |  | 0             | 0   | 0 | 0   |  |
| 2021 Counts during Covid-19:                | 31 | 0     | 67       | 98  |  | 0     | 0     | 0 | 0             | 0 | 119 | 21 | 140 |  | 55            | 153 | 0 | 208 |  |
| Adjusted / Projected Existing 2021 Volumes: | 53 | 0     | 114      | 167 |  | 0     | 0     | 0 | 0             | 0 | 202 | 36 | 238 |  | 94            | 260 | 0 | 354 |  |
| Growth Factor (%):                          | 1  | 1     | 1        |     |  | 1     | 1     | 1 |               | 1 | 1   | 1  |     |  | 1             | 1   | 1 |     |  |
| No-Build 2023 Volumes:                      | 54 | 0     | 116      | 170 |  | 0     | 0     | 0 | 0             | 0 | 206 | 37 | 243 |  | 96            | 265 | 0 | 361 |  |
| Total New Trips:                            | 10 | 0     | 13       | 0   |  | 0     | 0     | 0 | 0             | 0 | 0   | 3  | 3   |  | 4             | 0   | 0 | 4   |  |
| Future 2023 Traffic Volumes:                | 64 | 0     | 129      | 193 |  | 0     | 0     | 0 | 0             | 0 | 206 | 40 | 246 |  | 100           | 265 | 0 | 365 |  |

|   |    | North | ern Ave |     |   |       | -     |     |   | Churcl | h Street |     | Church Street |     |   |     |  |
|---|----|-------|---------|-----|---|-------|-------|-----|---|--------|----------|-----|---------------|-----|---|-----|--|
|   |    | Nort  | hbound  |     |   | South | bound |     |   | Eastb  | ound     |     | Westbound     |     |   |     |  |
| Condition                                   | L  | T     | R       | Tot | L | T     | R     | Tot | L | T      | R        | Tot | L             | T   | R | Tot |  |
| 2020 Volumes from 18-120 Project Build:     | 0  | 0     | 0       | 0   | 0 | 0     | 0     | 0   | 0 | 0      | 0        | 0   | 0             | 0   | 0 | 0   |  |
| 2021 Counts during Covid-19:                | 19 | 0     | 78      | 97  | 0 | 0     | 0     | 0   | 0 | 219    | 75       | 294 | 131           | 256 | 0 | 387 |  |
| Adjusted / Projected Existing 2021 Volumes: | 22 | 0     | 90      | 112 | 0 | 0     | 0     | 0   | 0 | 252    | 86       | 338 | 151           | 294 | 0 | 445 |  |
| Growth Factor (%):                          | 1  | 1     | 1       |     | 1 | 1     | 1     |     | 1 | 1      | 1        |     | 1             | 1   | 1 |     |  |
| No-Build 2023 Volumes:                      | 22 | 0     | 92      | 114 | 0 | 0     | 0     | 0   | 0 | 257    | 88       | 345 | 154           | 300 | 0 | 454 |  |
| Total New Trips:                            | 6  | 0     | 8       | 0   | 0 | 0     | 0     | 0   | 0 | 0      | 10       | 10  | 14            | 0   | 0 | 14  |  |
| Future 2023 Traffic Volumes:                | 28 | 0     | 100     | 128 | 0 | 0     | 0     | 0   | 0 | 257    | 98       | 355 | 168           | 300 | 0 | 468 |  |

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#### 2. Northern Ave @ Indian Creek

#### A.M. Peak Hour

|   |   | North | ern Ave |     |    | North | ern Ave |       |      |   | -         |     | Indian Creek Way |   |    |     |  |
|---|---|-------|---------|-----|----|-------|---------|-------|------|---|-----------|-----|------------------|---|----|-----|--|
|   |   | North | bound   |     |    |       |         | Eastl | ound |   | Westbound |     |                  |   |    |     |  |
| Condition                                   | L | T     | R       | Tot | L  | T     | R       | Tot   | L    | T | R         | Tot | L                | T | R  | Tot |  |
| 2020 Volumes from 18-120 Project Build:     | 0 | 0     | 0       | 0   | o  | 0     | 0       | 0     | 0    | 0 | 0         | 0   | 0                | 0 | 0  | 0   |  |
| 2021 Counts during Covid-19:                | 0 | 44    | 8       | 52  | 40 | 37    | 0       | 77    | 0    | 0 | 0         | 0   | 13               | 0 | 36 | 49  |  |
| Adjusted / Projected Existing 2021 Volumes: | 0 | 75    | 14      | 89  | 68 | 63    | 0       | 131   | 0    | 0 | 0         | 0   | 22               | 0 | 61 | 83  |  |
| Growth Factor (%):                          | 1 | 1     | 1       |     | 1  | 1     | 1       |       | 1    | 1 | 1         |     | 1                | 1 | 1  |     |  |
| No-Build 2023 Volumes:                      | 0 | 77    | 14      | 91  | 69 | 64    | 0       | 133   | 0    | 0 | 0         | 0   | 22               | 0 | 62 | 84  |  |
| Total New Trips:                            | 0 | 23    | 27      | 50  | 0  | 7     | 0       | 7     | 0    | 0 | 0         | 0   | 8                | 0 | 0  | 8   |  |
| Future 2023 Traffic Volumes:                | 0 | 100   | 41      | 141 | 69 | 71    | 0       | 140   | 0    | 0 | 0         | 0   | 30               | 0 | 62 | 92  |  |

|   |   | North | ern Ave |     |    | Northe | rn Ave |     |   |       | -    |     | Indian Creek Way |   |    |     |  |
|---|---|-------|---------|-----|----|--------|--------|-----|---|-------|------|-----|------------------|---|----|-----|--|
|   |   | North | bound   |     |    | South  | bound  |     |   | Eastl | ound |     | Westbound        |   |    |     |  |
| Condition                                   | L | T     | R       | Tot | L  | T      | R      | Tot | L | T     | R    | Tot | L                | T | R  | Tot |  |
| 2020 Volumes from 18-120 Project Build:     | 0 | 0     | 0       | 0   | 0  | 0      | 0      | 0   | 0 | 0     | 0    | 0   | 0                | 0 | 0  | 0   |  |
| 2021 Counts during Covid-19:                | 0 | 57    | 23      | 80  | 64 | 120    | 0      | 184 | 0 | 0     | 0    | 0   | 25               | 0 | 49 | 74  |  |
| Adjusted / Projected Existing 2021 Volumes: | 0 | 66    | 26      | 92  | 74 | 138    | 0      | 212 | 0 | 0     | 0    | 0   | 29               | 0 | 56 | 85  |  |
| Growth Factor (%):                          | 1 | 1     | 1       |     | 1  | 1      | 1      |     | 1 | 1     | 1    |     | 1                | 1 | 1  |     |  |
| No-Build 2023 Volumes:                      | 0 | 67    | 27      | 94  | 75 | 141    | 0      | 216 | 0 | 0     | 0    | 0   | 30               | 0 | 57 | 87  |  |
| Total New Trips:                            | 0 | 14    | 16      | 30  | 0  | 24     | 0      | 24  | 0 | 0     | 0    | 0   | 27               | 0 | 0  | 27  |  |
| Future 2023 Traffic Volumes:                | 0 | 81    | 43      | 124 | 75 | 165    | 0      | 240 | 0 | 0     | 0    | 0   | 57               | 0 | 57 | 114 |  |

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#### 3. N. Decatur Rd @ Northern Ave

#### A.M. Peak Hour

|   |   |       | -      |     |    |       | ern Ave |     |    | N.Decat |      | 1   |   | N.Deca |       | 1   |
|---|---|-------|--------|-----|----|-------|---------|-----|----|---------|------|-----|---|--------|-------|-----|
|   |   | Nortl | hbound |     |    | South | ibound  |     |    | Eastb   | ound |     |   | West   | bound |     |
| Condition                                   | L | T     | R      | Tot | L  | T     | R       | Tot | L  | T       | R    | Tot | L | T      | R     | Tot |
| 2020 Volumes from 18-120 Project Build:     | 0 | 0     | 0      | 0   | 0  | 0     | 0       | 0   | 0  | 0       | 0    | 0   | 0 | 0      | 0     | 0   |
| 2021 Counts during Covid-19:                | 0 | 0     | 0      | 0   | 14 | 0     | 66      | 80  | 25 | 189     | 0    | 214 | 0 | 412    | 9     | 421 |
| Adjusted / Projected Existing 2021 Volumes: | 0 | 0     | 0      | 0   | 24 | 0     | 112     | 136 | 43 | 321     | 0    | 364 | 0 | 700    | 15    | 715 |
| Growth Factor (%):                          | 1 | 1     | 1      |     | 1  | 1     | 1       |     | 1  | 1       | 1    |     | 1 | 1      | 1     |     |
| No-Build 2023 Volumes:                      | 0 | 0     | 0      | 0   | 24 | 0     | 114     | 138 | 44 | 327     | 0    | 371 | 0 | 714    | 15    | 729 |
| Total New Trips:                            | 0 | 0     | 0      | 0   | 7  | 0     | 10      | 17  | 3  | 0       | 0    | 3   | 0 | 0      | 2     | 2   |
| Future 2023 Traffic Volumes:                | 0 | 0     | 0      | 0   | 31 | 0     | 124     | 155 | 47 | 327     | 0    | 374 | 0 | 714    | 17    | 731 |

|   |   |       | -      |     |    | North | ern Ave |     |    | N.Decat | ur Roa | d   |   | N.Deca | tur Road | 1   |
|---|---|-------|--------|-----|----|-------|---------|-----|----|---------|--------|-----|---|--------|----------|-----|
|   |   | North | hbound |     |    | South | bound   |     |    | Eastb   | ound   |     |   | West   | bound    |     |
| Condition                                   | L | T     | R      | Tot | L  | T     | R       | Tot | L  | T       | R      | Tot | L | T      | R        | Tot |
| 2020 Volumes from 18-120 Project Build:     | 0 | 0     | 0      | 0   | 0  | 0     | 0       | 0   | 0  | 0       | 0      | 0   | 0 | 0      | 0        | 0   |
| 2021 Counts during Covid-19:                | 0 | 0     | 0      | 0   | 59 | 0     | 83      | 142 | 62 | 763     | 0      | 825 | 0 | 338    | 29       | 367 |
| Adjusted / Projected Existing 2021 Volumes: | 0 | 0     | 0      | 0   | 68 | 0     | 95      | 163 | 71 | 877     | 0      | 948 | 0 | 389    | 33       | 422 |
| Growth Factor (%):                          | 1 | 1     | 1      |     | 1  | 1     | 1       |     | 1  | 1       | 1      |     | 1 | 1      | 1        |     |
| No-Build 2023 Volumes:                      | 0 | 0     | 0      | 0   | 69 | 0     | 97      | 166 | 72 | 895     | 0      | 967 | 0 | 397    | 34       | 431 |
| Total New Trips:                            | 0 | 0     | 0      | 0   | 4  | 0     | 6       | 10  | 10 | 0       | 0      | 10  | 0 | 0      | 7        | 7   |
| Future 2023 Traffic Volumes:                | 0 | 0     | 0      | 0   | 73 | 0     | 103     | 176 | 82 | 895     | 0      | 977 | 0 | 397    | 41       | 438 |

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#### 4. N. Indian @ Indian Creek

#### A.M. Peak Hour

|   |     | N.Indian | Creek l | Dr  | ] | N.Indian | Creek I | )r  | Inc |   | Creek Wa<br>bound | ıy  |   | Was | -<br>tbound |     |
|---|-----|----------|---------|-----|---|----------|---------|-----|-----|---|-------------------|-----|---|-----|-------------|-----|
| Condition                                   | L   | T        | R       | Tot | L | T        | R       | Tot | L   | T | R                 | Tot | L | T   | R           | Tot |
| 2020 Volumes from 18-120 Project Build:     | 95  | 574      | 0       | 669 | 0 | 260      | 136     | 396 | 67  | 0 | 72                | 139 | 0 | 0   | 0           | 0   |
| 2021 Counts during Covid-19:                | 0   | 0        | 0       | 0   | 0 | 0        | 0       | 0   | 0   | 0 | 0                 | 0   | 0 | 0   | 0           | 0   |
| Adjusted / Projected Existing 2021 Volumes: | 96  | 580      | 0       | 676 | 0 | 263      | 137     | 400 | 68  | 0 | 73                | 141 | 0 | 0   | 0           | 0   |
| Growth Factor (%):                          | 1   | 1        | 1       |     | 1 | 1        | 1       |     | 1   | 1 | 1                 |     | 1 | 1   | 1           |     |
| No-Build 2023 Volumes:                      | 98  | 592      | 0       | 690 | 0 | 268      | 140     | 408 | 69  | 0 | 74                | 143 | 0 | 0   | 0           | 0   |
| Total New Trips:                            | 3   | 0        | 0       | 3   | 0 | 0        | 5       | 5   | 17  | 0 | 10                | 27  | 0 | 0   | 0           | 0   |
| Future 2023 Traffic Volumes:                | 101 | 592      | 0       | 693 | 0 | 268      | 145     | 413 | 86  | 0 | 84                | 170 | 0 | 0   | 0           | 0   |

|   |    | N.Indian | Creek | Dr  |   | N.Indian | Creek I | Or  | Inc | dian ( | Creek Wa | y   |   |     | -      |     |
|---|----|----------|-------|-----|---|----------|---------|-----|-----|--------|----------|-----|---|-----|--------|-----|
|   |    | North    | bound |     |   | South    | bound   |     |     | East   | bound    |     |   | Wes | tbound |     |
| Condition                                   | L  | T        | R     | Tot | L | T        | R       | Tot | L   | T      | R        | Tot | L | T   | R      | Tot |
| 2020 Volumes from 18-120 Project Build:     | 83 | 515      | 0     | 598 | 0 | 753      | 47      | 800 | 56  | 0      | 103      | 159 | 0 | 0   | 0      | 0   |
| 2021 Counts during Covid-19:                | 0  | 0        | 0     | 0   | 0 | 0        | 0       | 0   | 0   | 0      | 0        | 0   | 0 | 0   | 0      | 0   |
| Adjusted / Projected Existing 2021 Volumes: | 84 | 520      | 0     | 604 | 0 | 761      | 47      | 808 | 57  | 0      | 104      | 161 | 0 | 0   | 0      | 0   |
| Growth Factor (%):                          | 1  | 1        | 1     |     | 1 | 1        | 1       |     | 1   | 1      | 1        |     | 1 | 1   | 1      |     |
| No-Build 2023 Volumes:                      | 86 | 530      | 0     | 616 | 0 | 776      | 48      | 824 | 58  | 0      | 106      | 164 | 0 | 0   | 0      | 0   |
| Total New Trips:                            | 10 | 0        | 0     | 10  | 0 | 0        | 17      | 17  | 10  | 0      | 6        | 16  | 0 | 0   | 0      | 0   |
| Future 2023 Traffic Volumes:                | 96 | 530      | 0     | 626 | 0 | 776      | 65      | 841 | 68  | 0      | 112      | 180 | 0 | 0   | 0      | 0   |

A&R Engineering January 2021

5. Nothern Ave @ Drwy 1 (N)

#### A.M. Peak Hour

|   |   |    | ern Ave |     |   |    | rn Ave<br>bound |     |   | Facth | -<br>ound |     | Sit |   | eway 1 (I | N)  |
|---|---|----|---------|-----|---|----|-----------------|-----|---|-------|-----------|-----|-----|---|-----------|-----|
| Condition                                   | L | T  | R       | Tot | L | T  | R               | Tot | L | T     | R         | Tot | L   | T | R         | Tot |
| 2020 Volumes from 18-120 Project Build:     | 0 | 0  | 0       | 0   | 0 | 0  | 0               | 0   | 0 | 0     | 0         | 0   | 0   | 0 | 0         | 0   |
| 2021 Counts during Covid-19:                | 0 | 36 | 0       | 36  | 0 | 40 | 0               | 40  | 0 | 0     | 0         | 0   | 0   | 0 | 0         | 0   |
| Adjusted / Projected Existing 2021 Volumes: | 0 | 61 | 0       | 61  | 0 | 68 | 0               | 68  | 0 | 0     | 0         | 0   | 0   | 0 | 0         | 0   |
| Growth Factor (%):                          | 1 | 1  | 1       |     | 1 | 1  | 1               |     | 1 | 1     | 1         |     | 1   | 1 | 1         |     |
| No-Build 2023 Volumes:                      | 0 | 62 | 0       | 62  | 0 | 69 | 0               | 69  | 0 | 0     | 0         | 0   | 0   | 0 | 0         | 0   |
| Total New Trips:                            | 0 | 20 | 3       | 0   | 9 | 6  | 0               | 15  | 0 | 0     | 0         | 0   | 10  | 0 | 30        | 40  |
| Future 2023 Traffic Volumes:                | 0 | 82 | 3       | 85  | 9 | 75 | 0               | 84  | 0 | 0     | 0         | 0   | 10  | 0 | 30        | 40  |

|   |   | North | ern Ave |     |    | Northe | rn Ave |     |   |       |      |                                 |   | Site Dr | iveway 1 ( | (N) |
|---|---|-------|---------|-----|----|--------|--------|-----|---|-------|------|---------------------------------|---|---------|------------|-----|
|   |   | North | bound   |     |    | South  | bound  |     |   | Eastb | ound | 8<br>8<br>8<br>8<br>8<br>8<br>8 |   | We      | estbound   |     |
| Condition                                   | L | T     | R       | Tot | L  | T      | R      | Tot | L | T     | R    | Tot                             | L | T       | R          | Tot |
| 2020 Volumes from 18-120 Project Build:     | 0 | 0     | 0       | 0   | 0  | 0      | 0      | 0   | 0 | 0     | 0    | 0                               | 0 | 0       | 0          | 0   |
| 2021 Counts during Covid-19:                | 0 | 49    | 0       | 49  | 0  | 64     | 0      | 64  | 0 | 0     | 0    | 0                               | 0 | 0       | 0          | 0   |
| Adjusted / Projected Existing 2021 Volumes: | 0 | 56    | 0       | 56  | 0  | 74     | 0      | 74  | 0 | 0     | 0    | 0                               | 0 | 0       | 0          | 0   |
| Growth Factor (%):                          | 1 | 1     | 1       |     | 1  | 1      | 1      |     | 1 | 1     | 1    |                                 | 1 | 1       | 1          |     |
| No-Build 2023 Volumes:                      | 0 | 57    | 0       | 57  | 0  | 75     | 0      | 75  | 0 | 0     | 0    | 0                               | 0 | 0       | 0          | 0   |
| Total New Trips:                            | 0 | 12    | 10      | 22  | 31 | 20     | 0      | 51  | 0 | 0     | 0    | 0                               | 6 | 0       | 18         | 24  |
| Future 2023 Traffic Volumes:                | 0 | 69    | 10      | 79  | 31 | 95     | 0      | 126 | 0 | 0     | 0    | 0                               | 6 | 0       | 18         | 24  |

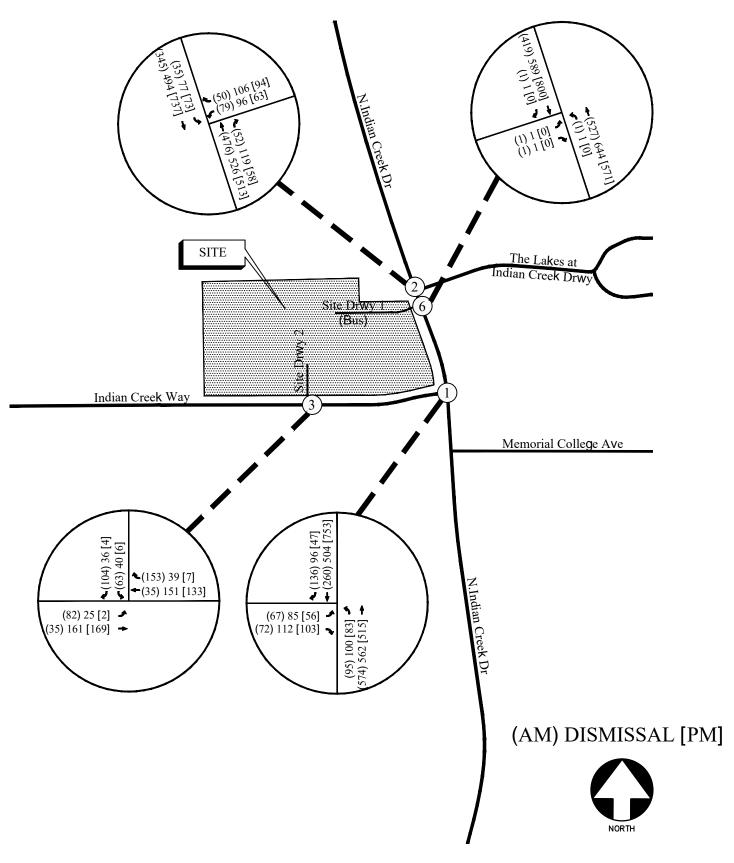
A&R Engineering January 2021

6. Northern Ave @ Drwy 2 (S)

#### A.M. Peak Hour

|   |   |    | ern Ave |     |   | Northe<br>South | rn Ave |     |   | Eastl | -<br>ound |     | Si |   | eway 2 ( | S)  |
|---|---|----|---------|-----|---|-----------------|--------|-----|---|-------|-----------|-----|----|---|----------|-----|
| Condition                                   | L | Т  | R       | Tot | L | T               | R      | Tot | L | T     | R         | Tot | L  | T | R        | Tot |
| 2020 Volumes from 18-120 Project Build:     | 0 | 0  | 0       | 0   | 0 | 0               | 0      | 0   | 0 | 0     | 0         | 0   | 0  | 0 | 0        | 0   |
| 2021 Counts during Covid-19:                | 0 | 36 | 0       | 36  | 0 | 40              | 0      | 40  | 0 | 0     | 0         | 0   | 0  | 0 | 0        | 0   |
| Adjusted / Projected Existing 2021 Volumes: | 0 | 61 | 0       | 61  | 0 | 68              | 0      | 68  | 0 | 0     | 0         | 0   | 0  | 0 | 0        | 0   |
| Growth Factor (%):                          | 1 | 1  | 1       |     | 1 | 1               | 1      |     | 1 | 1     | 1         |     | 1  | 1 | 1        |     |
| No-Build 2023 Volumes:                      | 0 | 62 | 0       | 62  | 0 | 69              | 0      | 69  | 0 | 0     | 0         | 0   | 0  | 0 | 0        | 0   |
| Total New Trips:                            | 0 | 3  | 2       | 5   | 6 | 10              | 0      | 16  | 0 | 0     | 0         | 0   | 7  | 0 | 20       | 27  |
| Future 2023 Traffic Volumes:                | 0 | 65 | 2       | 67  | 6 | 79              | 0      | 85  | 0 | 0     | 0         | 0   | 7  | 0 | 20       | 27  |

|   |   | Northe | rn Ave |     |    | Northe | rn Ave |     |   |       | -    |     |   | Site D | riveway 2 | (S) |
|---|---|--------|--------|-----|----|--------|--------|-----|---|-------|------|-----|---|--------|-----------|-----|
|   |   | North  | bound  |     |    | South  | bound  |     |   | Eastl | ound |     |   | W      | estbound  |     |
| Condition                                   | L | T      | R      | Tot | L  | T      | R      | Tot | L | T     | R    | Tot | L | T      | R         | Tot |
| 2020 Volumes from 18-120 Project Build:     | 0 | 0      | 0      | 0   | 0  | 0      | 0      | 0   | 0 | 0     | 0    | 0   | 0 | 0      | 0         | 0   |
| 2021 Counts during Covid-19:                | 0 | 49     | 0      | 49  | 0  | 64     | 0      | 64  | 0 | 0     | 0    | 0   | 0 | 0      | 0         | 0   |
| Adjusted / Projected Existing 2021 Volumes: | 0 | 56     | 0      | 56  | 0  | 74     | 0      | 74  | 0 | 0     | 0    | 0   | 0 | 0      | 0         | 0   |
| Growth Factor (%):                          | 1 | 1      | 1      |     | 1  | 1      | 1      |     | 1 | 1     | 1    |     | 1 | 1      | 1         |     |
| No-Build 2023 Volumes:                      | 0 | 57     | 0      | 57  | 0  | 75     | 0      | 75  | 0 | 0     | 0    | 0   | 0 | 0      | 0         | 0   |
| Total New Trips:                            | 0 | 10     | 7      | 17  | 20 | 6      | 0      | 26  | 0 | 0     | 0    | 0   | 4 | 0      | 12        | 16  |
| Future 2023 Traffic Volumes:                | 0 | 67     | 7      | 74  | 20 | 81     | 0      | 101 | 0 | 0     | 0    | 0   | 4 | 0      | 12        | 16  |



FUTURE (BUILD) WEEKDAY PEAK HOUR VOLUMES

FIGURE 6

A&R Engineering Inc.

A&R Engineering October 2018

#### 1. Indian Creek @ Indian Creek

#### A.M. Peak Hour (7am - 8am)

|                                      | N  | Indian C |       |     | N | I. Indian ( |       | rive | Ir  |       | reek Wa | ıy  |   |      | -     |     |
|--------------------------------------|----|----------|-------|-----|---|-------------|-------|------|-----|-------|---------|-----|---|------|-------|-----|
|                                      |    | North    | bound |     |   | South       | bound |      |     | Easth | ound    |     |   | West | bound |     |
| Condition                            | L  | T        | R     | Tot | L | T           | R     | Tot  | L   | T     | R       | Tot | L | T    | R     | Tot |
| Existing 2018 Volumes:               | 78 | 562      | 0     | 640 | 0 | 250         | 20    | 270  | 89  | 0     | 67      | 156 | 0 | 0    | 0     | 0   |
| Growth Factor (%):                   | 2  | 2        | 2     |     | 2 | 2           | 2     |      | 2   | 2     | 2       |     | 2 | 2    | 2     |     |
| No-Build 2020 Volumes:               | 81 | 585      | 0     | 666 | 0 | 260         | 21    | 281  | 93  | 0     | 70      | 163 | 0 | 0    | 0     | 0   |
| New Car Trips:                       | 3  | 0        | 0     | 3   | 0 | 0           | 22    | 22   | 16  | 0     | 2       | 18  | 0 | 0    | 0     | 0   |
| New Bus Trips:                       | 0  | 0        | 0     | 0   | 0 | 0           | 1     | 1    | 1   | 0     | 0       | 1   | 0 | 0    | 0     | 0   |
| Total New Trips:                     | 3  | 0        | 0     | 3   | 0 | 0           | 23    | 23   | 17  | 0     | 2       | 19  | 0 | 0    | 0     | 0   |
| Redistributed Existing School Trips: | 11 | -11      | 0     | 0   | 0 | 0           | 92    | 92   | -43 | 0     | 0       | -43 | 0 | 0    | 0     | 0   |
| Future 2020 Volumes:                 | 95 | 574      | 0     | 669 | 0 | 260         | 136   | 396  | 67  | 0     | 72      | 139 | 0 | 0    | 0     | 0   |

#### School Dismissal Peak Hour (2pm - 3pm)

|                                      | N   | . Indian ( | Creek I | Prive | N | Indian ( | Creek D | rive | Ir | dian ( | Creek Wa | y   |   |      | -     |     |
|--------------------------------------|-----|------------|---------|-------|---|----------|---------|------|----|--------|----------|-----|---|------|-------|-----|
|                                      |     | North      | bound   |       |   | South    | bound   |      |    | East   | tbound   |     |   | West | bound |     |
| Condition                            | L   | T          | R       | Tot   | L | T        | R       | Tot  | L  | T      | R        | Tot | L | T    | R     | Tot |
| Existing 2018 Volumes:               | 94  | 541        | 0       | 635   | 0 | 484      | 76      | 560  | 78 | 0      | 107      | 185 | 0 | 0    | 0     | 0   |
| Growth Factor (%):                   | 2   | 2          | 2       |       | 2 | 2        | 2       |      | 2  | 2      | 2        |     | 2 | 2    | 2     |     |
| No-Build 2020 Volumes:               | 98  | 563        | 0       | 661   | 0 | 504      | 79      | 583  | 81 | 0      | 111      | 192 | 0 | 0    | 0     | 0   |
| New Car Trips:                       | 1   | 0          | 0       | 1     | 0 | 0        | 6       | 6    | 7  | 0      | 1        | 8   | 0 | 0    | 0     | 0   |
| New Bus Trips:                       | 0   | 0          | 0       | 0     | 0 | 0        | 1       | 1    | 1  | 0      | 0        | 1   | 0 | 0    | 0     | 0   |
| Total New Trips:                     | 1   | 0          | 0       | 1     | 0 | 0        | 7       | 7    | 8  | 0      | 1        | 9   | 0 | 0    | 0     | 0   |
| Redistributed Existing School Trips: | 1   | -1         | 0       | 0     | 0 | 0        | 10      | 10   | -4 | 0      | 0        | -4  | 0 | 0    | 0     | 0   |
| Future 2020 Volumes:                 | 100 | 562        | 0       | 662   | 0 | 504      | 96      | 600  | 85 | 0      | 112      | 197 | 0 | 0    | 0     | 0   |

#### P.M. Peak Hour (4pm - 6pm)

|                                      | N  | . Indian (<br><b>North</b> | Creek D<br><b>bound</b> |     | N | . Indian (<br>South | Creek D<br>bound | rive | Ir |   | Creek Wa<br>bound | ny  |   | West | -<br>bound |     |
|--------------------------------------|----|----------------------------|-------------------------|-----|---|---------------------|------------------|------|----|---|-------------------|-----|---|------|------------|-----|
| Condition                            | L  | T                          | R                       | Tot | L | T                   | R                | Tot  | L  | T | R                 | Tot | L | T    | R          | Tot |
| Existing 2018 Volumes:               | 80 | 495                        | 0                       | 575 | 0 | 725                 | 42               | 767  | 54 | 0 | 98                | 152 | 0 | 0    | 0          | 0   |
| Growth Factor (%):                   | 2  | 2                          | 2                       |     | 2 | 2                   | 2                |      | 2  | 2 | 2                 |     | 2 | 2    | 2          |     |
| No-Build 2020 Volumes:               | 83 | 515                        | 0                       | 598 | 0 | 754                 | 44               | 798  | 56 | 0 | 102               | 158 | 0 | 0    | 0          | 0   |
| New Car Trips:                       | 0  | 0                          | 0                       | 0   | 0 | 0                   | 1                | 1    | 1  | 0 | 0                 | 1   | 0 | 0    | 0          | 0   |
| New Bus Trips:                       | 0  | 0                          | 0                       | 0   | 0 | 0                   | 0                | 0    | 0  | 0 | 0                 | 0   | 0 | 0    | 0          | 0   |
| Total New Trips:                     | 0  | 0                          | 0                       | 0   | 0 | 0                   | 1                | 1    | 1  | 0 | 0                 | 1   | 0 | 0    | 0          | 0   |
| Redistributed Existing School Trips: | 0  | 0                          | 0                       | 0   | 0 | -1                  | 2                | 1    | -1 | 0 | 1                 | 0   | 0 | 0    | 0          | 0   |
| Future 2020 Volumes:                 | 83 | 515                        | 0                       | 598 | 0 | 753                 | 47               | 800  | 56 | 0 | 103               | 159 | 0 | 0    | 0          | 0   |



# DeKalb County Department of Planning & Sustainability

Michael L. Thurmond Chief Executive Officer Andrew A. Baker, AICP Director



### APPLICATION TO AMEND OFFICIAL ZONING MAP OF DEKALB COUNTY, GEORGIA

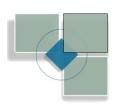
|   |   |   | Z/C                              | Z No  | 2             |
|---|---|---|----------------------------------|---|---------------|
| D-4- D'                                       | and the same  | A . P . P   | KT.                              | Z No  |               |
| Date Receive                                  | ed:   | Application   | on No.:                          |   |               |
| Applicant: _                                  | Inline Communities LLC  | c/o Battle Law PC   | E-Mail:                          | mlb@battlelawpc.com   |               |
|   | iling Address:<br>One West Court Square , Suite 75                    | 0, Decatur GA 30030                                       |                                  |   |               |
| Applicant Pho                                 | one: (404) 601-7616   |   | Fax: (404) 74                    | 45-0045   |               |
| ******  | *********   | *******   | ******                           | ************  |               |
| Owner(s): I                                   | Refer to attachment<br>f more than one owner, atta                    | ch as Exhibit "A")  | E-Mail:                          |   |               |
| Owner's Mail                                  | ing Address:  |   |                                  |   |               |
| Owner(s) Pho                                  | one:  |   | Fax:                             |   |               |
| Address/Loca                                  | ation of Subject Property: _  | 671, 657, 635, 655, 649,                                  | 641, 631, 623 No                 | orthern Ave Clarkston GA 30021  |               |
| District(s): 18                               | Land Lot(s):  | 045 Bloc  | k:08                             | Parcel(s: <u>095, 001, 008, 003, 005,</u>                               | 006, 007, 004 |
| Acreage: <u>22.0</u>                          | 7   | Commission Dis  | strict(s): Distri                | ct 4, Super District 6  |               |
| Present Zonir                                 | ng Category: R-75 & MR-2  | Propos  | sed Zoning Ca                    | ategory: RSM  |               |
| Present Land                                  | Use Category: Suburban  | *******   | *****                            | ***********   |               |
|   |   | AD THE FOLLOWIN   |                                  |   |               |
| attachments                                   |   | the attachments. A  | An application                   | ent accepts it. It must include the , which lacks any of the required . |               |
|   | e with the Conflict of Interes  | sure of Campaign Cost in Zoning Act, O.C                  |                                  | er 36-67A, the following questions                                      | i             |
| must be answ<br>Have you the<br>two years imr |   | nore in campaign co                                       | entributions to<br>? Yes         | a local government official within                                      | í.            |
| If the answe showing;                         | r is yes, you must file a   | disclosure report w                                       | ith the gover                    | ning authority of DeKalb County   | ê<br>2        |
| 1.  | The name and official po  | osition of the local                                      | government                       | official to whom the campaign   | Ĭ             |
| 2.  | The dollar amount and des   |   |                                  | oution made during the two years te of each such contribution.          | 8             |
| The disclosur<br>C.E.O. and the               | re must be filed within 10 da<br>ne Board of Commissioners.           | , DeKalb County, 13                                       | 00 Commerce                      | and must be submitted to the Drive, Decatur, Ga. 30030.                 |               |
| APARATION                                     | DATE / SEAL   | Check   | One: Owner                       | Agent   |               |
| APRIL 02 200                                  | 330 West Ponce de Leor<br>(Voice) 404.371.2155 – [Plann<br>Web Addres | iing Fax] (404) 371-455<br>ss <u>http://www.dekalbcou</u> | 6 [Developmer<br>untyga.gov/plan | nt Fax] (404) 371-3007<br>Ining   |               |
| APRIL 02: 6                                   | Web Addres  |   | untyga.gov/plan                  | ning  |               |



# DeKalb County Department of Planning & Sustainability

Michael L. Thurmond Chief Executive Officer

Andrew A. Baker, AICP
Director



<u>Section 27-832</u>. Standards and factors governing review of proposed amendments to official zoning maps. The following standards and factors are found to be relevant to the exercise of the County's zoning powers and shall govern the review of all proposed amendments to the official zoning maps:

- A. Whether the zoning proposal is in conformity with the policy and intent of the Comprehensive Plan.
- B. Whether the zoning proposal will permit a use that is suitable in view of the use and development of adjacent and nearby properties.
- C. Whether the property to be affected by the zoning proposal has a reasonable economic use as currently zoned.
- D. Whether the zoning proposal will adversely affect the existing use or usability of adjacent or nearby properties.
- E. Whether there are other existing or changing conditions affecting the use and development of the property which give supporting grounds for either approval or disapproval of the zoning proposal.
- F. Whether the zoning proposal will adversely affect historic buildings, sites, districts, or archaeological resources.
- G. Whether the zoning proposal will result in a use which will or could cause excessive or burdensome use of existing streets, transportation facilities, utilities or schools.

<u>Section 27-833. Conditions</u>. Conditions may be requested by an applicant, recommended by the Planning Department and Planning Commission, and imposed by the Board of County Commissioners, as a part of any proposed change to the official zoning map, in accordance with the following requirement:

- A. Conditions may be imposed so as to ameliorate the effect(s) of the proposed developmental change for the protection or benefit of neighboring persons or properties consistent with the purpose and intent of the district(s) involved, and the goals and objectives of the Comprehensive Plan and state law. No condition shall be imposed which reduces the requirements of the district(s) involved. All conditions shall be of sufficient specificity to allow lawful and consistent application and enforcement. All conditions shall be supported by a record that evidences the relationship between the condition in the form of a development exaction for other than a project improvement shall be imposed within the meaning of the Georgia Development Impact Fee Act, as amended.
- B. The Board of Commissioners shall not impose any condition on a proposed amendment to the official zoning map that was not previously reviewed by the Planning Commission unless said condition has been reviewed by the Law Department, Planning Department, and the Public Works Department for legality, enforceability, and recommendation. The Board of County Commissioners may defer final action on any such proposed amendment for up to 60 days to allow for this review and may take action without referral back to the Planning Commission.
- C. Once imposed, conditions shall become an integral part of the approved amendment and shall be enforced as such. Changes to approved conditions shall be authorized only pursuant to Section 27-845 of this chapter.

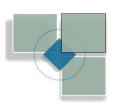
Page 2 of 3 Revised 1/1/17



# DeKalb County Department of Planning & Sustainability

Michael L. Thurmond Chief Executive Officer

Andrew A. Baker, AICP Director



### **Filing Fees**

Filing fees shall not be refunded at any time following the zoning schedule deadline date.

District Filing Fee

R-200, R-150, R-30, 000, R-20, 000, R-100, R-85 \$500.00 R-75, R-60, TND, R-A5, R-50, R-A8, R-DT, MHP,

RM-100, RM-150, CH (4-12 du/acre)

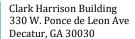
R-200, R-150, R-30, 000, R-20, 000, R-100, R-85, R-75, \$750.00

R-60, R-A8, R-DT, MHP, TND, RM-150, RM-100,

RM-85, RM-75, CH, RM-HD, O-I (high-rise apts.) (18 up du/acre)

O-I, O-D, OCR, OIT, NS, CH, C-1, C-2, M, M-2 \$750.00

Applicants requiring more than one zoning district shall be charged the highest of the applicable fee.





Chief Executive Officer
Michael Thurmond

**DEPARTMENT OF PLANNING & SUSTAINABILITY** 

Director

Andrew A. Baker, AICP

# PRE-APPLICATION FORM REZONE, SPECIAL LAND USE PERMIT, MODIFICATION, AND LAND USE

(Required prior to filing application: signed copy of this form must be submitted at filing)

| Applicant Name: Inline Communities c/o Battle Law PC Phone: 404.601.7616 Email: mlb@battlelawpc.com  |  |  |  |
|--|--|--|--|
| Property Address: 671, 657, 635, 655, 649, 641, 631, 623 Northern Ave, Clarkston GA 30021  |  |  |  |
| Tax Parcel ID: 18 045 08 003 Comm. District(s): District 4, Super District 6 Acreage: 22.07  |  |  |  |
| Existing Use: Suburban Proposed Use: Suburban  |  |  |  |
| Supplemental Regs: _No Overlay District: No DRI: _No   |  |  |  |
| <b>Rezoning</b> : Yes <u>x</u> No  |  |  |  |
| Existing Zoning: <u>R75 and MR2</u> Proposed Zoning: <u>RSM</u> Square Footage/Number of Units: <u>151 units</u>   |  |  |  |
| Rezoning Request: <u>Development of 8- rear loaded townhome units</u> , 27 front loaded townhome units, 44 single family <u>detached homes</u>   |  |  |  |
| Land Use Plan Amendment: Yes No _X   |  |  |  |
| Existing Land Use:SUB Proposed Land Use: Consistent Inconsistent   |  |  |  |
| Special Land Use Permit: Yes No _X Article Number(s) 27  |  |  |  |
| Special Land Use Request(s)  |  |  |  |
| Major Modification:  Existing Case Number(s):NA Existing zoning conditions CZ 10 16332 will no longer be effective if Board of Commissioners approves the rezoning  Condition(s) to be modified: |  |  |  |
|  |  |  |  |



### **DEPARTMENT OF PLANNING & SUSTAINABILITY**

# WHAT TO KNOW BEFORE YOU FILE YOUR APPLICATION

| Pre-submittal Community Meeting:X Review Calendar Dates:X PC: _3/2/21* BOC:   |
|---|
| _3/23/21* Letter of Intent:X Impact Analysis:X Owner Authorization(s):X   |
| Campaign Disclosure:X Zoning Conditions: _X Community Council Meeting: _2/16/21*  |
| Public Notice, Signs: _Applicant must pick up and post Tree Survey, Conservation:X Land   |
| Disturbance Permit (LDP):X Sketch Plat:X Bldg. Permits:X Fire   |
| nspection:X Business License: State License: Lighting Plan:X Tent   |
| Permit: Submittal Format: NO STAPLES, NO BINDERS PLEASE   |
| Assuming the Board of Commissioners adopts the draft 2021 zoning calendar as original proposed.   |
| Review of Site Plan   |
| Density: 6.84 units/acre Density Bonuses: 4 UPA (BASE) - 8 UPA (DENSITY BONUSES) 20% Enhanced Den (2.0upa) / Public Art ( 0.8 upa)  |
| Mix of Uses: Open Space: 7.47 acres_  |
| Enhanced Open Space: 20% ADDITIONAL OF SITE AREA (3.73 ACRES)   |
| Townhome setbacks front: (arterial/local): 20 feet (development)/10 feet (local) side setback (interior): 0 feet (10' puilding separation) side corner: 20 feet(development)/ 10 feet (local) rear setback (w/o alley)/(w alley): 15 feet/ 10 feet (local)  |
| Detached housing Setbacks: front <u>20 feet min/30 feet max</u> sides <u>3' building separation</u> side corner <u>20 feet</u> rear<br><u>20 feet</u> Lot Size: <u>20' x 45' townhomes, 24' x 50' townhomes, 60' detached single family lots, 30' x 90'</u><br><u>detached single family lots</u>   |
| Frontage: Street Widths: Landscape Strips: Buffers: Parking Lot Landscaping: Parking - Auto: 528 spaces (1 garage + 2 driveway space) residential guest parking - 63 psaces, total parking provided - 591 spaces Parking - Bicycle: Screening: Streetscapes: Sidewalks: Fencing/Walls: Bldg. Height: Bldg. Orientation: Bldg. Separation: Bldg. Materials: Roofs: Fenestration: Façade Design: Barages: Pedestrian Plan: Perimeter Landscape Strip: |
| Possible Variances: _Applicant will need to verify compliance with MR-2 zoning standards on site plan   |
| egarding all these issues. Guest parking may count against the maximum parking requirements; a parking  |
| variance may be required if this is the case.   |
|   |
| _   |
| Comments: Density of adjacent apartments will be shown on plan to demonstrate appropriateness of proposed density. Sidewalks  |

will be provided on plan submitted to Planning Department. Streetscape standards may apply to private driveways, this will be determined when rezoning application is submitted and under review. Must comply with MR-2 zoning requirements or variances



## **DEPARTMENT OF PLANNING & SUSTAINABILITY**

| will be required. Tree survey will be done; removal of specimen trees will require County Arborist approval. Plan appears to comply |  |               |  |  |
|---|--|---------------|--|--|
| with perimeter lot compatibility standards of Article 5; will be verified when rezoning application is submitted and under review.  |  |               |  |  |
| All tax parcel ids that are proposed for rezoning will be listed on the submitted rezoning application to the Planning Department.  |  |               |  |  |
| Planner:John ReidDate   |  | _Date12/10/20 |  |  |
| Filing Fees   |  |               |  |  |
| <b>REZONING:</b>  | RE, RLG, R-100, R-85, R-75, R-60, MHP, RSM, MR-1         | \$500.00      |  |  |
|   | RNC, MR-2, HR-1, HR-2, HR-3, MU-1, MU-2, MU-3, MU-4, MU- | 5 \$750.00    |  |  |
|   | OI, OD, OIT, NS, C1, C2, M, M2                           | \$750.00      |  |  |
| LAND USE MAP AMENDMENT  |  | \$500.00      |  |  |
| SPECIAL LAND USE PERMIT   |  | \$400.00      |  |  |

### Community Meeting Sign-up Sheet and Chat - 655 Northern 12/10

From summer: For those just entering, please add you emails and names. This will constitute as our sign up sheet for the meeting. Feel free to send this to me privately

- Maggie & Scott Nesbit <u>magsco@gmail.com</u>, scottnesbit@gmail.com
- rita valenti my phone: 678-328-8725
- Caitlin Thigpen Caitlin.awalt@gmail.com
- John Short here :) Using Lindsay's zoom
- Nai/GAMVP naingkokooo@gmail.com
- coopertisdale@hotmail.com. 404-405-8010

### Chat:

| 18:04:09  | From summer To Jennifer Kapner(privately): Thats fine! You can send it to me    |  |  |  |
|---|---|--|--|--|
| as well   |   |  |  |  |
| 18:07:31  | From summer: For this just entering, please add you emails and names. This will |  |  |  |
| constitute as our sign up sheet for the meeting. Feel free to send this to me privately |   |  |  |  |
| 18:07:52  | From Maggie & Scott Nesbit To summer(privately): magsco@gmail.com               |  |  |  |
| 18:08:00  | From Maggie & Scott Nesbit To summer(privately): maggie scott nesbit            |  |  |  |
| 18:08:13  | From Maggie & Scott Nesbit To summer(privately): scottnesbit@gmail.com          |  |  |  |
| 18:08:18  | From Maggie & Scott Nesbit To summer(privately): scott j nesbit                 |  |  |  |
| 18:08:27  | From Maggie & Scott Nesbit To summer(privately): thank you summer               |  |  |  |
| 18:09:03  | From summer To Maggie & Scott Nesbit(privately): Thank you Scott and            |  |  |  |
| Maggie!   |   |  |  |  |
| 18:20:01  | From rita valenti : hand raised please!   |  |  |  |
| 18:22:00  | From rita valenti : please unmute me!!  |  |  |  |
| 18:22:50  | From summer To rita valenti(privately): Hi Rita, if you'd like to send me your  |  |  |  |
| question in the chat I can relay it   |   |  |  |  |
| 18:22:54  | From rita valenti: they say I'm muted! Please unmute me                         |  |  |  |
| 18:23:25  | From rita valenti : my hand is raised please unmute me                          |  |  |  |
| 18:23:27  | From Maggie & Scott Nesbit : ok rita i see you too                              |  |  |  |
| 18:23:45  | From summer To rita valenti(privately): We've unmuted, please make sure         |  |  |  |
| your computer is connected to your microphone   |   |  |  |  |
| 18:24:34  | From summer : Rita you are unmuted  |  |  |  |
| 18:25:06  | From rita valenti : I am audio on my phone: 678-328-8725. Please unmute - not   |  |  |  |
| on computer   |   |  |  |  |
| 18:26:21  | From rita valenti: I am not getting unmute requests! Please unmute 678-328-     |  |  |  |
| 8725  |   |  |  |  |
| 18:26:44  | From rita valenti : I have pressed *6   |  |  |  |
| 18:26:51  | From summer To rita valenti(privately): Working to get you the code for your    |  |  |  |
| mobile  |   |  |  |  |

| 18:27:38<br>chat   | From summer: in the meantime, Rita please feel free to ask your question in the  |  |  |  |
|--|--|--|--|--|
| 18:29:33<br>will be a part of<br>lived with con  | From Belle Anderson: Sorry I came in late, I have a concern about the noise that of the construction and how long it will take to complete this construction. I have struction on this site for years and it has been horrible for me on Sandy Woods |  |  |  |
| Lane.  |  |  |  |  |
| 18:30:30   | From scott and maggie : thank you, Rita!   |  |  |  |
| 18:30:43   | From Maggie & Scott Nesbit : yes thank you   |  |  |  |
| 18:30:51   | From jessjones : RITA FOR THE WIN  |  |  |  |
| 18:30:54   | From Caitlin Thigpen : Thank you Rita!   |  |  |  |
| 18:31:20   | From Maggie & Scott Nesbit: also thank you nai and vasav   |  |  |  |
| 18:32:57   | From rita valenti: The only zoning change was to 657 NORTHERN - for RM85:  |  |  |  |
| •  | ose properties.  |  |  |  |
| 18:34:04   | From rita valenti: That's all on Indian Creek Way. And those existing  |  |  |  |
| apartments ha  | ave been here for 30 plus years and are much smaller.  |  |  |  |
| 18:35:41   | From jessjones : Please don't have an entrance on Creekview.   |  |  |  |
| 18:36:10   | From Maggie & Scott Nesbit : yes rita  |  |  |  |
| 18:36:55   | From jessjones: Who's sending Rita drinks after this?  |  |  |  |
| 18:38:18   | From hibo hussein : great job Rita!!   |  |  |  |
| 18:39:51   | From KWood : Belle Anderson,   |  |  |  |
| 18:40:15   | From KWood: Belle Anderson, are you wanting to know the total duration if it   |  |  |  |
| were to be ap  | proved of construction and building homes?   |  |  |  |
| 18:41:32   | From Victoria Webb: Usually projects of this magnitude can take years. Only  |  |  |  |
| speaking from  | experience with a 17 acre development near me. Began in 2016, still building   |  |  |  |
| out.   |  |  |  |  |
| 18:46:33   | From rita valenti: The last developer clear cut the entire property, and then  |  |  |  |
| went belly up.   | Your developers don't even have a track record. The so-called "adjoining"  |  |  |  |
| properties are   | e not part of the Northern Avenue and Dial Heights. All this land was dairy many   |  |  |  |
| years: the zoning decades ago was totally chaotic. The development you are proposing is to |  |  |  |  |
| increase dens  | ity -not conforming to the existing neighbor.  |  |  |  |
| 18:48:10   | From rita valenti: This area is outside the Clarkston City limits, not surrounded  |  |  |  |
| by it. There is  | absolutely no motion to redevelop the apartments on Indian Creek Way - but if  |  |  |  |
| InLine wanted  | l to do that - that would be great.  |  |  |  |
| 18:48:43   | From rita valenti: There was a deer in my front yard this AM. 660 Northern   |  |  |  |
| 18:48:55   | From hibo hussein: I agree Rita, those apartments really need to be  |  |  |  |
| redeveloped  |  |  |  |  |
| 18:48:58   | From Maggie Nesbit : migrate = displaced   |  |  |  |
| 18:49:13   | From rita valenti : right, Maggie  |  |  |  |
| 18:49:24   | From jessjones: COMPLETELY agree Scott and Maggie  |  |  |  |
| 18:49:42   | From Caitlin Thigpen : Thank you both  |  |  |  |
| 18:52:51   | From hibo hussein : thank you scott!!  |  |  |  |
| 18:54:29   | From rita valenti : Density and town homes don't necessarily equate to   |  |  |  |
| affordable ho  | affordable housing.  |  |  |  |
|  |  |  |  |  |

|               | 19:22:28                         | From Bryan Musolf : Bryan Musolf   |  |
|---------------|----------------------------------|--|--|
|               | 19:22:39                         | From KWood: The wildlife will move during construction but as you have seen        |  |
|               | all over the city                | y it comes back very quickly after landscaping installed. The buffer area will not |  |
| be disturbed. |                                  |  |  |
|               | 19:22:45                         | From Bryan Musolf : Bryan Musolf InLine Communities.                               |  |
|               | bryan@inliined                   | communities.com  |  |
|               | 19:22:46                         | From Maggie Nesbit : yes fran and belle too  |  |
|               | 19:22:48                         | From summer: If you all have any questions please feel free to email me at         |  |
|               | ssw@battlelawpc.com              |  |  |
|               | 19:22:59                         | From Lindsay Short: I wish the developers would consider a conservation            |  |
|               | community. W                     | ould be MUCH more in line with our community.                                      |  |
|               | 19:22:59                         | From jessjones: Fran needs to be able to ask her question                          |  |
|               | 19:23:01                         | From Belle Anderson: Please also think about those of us who live on the           |  |
|               | 'boundaries' ar                  | nd when it is approved and we have valid concerns we would like to speak with      |  |
|               | more than an 'answering machine' |  |  |
|               | 19:23:01                         | From Maggie Nesbit : please share chat with all of us                              |  |
|               | 19:23:08                         | From scott and maggie : exactly Lindsay!   |  |
|               | 19:23:17                         | From Caitlin Thigpen : I downloaded it Maggie                                      |  |
|               | 19:23:19                         | From Victoria Webb : Thank you everyone.   |  |
|               | 19:23:21                         | From Caitlin Thigpen : I can email to you  |  |
|               | 19:23:32                         | From Lindsay Short : John Short here :) Using Lindsay's zoom                       |  |
|               | 19:23:32                         | From summer: for access to the chat and zoom meeting email me a request at         |  |
|               | ssw@battlelaw                    | vpc.com  |  |
|               | 19:23:43                         | From Maggie Nesbit : yes please sharw  |  |
|               | 19:23:48                         | From Caitlin Thigpen: John say hey to Lindsay for me!                              |  |
|               | 19:23:48                         | From Maggie Nesbit : thank you   |  |
|               | 19:23:57                         | From Batoya Clements : Batoya Clements - bdc@battlelawpc.com                       |  |
|               | 19:24:04                         | From Nai/GAMVP: will we get a copy of the map???                                   |  |
|               | 19:24:14                         | From scott and maggie: thanks so much to MICHELLE!!!                               |  |
|               | 19:24:18                         | From summer: Yes we can share a copy of the site plan                              |  |
|               | 19:24:36                         | From Nai/GAMVP: please do - naingkokooo@gmail.com or streetwide please             |  |
|               | 19:25:01                         | From iPad (2) To summer(privately): coopertisdale@hotmail.com. 404-405-            |  |
|               | 8010                             |  |  |
|               | 19:25:13                         | From summer: it would be helpful for those requesting information to send me       |  |
|               |                                  | v@battlelawpc.com  |  |
|               | 19:25:14                         | From Belle Anderson: I would like a copy of the site plan. I think we all would.   |  |
|               | Thanks again.                    |  |  |
|               | 19:25:52                         | From Cooper Sanchez: someone needs to change the battery one their fire            |  |
|               |                                  | sleep on this issue. I don't know how you could.                                   |  |
|               | 19:26:43                         | From iPad (2) To summer(privately): I think it was you that asked for our info     |  |
|               | directly.                        |  |  |
|               | 19:26:46                         | From Maggie Nesbit : thanks y'all! best night to you                               |  |
|               |                                  |  |  |

### LETTER OF INTENT AND IMPACT ANALYSIS

As Required by

City of South Fulton, Georgia Zoning Ordinance

For

Rezoning Application pursuant to the Dekalb County Zoning Ordinance

by

Inline Communities LLC (the "Applicant")

For

22.07 acres of land located at 671, 657, 635, 655, 649, 641, 631, 623 Northern Ave Clarkston GA 30021

Submitted for Applicant by:

Michèle L. Battle

Battle Law, P.C.

One West Court Square, Suite 750

Decatur, Georgia 30030

(404) 601-7616 Phone

(404) 745-0045 Facsimile mlb@battlelawpc.com

### Statement of Intent and Impact Analysis

The applicant, Inline Communities LLC is seeking to develop 22.07 acres at 671, 657, 635, 655, 649, 641, 631, 623 Northern Ave Clarkston GA 30021 (collectively, the "Subject Property") for the development of 151 residential units for a Residential Community Development. The Subject Property is currently zoned R-75 and MR-2 with a land use designation of Suburban. The applicant is seeking to rezone to the Subject Property to Small Lot Residential Mix (RSM) to allow for 6.84 units per acre.

This document is submitted as the Letter of Application regarding this Application, and a preservation of the Applicant's constitutional rights. A surveyed plat and site plan of the Subject Property has been filed contemporaneously with the Application, along with other required materials.

### **IMPACT ANALYSIS**

1. Does the zoning proposal permit a use that is suitable in view of the use and development of adjacent and nearby property?

The Subject Property is currently zoned R-75 and MR-2. It is adjacent to properties zoned MR-2 allowing for 18 units per acre and R-75 allowing for 8 units per acre in a land use designated Suburban. The proposed rezoning to RSM at 12 units per acre is consistent with the Suburban land use designation and will harmonious with the surrounding properties and introduce a mix of living options, it will provide for new homes owners in the area on a currently underdeveloped lot. The anticipated price points on the homes be equal to or greater than the surrounding home values, which will help support the existing home values in the area, supporting the continued growth and development of area.

2. Does the zoning proposal adversely affect the existing use or usability of adjacent or nearby property?

The proposed rezoning to RSM will not adversely affect the existing use or usability of adjacent or nearby property. The proposed rezoning will support the continued growth and development of the surrounding area.

3. Does the property to be rezoned have a reasonable economic use as currently zoned?

The Subject Property has no reasonable economic use as currently zoned R 75 and MR2 with conditions as a private school. The property has been on the market for sale for an excess of 10 years. The current zoning conditions severely restrict the use of the property to a non-residential private school which is a specialized use with no marketability outside of private school operators.

4. Will the zoning proposal result in a use that could cause an excessive or burdensome use of existing streets, transportation facilities, utilities or schools?

Located on Northern Ave, there is the possibility for an increase of traffic on the existing street. The Applicant aims to provide a traffic assessment and perform modifications that will lessen the traffic congestion originating from the development along Northern Avenue. With respect to the public schools in the area, at 165 units per acre, there should not a substantial increase of students who would attend one of the three public schools in the area. The tax dollars generated by the Subject Property as well as the other subdivision coming into the area, provides an opportunity for the expansion of existing facilities.

5. Is the zoning proposal in conformity with the policies and intent of the land use plan?

The South Fulton Comprehensive Land Use Map shows the Subject Property as having a land use designation of Suburban. The RSM zoning designation is a permitted and in conformity with the Suburban designation.

## 6. Are there existing or changing conditions that affect the use and development of the property which support either approval or denial of the zoning proposal?

With the approved rezoning to RSM of The Subject Property, we aim to create two entrance points for The Subject Property, have building maximum height of 45', promote pedestrian connectivity throughout the property, have a mix of single and multi-family units that is harmonious in scale, provides flexibility of design, and provides usable amenity spaces, with no additional conditions or variances applied to the Subject Property.

### 7. Does the zoning proposal permit a use that can be considered environmentally adverse to the natural resources, environment and citizens of City of South Fulton?

The rezoning will not permit any use that can be considered environmentally adverse to the natural resources, environment and citizens of the Dekalb County which is not typical of development projects. It will include a 75' stream buffer and two water detention ponds adjacent to the flood zone and aims to keep natural wooded areas at the west-end of the property.

### NOTICE OF CONSTITUTIONAL ALLEGATIONS AND PRESERVATION OF CONSTUTIONAL RIGHTS

The portions of the DeKalb County Zoning Ordinance, facially and as applied to the Subject Property, which restrict or classify or may restrict or classify the Subject Property so as to prohibit its development as proposed by the Applicant are or would be unconstitutional in that they would destroy the Applicant's property rights without first paying fair, adequate and just compensation for such rights, in violation of the Fifth Amendment and Fourteenth Amendment of the Constitution of the United States and Article I, Section I, Paragraph I of the Constitution of the State of Georgia of 1983, Article I, Section III, Paragraph I of the Constitution of the State of Georgia of 1983, and would be in violation of the Commerce Clause, Article I, Section 8, Clause 3 of the Constitution of the United States.

The application of the DeKalb County Zoning Ordinance to the Subject Property which restricts its use to any classification other than that proposed by the Applicant is unconstitutional, illegal, null and void, constituting a taking of Applicant's Property in violation of the Just Compensation Clause of the Fifth Amendment to the Constitution of the United States, Article I, Section I, Paragraph I, and Article I, Section III, Paragraph I of the Constitution of the State of Georgia of 1983, and the Equal Protection and Due Process Clauses of the Fourteenth Amendment to the Constitution of the United States denying the Applicant an economically viable use of its land while not substantially advancing legitimate state interests.

A denial of this Application would constitute an arbitrary irrational abuse of discretion and unreasonable use of the zoning power because they bear no substantial relationship to the public health, safety, morality or general welfare of the public and substantially harm the Applicant in violation of the due process and equal protection rights guaranteed by the Fifth Amendment and Fourteenth Amendment of the Constitution of the United States, and Article I, Section I, Paragraph I and Article I, Section III, Paragraph 1 of the Constitution of the State of Georgia.

A refusal by the DeKalb County Board of Commissioners to rezone the Subject Property to the classification as requested by the Applicant would be unconstitutional and discriminate in an arbitrary, capricious and unreasonable manner between the Applicant and owners of similarly situated property in violation of Article I, Section I, Paragraph II of the Constitution of the State of Georgia of 1983 and the Equal Protection Clause of the Fourteenth Amendment to the Constitution of the United States. Any rezoning of the Property subject to conditions which are different from the conditions requested by the Applicant, to the extent such different conditions would have the effect of further restricting Applicant's utilization of the property, would also constitute an arbitrary, capricious and discriminatory act in zoning the Subject Property to an unconstitutional classification and would likewise violate each of the provisions of the State and Federal Constitutions set forth hereinabove

A refusal to allow the rezoning in questions would be unjustified from a fact-based standpoint and instead would result only from constituent opposition, which would be an unlawful delegation of authority in violation of Article IX, Section II, Paragraph IV of the Georgia Constitution.

A refusal to allow the rezoning in question would be invalid in as much as it would be denied pursuant to an ordinance which is not in compliance with the Zoning Procedures Law, O.C.G.A Section 36-66/1 et seq., due to the manner in which the Ordinance as a whole and its map(s) have been adopted.

The existing zoning classification on the Subject Property is unconstitutional as it applies to the Subject Property. This notice is being given to comply with the provisions of O.C.G.A. Section 36-11-1 to afford the County an opportunity to revise the Property to a constitutional classification. If action is not taken by the County to rectify this unconstitutional zoning classification within a reasonable time, the Applicant is hereby placing the County on notice that it may elect to file a claim in the Superior Court of DeKalb County demanding just and adequate compensation under Georgia law for the taking of the Subject Property, diminution of value of the Subject Property, attorney's fees and other damages arising out of the unlawful deprivation of the Applicant's property rights.

#### **ZONING DESCRIPTION**

ALL THAT TRACT OR PARCEL OF LAND lying and being in Land Lots 44, 45 and 66 of the 18th District, DeKalb County, Georgia and being more particularly described as follows:

To find the TRUE POINT OF BEGINNING, commence from a point, at the intersection of the easterly right-of-way line of Northern Avenue (50' R/W) and the Land Lot Line common to Land Lots 45 and 66; thence along said right-of-way line 117.33 feet along an arc of a curve to the right, said curve having a radius of 644.97 feet and a chord bearing and distance of South 5 degrees 19 minutes 0 seconds East 117.17 feet to a point and the TRUE POINT OF BEGINNING; thence leaving said right-of-way line South 89 degrees 32 minutes 2 seconds East a distance of 195.06 feet to a point; thence North 0 degrees 19 minutes 33 seconds East a distance of 117.09 feet to a point; thence South 89 degrees 40 minutes 43 seconds East a distance of 754.46 feet to a point; thence North 38 degrees 57 minutes 2 seconds East a distance of 161.35 feet to a point; thence North 89 degrees 14 minutes 14 seconds East a distance of 135.98 feet to a point; thence South 19 degrees 36 minutes 47 seconds East a distance of 637.19 feet to a point; thence South 23 degrees 15 minutes 51 seconds East a distance of 271.01 feet to a point; thence North 88 degrees 54 minutes 34 seconds West a distance of 777.54 feet to a point; thence North 89 degrees 10 minutes 23 seconds West a distance of 737.54 feet to a point on said right-of-way line; thence along said right-of-way line the following courses and distances: North 0 degrees 8 minutes 46 seconds West a distance of 217.32 feet to a point; thence North 01 degree 4 minutes 44 seconds East a distance of 367.94 feet to a point to a point and the TRUE POINT OF BEGINNING.

Said tract containing 22.351 acres, more or less.

### Campaign Contribution Disclosure Statements

### CAMPAIGN CONTRIBUTIONS DISCLOSURE STATEMENT

Pursuant to the provisions of 36 O.C.G.A. 67(A), please find below a list of those contributions made by Michèle L Battle or Battle Law, P.C. in the past two years, aggregating \$250.00 or more, to local government officials who will consider this application.

| NAME OF<br>GOV'T OFFICIAL | OFFICIAL POSITION | AMOUNT OF  CONTRIBUTION |
|---------------------------|-------------------|-------------------------|
| Kathie Gannon             | Commissioner      | \$350                   |
| Mereda Davis Johnson      | Commissioner      | \$500                   |
| Larry Johnson             | Commissioner      | \$700                   |
| Lorraine Cochran-Johnson  | Commissioner      | \$250                   |
|                           |                   |                         |

Printed Name: //



404.371.2155 (o) 404.371.4556 (f) DeKalbCountyGa.gov Clark Harrison Building 330 W. Ponce de Leon Ave Decatur, GA 30030

Chief Executive Officer

### DEPARTMENT OF PLANNING & SUSTAINABILITY

Director

Michael Thurmond

Andrew A. Baker, AICP

### **REZONE APPLICATION AUTHORIZATION**

Completion of this form is required if the individual making the request is <u>not</u> the owner of the property.

| DATE:12/8/2020   |   |
|--|---|
| CHECK TYPE OF APPLICATION:   |   |
| ( ) LAND USE PLAN  |   |
| (x) REZONE   |   |
| ( ) MINOR MODIFICATION   |   |
| TO WHOM IT MAY CONCERN:  |   |
| ( ' ) ( ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' '  | nd Holdings LLC  of owner(s))                 |
| to file an application of the property described by Inline Community to file an application of Applicant or A | elow or attached hereby delegate authority to |
| Notary Public OF OF  | Owner Owner                                   |
| Notary Public  | Owner   |
| Notary Public  | Owner   |

404.371.2155 (o) 404.371.4556 (f) DeKalbCountyGa.gov Clark Harrison Building 330 W. Ponce de Leon Ave Decatur, GA 30030

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|----------------------------|---------------------|---|
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| ( ) LAND USE PLAN          |                     |   |
| (X) REZONE                 |                     |   |
| ( ) MINOR MODIFICATION     |                     |   |
| TO WHOM IT MAY CONCERN:    |                     |   |
| ( I ) (WE),                |                     | GE, ANJALI of owner(s))   |
|                            | Inline Commun       | below or attached hereby delegate authority to sities LLC c/o Battle Law PC Agent Representing Owner) |
| Notary Public              | NOO 8/1/            | GRANDHIGE, NJALI<br>Owner   |
| Notary Public              | ATON ON MINISTRALIA | Owner   |
| Notary Public              | M B W               | Owner   |

404.371.2155 (o) 404.371.4556 (f) DeKalbCountyGa.gov Clark Harrison Building 330 W. Ponce de Leon Ave Decatur, GA 30030

Chief Executive Officer

### DEPARTMENT OF PLANNING & SUSTAINABILITY

Director

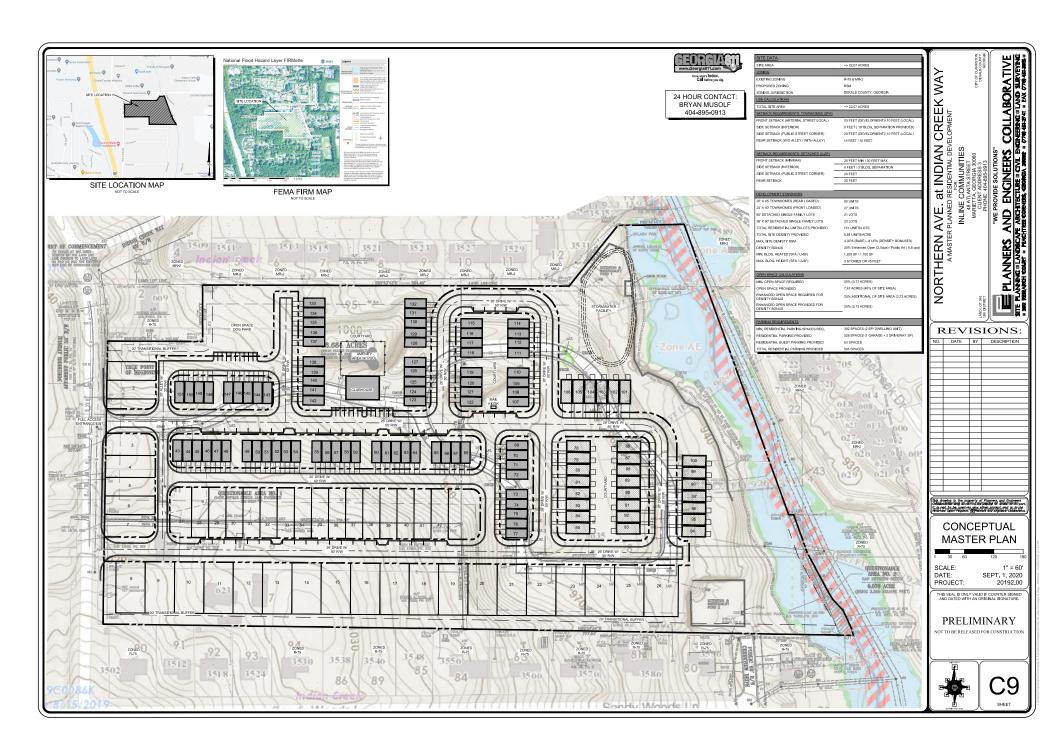
Michael Thurmond

Andrew A. Baker, AICP

### REZONE APPLICATION AUTHORIZATION

Completion of this form is required if the individual making the request is <u>not</u> the owner of the property.

| DATE:12/8/2020               |                          |   |
|------------------------------|--------------------------|---|
| CHECK TYPE OF APPLIC         | CATION:                  |   |
| ( ) LAND USE PLAN            |                          |   |
| (X) REZONE                   |                          |   |
| ( ) MINOR MODIFICATION       | N                        |   |
|                              |                          |   |
| TO WHOM IT MAY CONC          | CERN:                    |   |
| (I) (WE),                    | GRANDHI                  | GE, HEMANTH                                   |
|                              | (Name o                  | of owner(s))                                  |
| being (owner )/(owners) of   | the property described b | elow or attached hereby delegate authority to |
|                              | Inline Communi           | ities LLC c/o Battle Law PC                   |
|                              | (Name of Applicant or A  | Agent Representing Owner)                     |
| to file an application on (m | y) / (our) behalf.       |   |
| Lunda M. Bera                | na                       | GRANDHIGE, HEMANTH                            |
| Notary Public                | M BRANNSSION ELS. 72     | Owner   |
| Notary Public                | D NOTARL RES             | Owner   |
|                              | PLANUARY 25 20 40        |   |



Thank you to the members of the Planning Commission for sharing your time and care to support and advocate for your neighbors in DeKalb County.

I am Fran Mohr, Immediate Past President of the Dial Heights Neighborhood Association and resident of Dial Heights for 34 years. My statement represents the wishes and concerns of the community directly impacted by the proposed development.

The Dial Heights and Northern Avenue neighborhoods are comprised of single-family homes predominantly mid-century modern style and brick ranch with DeKalb County's signature feature of mature trees and greenspace. This area of DeKalb County is labeled a "suburban" thus supporting the desire to have this area be one of lower density and greenspace that supports good air quality, decreased water runoff and aesthetic, desirable neighborhoods. The woodlands also help mitigate some of the air and noise pollution stemming from I285.

The Dial Heights and Northern Avenue neighborhoods would prefer the parcels be acquired and used as a natural greenspace that would benefit the community and the environment. Being a part of the Park Bond green space buying program would be a perfect mechanism for such an acquisition.

If it is to be developed, we ask that you oppose the spot zoning to RSM. Instead we propose that all parcels remain or be changed to R-75 which is compatible with the adjoining single-family residential neighborhoods and is allowed in a designated Suburban area of the county's comprehensive plan. The current MR-2 zoning applied to the east side of the land, including the wetlands and floodplain is not an allowable zone within a Suburban designation. Battle Law is quick to point out there are MR-2 Zoned parcels to the north of the proposed development on Indian Creek Way. Those were built in the 1970's and grandfathered in as an exception to the Suburban land use designation. The rezoning request seeks to eliminate all characteristics of the existing R-75.

### 1) Density

- The density of proposed development along the outer perimeter of the site that directly abuts
  the Dial Heights Neighborhood, Northern Avenue, and the wetlands and floodplain of Indian
  Creek should most closely resemble R-75 zoning characteristics, including but not limited to
  maximum densities, minimum lot sizes, and setbacks. This will create a more realistic buffer of
  density, impacting noise, appeal, neighborhood characteristics, traffic, and runoff.
- As identified by the Planning Department, the proposed building heights of the single-family detached lots along the southern portion of the site needs to be clarified. A maximum of two stories would be consistent and more compatible with the one and two-story single-family homes to the west on Northern Avenue and to the south in Dial Heights.
- R-75 zoning and lower density would help address some of our main concerns caused by increased density: traffic and environmental impact, including water runoff and flooding,

#### 2) Traffic was - and is - a major concern.

Northern Avenue's ability to absorb an increase of traffic has been questioned at least as far back as the last two proposed developments: the 72-home development in 2006, and the school in 2010. It is important to note that the community was less concerned about an increase of traffic with the Fugee School as many of the students would be walking to school. We were supportive because the school benefitted the community.

We are concerned about an increase in traffic on Northern Avenue that already is overstressed, undermaintained, and does not meet current county standards and requirements of a Collector Street. Even with the decreased traffic during the pandemic, traffic is heavy, making it difficult to pull out from neighborhood streets and driveways quickly and safely. Northern Avenue also is used as a quick cutthrough, adding to the volume and speed on the road.

The application states the developer would perform modifications that will 'lessen traffic congestion.' A move to maintain and rezone all parcels as R-75 to keep the density at a minimum would help provide one solution.

Battle Law also mentioned a bike lane would be added in front of the property as a bonus and solution to traffic. That is a requirement by DeKalb County and ultimately a possible solution if a bike lane is installed along the entire length of Northern Avenue and connects with the PATH installation along Church Street in Clarkston.

### 3) Environmental Concerns: Greenspace, water runoff, flooding, etc.

We agree with the Planning Department's desire to know what trees are being preserved to save as much of the existing tree canopy and native woodlands as possible to provide dense buffers between the proposed development and existing neighborhoods. The woodlands and greenspace support a healthier environment and help mitigate the air and noise pollution stemming from immediately adjacent I-285.

It appears the wetlands and floodplain areas of Indian Creek on the east perimeter are included in "enhance open space benefiting the community" to request increased density. The wetlands and floodplain of Indian Creek should be protected. Indian Creek is a major water artery in DeKalb County that runs through Dial Heights. County sewer lines for the neighborhood are installed alongside the creek and impacted by flooding.

In 2006, the developer, Emco Properties, worked closely with the surrounding neighborhoods to reach a compromise to build a maximum of 72 units, maintain dense buffers, and protect hardwood acreage and specimen trees in order to remain compatible with surrounding neighborhoods and to protect the wetlands and floodplain. As that developer did not honor that agreement, the clearing of some woodlands dramatically increased run off and resultant flooding of Indian Creek, which directly impacts the yards and basements of many Dial Heights homes.

Silt buildup in and erosion along the creek banks undercutting trees and exposing tree roots continue to be issues and creates potentially dangerous situations. The present proposal seeks twice as many units, with much less greenspace. The proposed density and loss of woodlands would significantly increase the runoff and the frequency of the flooding and erosion of Indian Creek impacting downstream and adjacent properties.

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Our community always will extend a welcome to new neighbors to this secret oasis of DeKalb County, but it doesn't have to be to the detriment of the existing neighborhoods.

If these parcels cannot be a public greenspace with a benefit to the wider community, please vote against spot zoning the current parcels in question to RSM. Instead, please consider recommending the MR-2 zoned parcels and wetland being rezoned to R75 to be compatible with the surrounding neighborhoods and suburban designation, to avoid overstressing an already inadequate Collector street, an overtaxed infrastructure, and to make less of a negative environmental impact.

#### Thank you

Fran Mohr, 436 Greenridge Circle, kfmohr@comcast.net, 404-210-7341

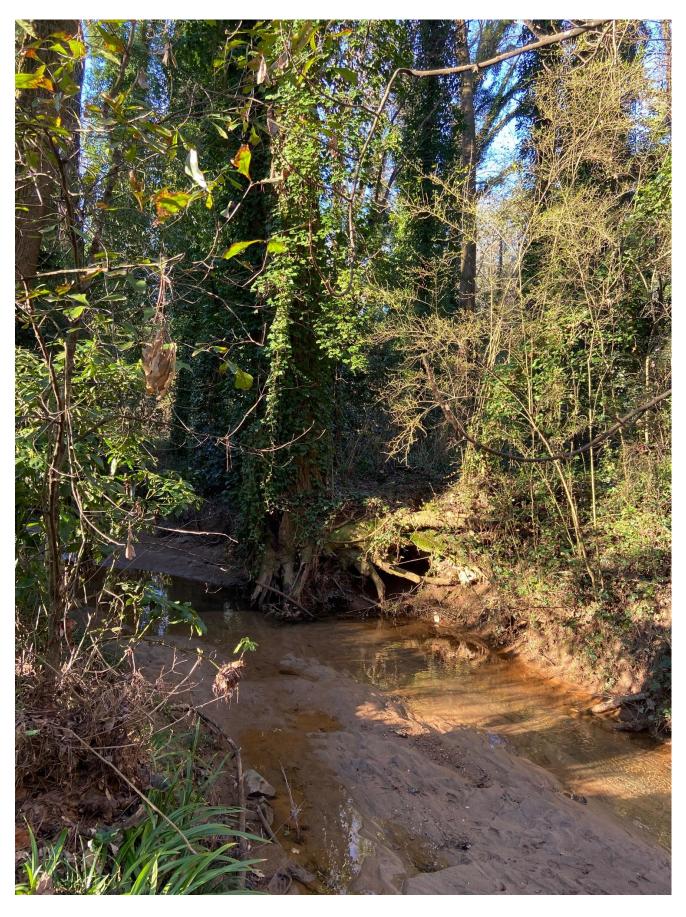
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Photos of Indian Creek from behind 436 Greenridge Circle. Small example of erosion, silt buildup and exposed tree roots. These large trees are in danger of falling potentially causing extensive property damage. DeKalb County sewage lines are buried in bank along creek.

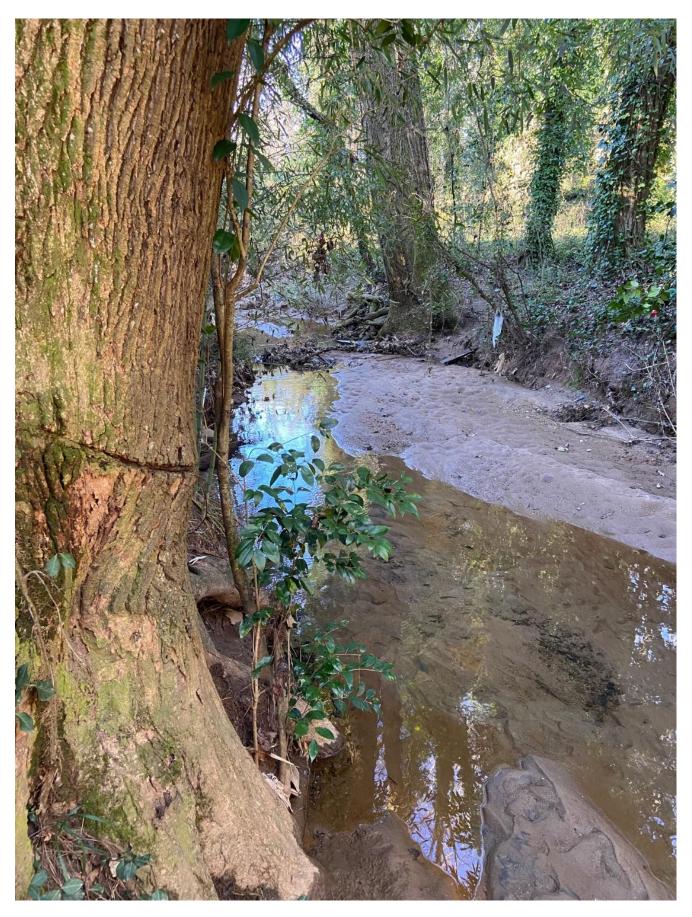








Statement on Behalf of the Dial Heights and Northern Neighborhoods Planning Commission Presentation – March 4, 2021





From: rita valenti <rita1880@att.net>

Subject: Proposed Rezoning on Northern Avenue, Clarkston 30021

Date: February 28, 2021 at 7:00:39 PM EST

To: Rita Valenti <rita1880@att.net>

#### Dear Commissioner Bradshaw,

I and dozens of my neighbors urge you to deny the InLine Communities LLC proposal to rezone 8 properties, roughly 22 acres of land on Northern Avenue from R75 to RSM.

I have lived on Northern Avenue for 38 years, initially at what was then Barron Estates Apartments and now in my home at 660 Northern Avenue.

The property in question currently has two inhabited houses and was formerly the site of three other homes. In short, what is being proposed is a change from 5 residential homes with wooded areas, a wet land and flood plain into a development of 149 residences. The poorly thought out proposed development and rezoning includes a 'postage stamp' of greenspace; virtually NO space between the proposed townhouses & single family barely detached homes; new asphalt roads in the development and a plan for two entries and exits onto Northern Avenue. The latter configuration of two roads emptying onto Northern, a busy collector street with frequent traffic of fire trucks, ambulances, school buses (pre-pandemic) and Marta Mobility Paratransit Service vans should be enough in itself to deny this ridiculously dense development. It is not only a question of increased traffic, it is a question of safety. Any entrances on Northern from this overly dense development is dangerous. The property lies between a very hazardous curb and a blind hill that severely limits vision of oncoming traffic in all directions. Indeed, many of us leaving our driveways on Northern, roll down our windows to try to listen for oncoming traffic - because we can't see it. Further, we do not support an entry/exit in Dial Heights either, another neighboring residential area.

This property has a long history. As mentioned, roughly 11 acres of the property was once home to three residences. When the wonderful matriarch of the property at 655 Northern site moved to a personal care home, all three properties were sold to a developer who proceeded to clear cut the land, including a 100 year old specimen oak tree that had been circled to be saved and along with scores of other old growth trees. The contractors left nothing but spindly pines at the ends and sides of the property. Drainage, flooding and runoffs became problematic for contingent home owners. Further, the contractors tore up the street and my neighbor's driveway and when asked about making repairs to the damage they had done, were physically menacing to me and my neighbor, Mark Anthony. They violated with impunity every noise and 'work start time' ordinances and in general made our lives hell for a few years. We do not intend to endure that again and we have concerns about the lack of a track record or even a website for this InLine Communities. Subcontractors that InLine sent to the property for 'due diligence' brought a huge bulldozer and drilling machines and those operators were unable and/or unwilling to provide information about who they worked for and what notice had been given to the community about their noisy land disturbing actions.

Aerial photos provided by the developer do not accurately reflect this community. Northern Avenue is a residential street with all but two homes at one story and three smaller acreage, low height quiet apartment complexes with long-term renters. Most of our homes are, even now, still affordable. Dial

Heights is a community of ranch homes. We are a suburban community. The sheer density of the proposal would more than double the amount of currently existing homes on a street already challenged by traffic, sewer infrastructure, and shrinking greenspace. To quote from Dekalb County Greenspace Program, "Though many consider green space a mere luxury, in Dekalb County it is a rapidly disappearing necessity....Greenspace also helps the community at large by lowering crime and reducing the cost of public services such as police protection and sewer and road maintenance. Not only does our air quality benefit, but treating drinking water also becomes easier and more affordable if we allow the waterways and surrounding ecosystems to naturally cleanse themselves." Of note, there is significant wetland space in the back of this property and Indian Creek runs throughout a number of properties on the east side of Northern. The impact of this development could have a larger environmentally negative effect beyond what is currently being addressed. While it is true that there was a 'rezoning' to MR-2 on a small section of this property, that was done ONLY because the community as a whole supported and felt it would be beneficial to locate a small school specifically for the neighboring immigrant and refugee children. Most folks on Northern Avenue do NOT see any benefit to the existing community from this development.

I am no expert on zoning. But I am an expert on where I live and the quality of life I and many of my neighbors want to maintain. Just because something can be done it doesn't mean it should be done. The first step in this process is to deny the rezoning request to RSM. It is simply not compatible with the existing community. Thank you for your service and your attention to this matter. I am more than happy to discuss my concerns with any of the decision makers in this process and am hopeful that the views of those impacted by this proposal will be given the most weight. Thank you again.

Sincerely,

Rita Valenti 660 Northern Avenue Clarkston, GA 30021 rita1880@att.net 678-328-8725 404-292-1219

### FW: Community feedback opposing Northern Ave rezoning to MR-2 (Z-21-1244531)

### Plansustain <plansustain@dekalbcountyga.gov>

Tue 3/2/2021 9:48 AM

To: 'hope.ranker@gmail.com' <hope.ranker@gmail.com>

Cc: Reid, John < jreid@dekalbcountyga.gov>

Good day, Ms. Ranker. That case is on this Thursday evening's Planning Commission agenda and your email has been forwarded to them and the Sr. Planner (John Reid) assigned to this case.

Thank you.

From: Hope Ranker <hope.ranker@gmail.com>

Sent: Tuesday, March 2, 2021 4:22 AM

To: Terry, Edward C. <ecterry@dekalbcountyga.gov>; Bradshaw, Stephen R. <SRBradshaw@dekalbcountyga.gov>;

Plansustain <plansustain@dekalbcountyga.gov>

Cc: Cato, Kelly E. <kecato@dekalbcountyga.gov>; Brooks, Alesia D. <adbrooks@dekalbcountyga.gov>; Public

Hearing < Public Hearing@dekalbcountyga.gov>

Subject: Community feedback opposing Northern Ave rezoning to MR-2 (Z-21-1244531)

Commissioners Terry and Bradshaw, and other Planning Commission members:

I'm writing to you as a community member to urge you to deny the request of Inline Communities to rezone 22 acres on Northern Ave, petition number Z-21-1244531. The proposed development would do great harm to our community: It would economically endanger and displace our low-income and refugee neighbors, it would increase the traffic danger of an already too-busy road, and it is generally far out of character with the other properties in our neighborhood.

My name is Hope Ranker. Since 2009 my wife and I have lived at 703 Northern Ave, three lots north of the subject property.

My primary concern is the massive social harm the proposed development would do to this suburban neighborhood. Many members of our neighborhood belong to Clarkston's beloved and internationally-recognized refugee community. Like much of the area around Clarkston, incomes are modest: The census tract that contains the subject property has a median household income of \$35k, with \$37k across the street to the west and \$29k across the street to the north. I'll list sources and notes below. Median house values around the property and west across Northern are \$150k. Just across the property's eastern creek and north across Indian Creek Way they are \$75-85k.

According to Ms Battle's presentation at the Community Council meeting, Inline intends to sell its townhomes starting at \$300k, and houses starting at \$450k. The zoning application represents this as "equal to or greater than surrounding home values." This is a huge understatement: It's 2-3 times the median cost of houses to the south and west, and it's 4-6 times the median cost of the condos to the east and north. It's more than median-income neighbors will make in ten years. This is a shocking figure for this neighborhood, and it marks the bottom end of this developer's range. Studies clearly and consistently show that this style of gentrification displaces vulnerable populations like our low-income and refugee neighbors. This is deeply inappropriate and causes direct and clear harm to the community of people who live here.

Secondary to the grievous social impact of this rezoning, it will also increase the already-high danger to me and my neighbors from traffic on Northern Ave. I live just north of the intersection of Northern and Indian Creek Way. In the vicinity of my house and the subject property, there are a series of blind turns and hills. As I leave my driveway I have about 300 feet of visibility in either direction. The county is unable to put speed bumps on the

road, and people regularly speed down it, losing control and crashing at least yearly into my yard or a neighbor's. My wife and I haven't bothered putting up a mailbox because our neighbors' get knocked down every few years.

Someone suggested at the Community Council meeting that more cars may slow down traffic in this section of the road. The applicant's traffic study seems to assure us that it won't. They carefully studied the expected impact of the development on the intersections, including the one right by me and the subject property. They found that rush hour traffic in front of my driveway will increase 11% from 5.7 cars per minute to 6.3, but that it won't cause any slowdown of traffic through our intersection. With Inline's proposed development I will be facing less than ten seconds between each speeding car as I try to leave my driveway with three hundred feet of road visibility. Fixing these problems is outside the scope of zoning, but it's not unfair to ask zoning to refrain from further exacerbating an already difficult road. If instead this application is accepted, I'm left wondering if crashes into my yard and my neighbors' will also increase by 11%.

Finally, aside from socioeconomic harm to the community and the increased traffic risk to me and my nearby neighbors, this proposed development simply doesn't fit in our community. Please take a moment to drive down Northern Ave if you live nearby, or if you don't, use online maps to do it virtually. You'll see that our street is a small, spread out neighborhood of modest homes surrounded in trees and green space. The applicants point out the density of nearby Navarro apartments (19 du/ac), but I would challenge the numbers they found: I calculate them at 12 du/ac, which I'll explain in notes below. If you take a virtual drive down Indian Creek Way you'll see that most of the complexes there had designers take care to incorporate pleasing building arrangements, green spaces, and trees to keep them as appealing as they could manage. Inline's proposed development would stick out like a sore thumb so close to any of the other MR-2 complexes in the area.

Ms Battle is an accomplished professional. Inline Communities hired her to make the case that their zoning change is fair, reasonable, and in line with development in the local community. Speaking as someone whose life and property would be directly negatively impacted by their development, I hope I've painted a very different picture. Inline's development is no fit at all for this community. It would increase danger on an already difficult road. And most importantly the shocking wealth disparity it proposes introducing into this neighborhood would cause long-lasting irreparable socioeconomic harm to my most vulnerable neighbors. I urge you to reject this deeply harmful application.

Thank you for your attention.

Hope Ranker 703 Northern Ave 678.524.2422 hope.ranker@gmail.com

#### **NOTES**

Median household income data is from the US Census Bureau's American Community Survey (2013-2017). The subject property is at the northwest corner of census tract 220.05. There's a nice visualization at: http://www.justicemap.org/index.php?gsLayer=&gfLon=-84.23865199&gfLat=33.80426593&giZoom=14&

My median house value numbers are from:

http://www.city-data.com/city/Clarkston-Georgia.html

in the map visualization a few screens down. The site doesn't provide reliable citations, but the numbers are in line with a cursory review of Dekalb county's GIS site, which includes property values from tax records: https://dekalbgis.maps.arcgis.com/apps/webappviewer/index.html?id=f241af753f414cdfa31c1fdef0924584

There's a long list of studies examining the displacement of vulnerable communities from gentrification here: https://www.urbandisplacement.org/publications

The University of Texas maintains a site with a few detailed case studies:

https://sites.utexas.edu/gentrificationproject/case-studies/

And the CDC maintains a list of references describing the public health impact of this sort of vulnerable population displacement:

https://www.cdc.gov/healthyplaces/healthtopics/gentrification.htm

I've been unable to find complete documentation for the vehicular crashes that plague our road. The DOT's site can be filtered by street name and date, but it's missing a lot of crashes that my neighbors can attest to: https://gdot.numetric.net/crash-data#/?view\_id=7

If you need substantiation of accident frequency, I can reach out to neighbors, several of whom keep thorough notes and pictures.

The traffic study doesn't list cars passing my house, but the appendix tables include cars entering the intersection of Northern and Indian Creek Way from the north and leaving it in that direction. Since I'm the second property north of that intersection, nearly all of those cars speed past my driveway.

You can drive up and down Northern here, facing north (toward Indian Creek Way and my house) just in front of the subject property:

https://www.google.com/maps/@33.7949984,-84.2462329,3a,75y,23.68h,90t/data=!3m7!1e1!3m5!1sPlCglfrYp7f 7yLAg\_w3IYg!2e0!6s%2F%2Fgeo3.ggpht.com%2Fcbk%3Fpanoid%3DPlCglfrYp7f7yLAg\_w3IYg%26output%3Dthum bnail%26cb\_cli

The zoning application shows Navarro Apartments with a density of 19 du/ac. This is an easy mistake to make, but it is a mistake. Navarro Apartments consists of parcel ids 18 066 07 002 through 017. Commercial residential units are in 003 through 017. 002 is the office, yards, and hardscape. If you exclude 002 then the 52 units are split across 2.63 acres, for a density of 19.8 du/ac. If you include 002, the same 52 units are split across 4.17 acres, for a density of 12.5 du/ac. A visual inspection of the property at the county GIS site will clearly show that 002 is part of the complex.